

**The big questions:**

- Is the mobility of sediment in the design channel similar to the reference reach?
- Is sediment transport maintained through the structure during low/moderate floods?
- Are key pieces and grade controls stable?



## Presentation outline

### Background

- Bedload transport
- Channel types and bed mobility/stability

Why do bed stability-mobility analysis?

Flow hydraulics and sediment entrainment

### Design application

- Bed mobility
- Bed stability

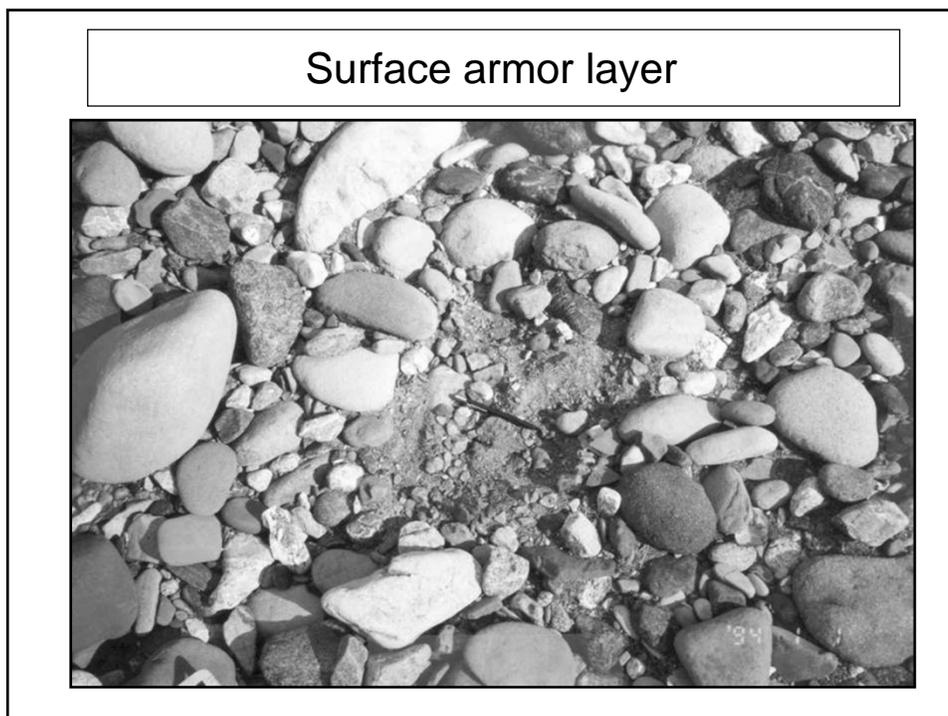
## Bedload transport phases

### Phase I bedload transport:

The transport of fine sediment over  
the immobile armor layer

### Phase II bedload transport:

The armor layer is breached and the  
bed is mobilized



### Relationship between channel type and sediment entrainment/bedload transport

Channel – Bed Type	Relative Mobility	Typical Transport at Bankfull	Structure	Channel Type
<b>Boulder Cobble</b>	Low	Phase I	Steps	Rosgen A Cascade Step Pool
<b>Cobble Gravel</b>	↑	Phase I Phase II	Particle clusters Armor	Rosgen B C Plane Bed Pool Riffle
<b>Sand</b>	↓ High	Phase II	Bed Forms	Rosgen C E Regime

## Bedload transport summary

Sediment entrainment and bedload transport have considerable uncertainty in prediction – uncertainty increases with gradient

- Sand bed streams –  $D_{84}$  may move constantly
- Gravel bed pool-riffle streams -  $D_{84}$  may move every few years
- Steep boulder step-pool streams -  $D_{84}$  may not move for decades



## Presentation outline

### Background

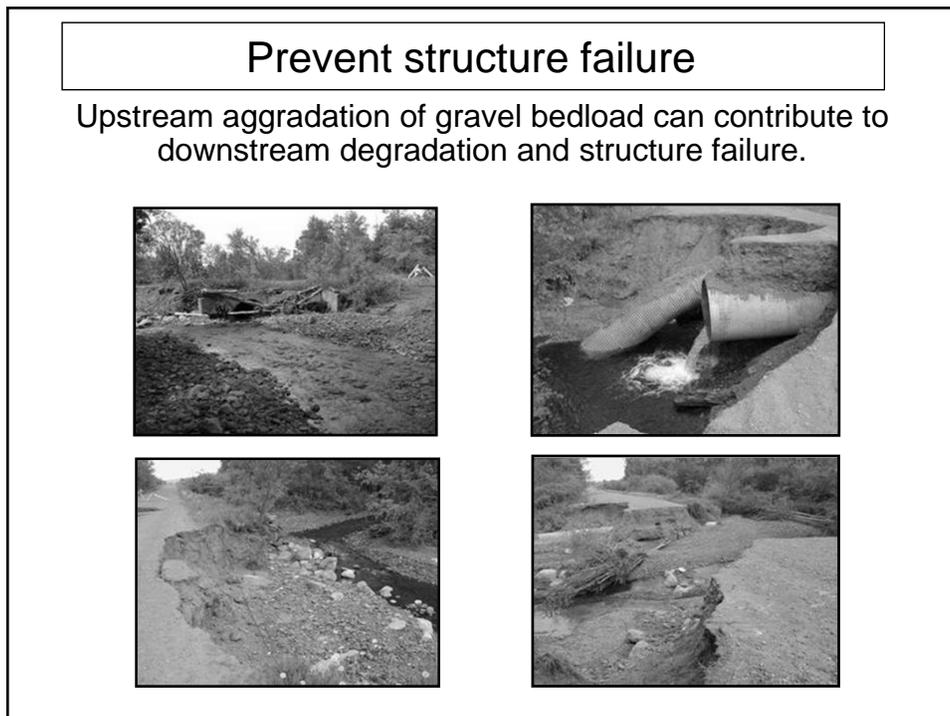
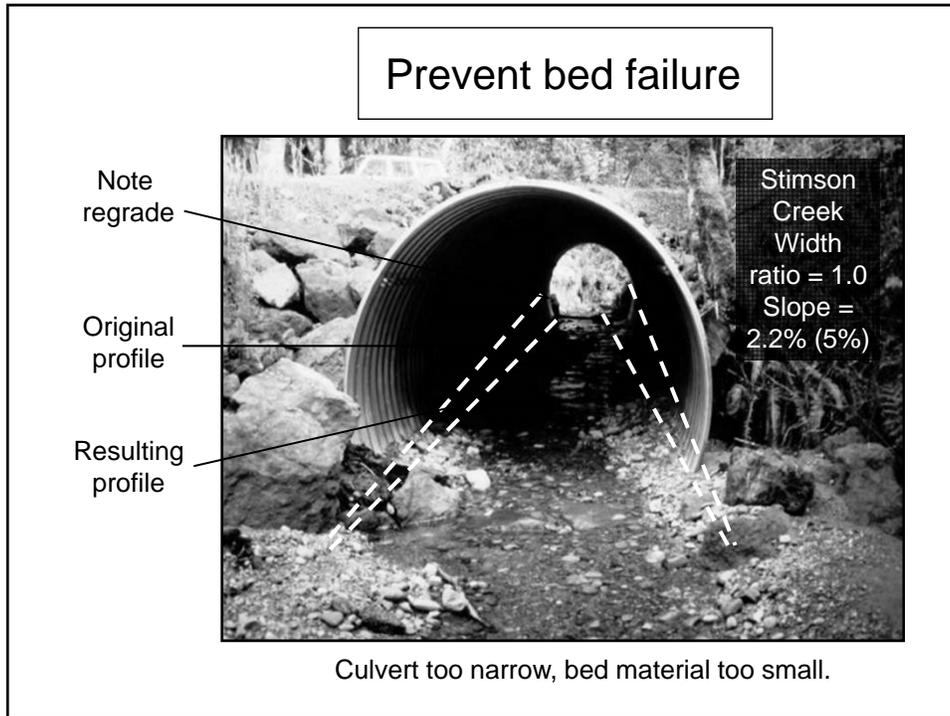
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## Bed mobility and stability considerations?

- Key bed or grade control features
  - steps, particle clusters
- Bank material
- Floodplain contraction
  - entrenchment ratio high
  - floodplain conveyance high

## Presentation outline

### Background

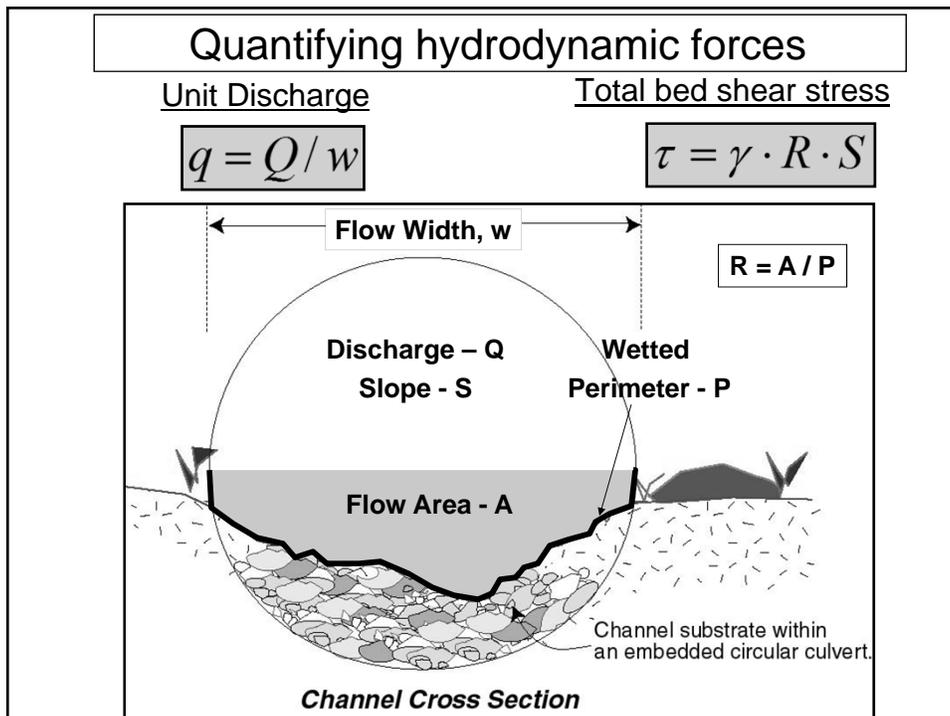
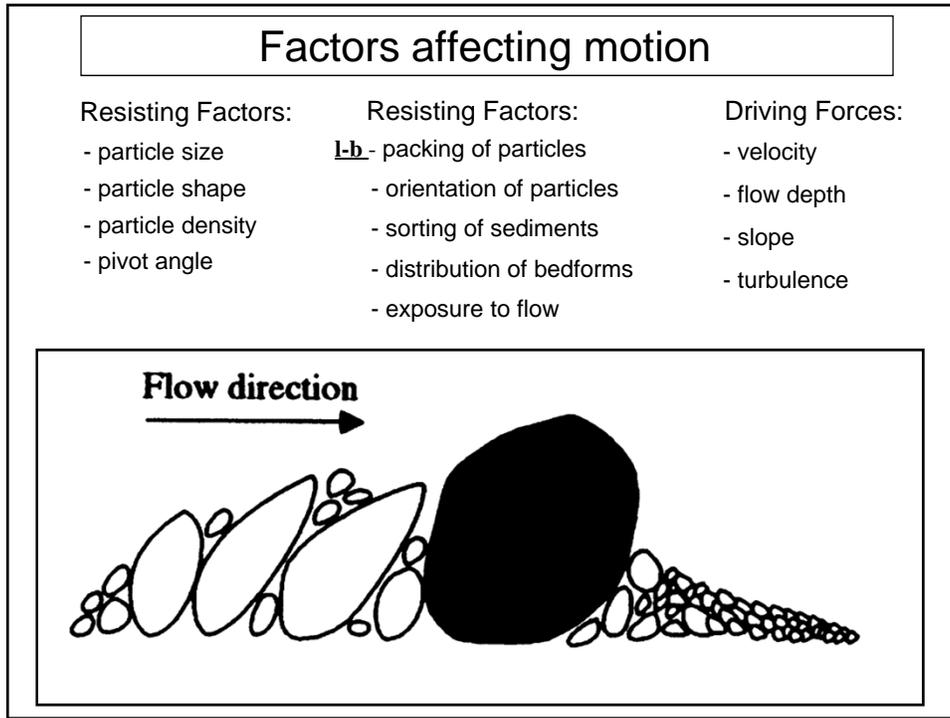
- Bedload transport
- Channel types and bed mobility/stability

### Why do bed stability-mobility analysis?

### Flow hydraulics and sediment entrainment

### Design application

- Bed mobility
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### Early effort to define motion

Shields (1936) for UNIFORM grains on flat bed

$$\tau_{cri}^* = \frac{\tau_{cri}}{\gamma(G-1)D_i} = \frac{\gamma RS}{\gamma(G-1)D_i} = \frac{RS}{(G-1)D_i}$$

$\tau_{cri}^*$  = critical Shields stress

$\tau_{cri}$  = critical shear stress - particle begins to move

$\gamma$  = unit weight of water

R = hydraulic radius

S = stream slope

G = specific gravity of sediment

$D_i$  = sediment particle size of interest

### Entrainment threshold table

Table 2. Entrainment thresholds for different particle sizes (modified from Julien, 1995). The Shields parameter and critical shear stress values are for the smallest number in the particle-size interval.

Particle size classification	Particle size, $D_s$ (mm)	angle of repose, $\phi$ (degrees)	Shields parameter, $\tau_*$	critical shear stress, $\tau_{cr}$ ( $N/m^2$ )
very large boulders	> 2048	42	0.054	1789
large boulders	1024-2048	42	0.054	895
medium boulders	512-1024	42	0.054	447
small boulders	256-512	42	0.054	224
large cobbles	128-256	42	0.054	112
small cobbles	64-128	41	0.052	54.0
very coarse gravels	32-64	40	0.050	26.1
coarse gravels	16-32	38	0.047	12.1
medium gravels	8-16	36	0.044	5.64
fine gravels	4-8	35	0.042	2.72
very fine gravels	2-4	33	0.039	1.26
very coarse sands	1-2	32	0.029	0.48
coarse sands	0.5-1.0	31	0.033	0.27
medium sands	0.25-0.50	30	0.048	0.19
fine sands	0.125-0.25	30	0.072	0.15
very fine sands	0.0625-0.125	30	0.109	0.11
coarse silt	0.0313-0.0625	30	0.165	0.084
medium silt	0.0156-0.0313	30	0.250	0.063

a. equations used to determine Shield's parameter:  
 $\tau_* = 0.25 (25296D)^{-0.6} \tan\phi$  when  $0.012 \text{ mm} < D < 0.75 \text{ mm}$  (medium silt to coarse sand)  
 $\tau_* = 0.013 (25296D)^{0.4} \tan\phi$  when  $0.75 \text{ mm} < D < 2 \text{ mm}$  (coarse sand to very coarse sand)  
 $\tau_* = 0.06 \tan\phi$  when  $D \geq 2 \text{ mm}$  (gravels, cobbles, and boulders)

### Application

Resisting force

$$\tau_{cri}^* = \frac{\tau_{cri}}{\gamma(G-1)D_i}$$

Driving force

$$\tau = \gamma \cdot R \cdot S$$

$$\tau_{cri} = \tau_{cri}^* \gamma(G-1)D_i$$

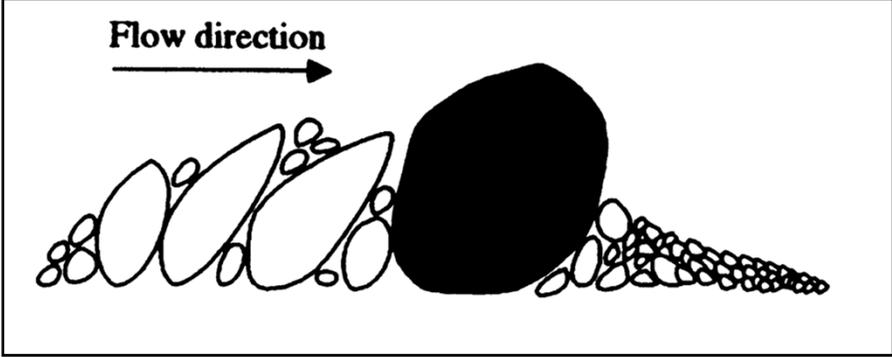
If  $\tau > \tau_{cri}$ , particle will move

If  $\tau < \tau_{cri}$ , particle will not move

Real stream beds are NOT UNIFORM

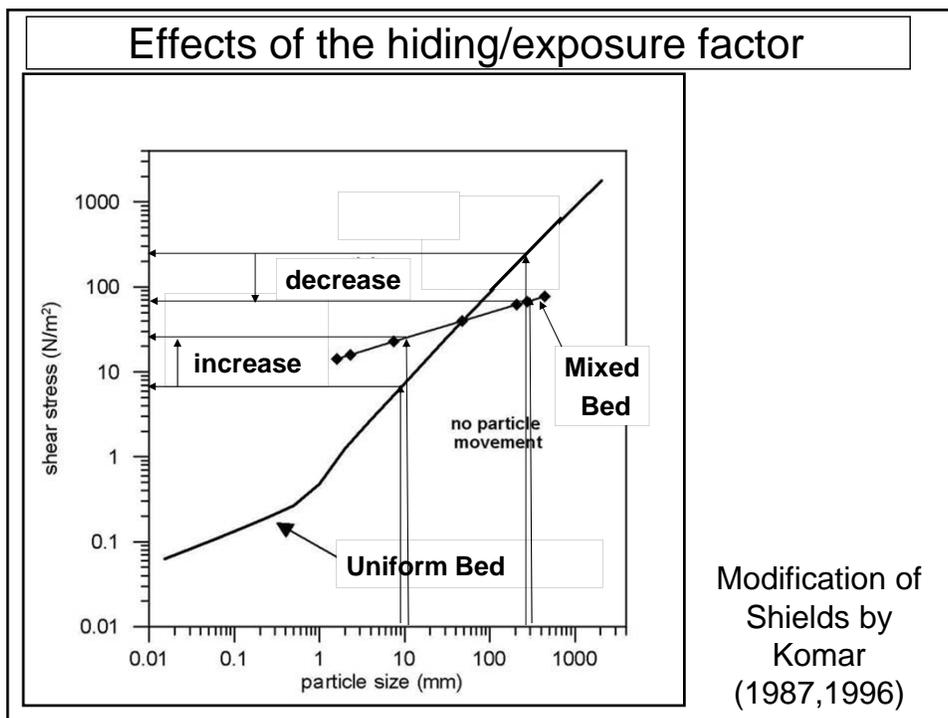


### Hiding/exposure for mixed grain sizes



Smaller particles shielded behind larger particles  
(less mobile than uniform bed)

Larger particles project into flow  
(more mobile than uniform bed)



Hiding/exposure factor  
for beds of mixed sizes

$$\tau_{cri} = f\left(\frac{D_i}{D_{50}}\right)$$

How big is my particle size  
compared to the median size on bed?

Modification of Shields  
Komar (1987, 1996)

Resisting force

$$\tau_{cri} = \lambda(G-1)\tau_{D50}^* D_i^{0.3} D_{50}^{0.7}$$

$$\lambda(G-1) = 16,170$$

Driving force

$$\tau = \gamma \cdot R \cdot S$$

If  $\tau > \tau_{cri}$ , particle will move

If  $\tau < \tau_{cri}$ , particle will not move

(Most applicable on slopes  $< 3\%$ ,  $D_i/D_{50}$  ratios  $< 25$ ,  
 $R/D_{84} > 5$ ,  $R/D_{50} > 10$ )

### Critical unit discharge Bathurst (1987)

Resisting force

$$q_{ci} (\text{m}^2/\text{s}) = q_{cr} [D_i / D_{50}]^b$$

$$q_{cr} = 0.15 g^{1/2} D_{50}^{3/2} S^{-1.12}$$

$$b = 1.5 [D_{84} / D_{16}]^{-1}$$

If  $q > q_{ci}$ , particle will move

If  $q < q_{ci}$ , particle will not move

(Most applicable on slopes  $> 2$  to  $3\%$ , and  
 $R/D_{84} < 5$  and  $R/D_{50} < 10$ )

Driving force

$$q = Q / w$$

### Presentation outline

#### Background

- Bedload transport
- Channel types and bed mobility/stability

Why do bed stability-mobility analysis?

Flow hydraulics and sediment entrainment

#### Design application

- Bed mobility
- Bed stability

### Bed mobility analysis - concepts

- Compares design channel bed to reference reach
- $D_{84}$  particle sizes should mobilize at the same flow in both channels
- Channel roughness, channel form, and bed mobility are controlled by  $D_{84}$  particles sizes
- Comparative analysis based on reference reach calibrates the model; therefore analysis is not sensitive to hydrology, hydraulics and model inputs or assumptions

### Procedure for bed mobility analysis: Reference reach

1. Determine flow hydraulics
  - Calculate shear stress and/or unit discharge
  - Active channel
  - Range of discharges:  $Q_{bf}$ ,  $Q_{1.5}$ - $Q_{100}$
2. Calculate critical shear stress and/or unit discharge
  - $D_{84}$  particle size
  - Entire particle size distribution
3. Determine when particle mobility occurs



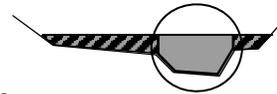
**Procedure for bed mobility analysis:  
Design channel**

**4. Determine flow hydraulics**

- Calculate shear stress and/or unit discharge
- Range of discharges:  $Q_{bf}$ ,  $Q_{1.5}$ - $Q_{100}$

**5. Calculate critical shear stress and/or unit discharge**

- $D_{84}$  particle size
- Entire particle size distribution



**6. Determine when particle mobility occurs**

**Procedure for bed mobility analysis:  
Adjust design bed as needed**

**7. Compare reference and design beds**

- Does  $D_{84}$  mobility occur at the same discharge?
- If yes, proceed to stability analysis
- If no, adjust particle sizes in the design channel until mobility occurs at the same discharge as in the reference reach

**8. If the particle size adjustment exceeds 25%,  
reevaluate design and make further adjustments**

### Bed stability analysis - concepts

- Key pieces provide stable banks and bed forms
- Key pieces are critical for bank stability and form due to lack of vegetation in culverts and under bridges
- Key pieces are permanent - up to stability design flow
- What is stability design flow?  
 $Q_{100}$  or higher



### Procedure for bed stability analysis

- Use same procedures as for mobility analysis
  - Shear Stress
  - Unit Discharge
- Except use  $D_{84}$  from key pieces (10 largest particles) in reference reach



## Sediment mobility (D84) and sediment stability (key pieces)

Example: Rountree Trib at W Main St

### Sediment mobility (Excel Example – Shear Stress – XS 2 Ref Reach)

Hydraulics											Particle Mobility/Stability						
Recurrence Interval	Discharge, Q (ft <sup>3</sup> /s)	Flood-plain n value	Channel n value	Channel slope, S <sub>c</sub>	Energy slope, S <sub>e</sub>	Total flow width, W <sub>t</sub> (ft)	Channel flow width, W <sub>c</sub> (ft)	Total hydraulic radius, R <sub>t</sub> (ft)	Channel Hydraulic Radius, R <sub>c</sub> (ft)	Total boundary shear stress, τ <sub>t</sub> (lb/ft <sup>2</sup> ) <sup>a</sup>	Channel boundary shear stress, τ <sub>c</sub> (lb/ft <sup>2</sup> ) <sup>b</sup>	D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose φ	Shield's entrainment for τ <sub>c,D50</sub> <sup>c</sup>	Critical Shear Stress to Entrain Particle Size, τ <sub>c,D84</sub> (lb/ft <sup>2</sup> ) <sup>d</sup>	D <sub>84</sub> Particle Mobile (yes/no)
<b>A. Reference reach cross section: XS2</b>																	
	50	0.080	0.040	0.0222	0.0194	25.55	9.99	0.52	0.85	0.63	<b>1.02</b>	45	150	42	0.054	1.18	no
	60	0.080	0.040	0.0222	0.0200	26.69	10.56	0.58	0.88	0.73	<b>1.10</b>	45	150	42	0.054	1.18	no
1.5	90	0.080	0.040	0.0222	0.0228	29.24	11.86	0.71	0.96	1.00	<b>1.36</b>	45	150	42	0.054	1.18	yes
2	110	0.080	0.040	0.0222	0.0225	30.96	12.73	0.79	1.01	1.11	<b>1.43</b>	45	150	42	0.054	1.18	yes
5	195	0.080	0.040	0.0222	0.0211	46.16	14.30	0.88	1.30	1.16	<b>1.72</b>	45	150	42	0.054	1.18	yes
10	242	0.080	0.040	0.0222	0.0224	47.92	14.30	0.98	1.43	1.37	<b>2.00</b>	45	150	42	0.054	1.18	yes
50	390	0.080	0.040	0.0222	0.0211	53.67	14.30	1.27	1.84	1.68	<b>2.43</b>	45	150	42	0.054	1.18	yes
100	495	0.080	0.040	0.0222	0.0215	56.81	14.30	1.43	2.07	1.92	<b>2.78</b>	45	150	42	0.054	1.18	yes
500	694	0.080	0.040	0.0222	0.0204	61.25	14.30	1.73	2.49	2.21	<b>3.16</b>	45	150	42	0.054	1.18	yes
500+	974	0.080	0.040	0.0222	0.0201	65.23	14.30	2.10	2.96	2.63	<b>3.71</b>	45	150	42	0.054	1.18	yes

D<sub>84</sub> mobilizes in reference reach between 60 and 90 cfs  
 (~1.5-yr flood)

### Sediment Mobility

(Excel Example – Shear Stress – XS 4.5 Middle 20' Wide Conspan Culvert)

Hydraulics											Particle Mobility/Stability						
Recurrence Interval	Discharge, Q (ft <sup>3</sup> /s)	Floodplain value	Channel value	Channel slope, S <sub>c</sub>	Energy slope, S <sub>e</sub>	Total flow width, W <sub>t</sub> (ft)	Channel width, W <sub>c</sub> (ft)	Total hydraulic radius, R <sub>t</sub> (ft)	Channel Hydraulic Radius, R <sub>c</sub> (ft)	Total boundary shear stress, τ <sub>b</sub> (lb/ft <sup>2</sup> ) <sup>a</sup>	Channel boundary shear stress, τ <sub>c</sub> (lb/ft <sup>2</sup> ) <sup>b</sup>	D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose φ	Shield's entrainment for τ <sub>c,0.050</sub> <sup>c</sup>	Critical Shear Stress to Entrain D <sub>84</sub> Particle Size, τ <sub>c,D84</sub> (lb/ft <sup>2</sup> ) <sup>d</sup>	D <sub>84</sub> Particle Mobile (yes/no)
<b>B. Preliminary stream simulation design channel for 20'x8' Conspan culvert with a channel-bed gradient of 0.0207 (mid culv)</b>																	
○	50	0.080	0.040	0.0207	0.0207	11.01	11.01	0.88	0.88	1.14	1.14	45	150	42	0.054	1.18	no
1.5	60	0.080	0.040	0.0207	0.0207	12.05	12.05	0.93	0.93	1.20	1.20	45	150	42	0.054	1.18	yes
	90	0.080	0.040	0.0207	0.0207	14.54	14.54	1.06	1.06	1.37	1.37	45	150	42	0.054	1.18	yes
2	110	0.080	0.040	0.0207	0.0207	20.00	15.00	0.91	1.18	1.17	1.52	45	150	42	0.054	1.18	yes
5	195	0.080	0.040	0.0207	0.0191	20.00	15.00	1.34	1.67	1.60	2.00	45	150	42	0.054	1.18	yes
10	242	0.080	0.040	0.0207	0.0186	20.00	15.00	1.53	1.91	1.77	2.22	45	150	42	0.054	1.18	yes
50	390	0.080	0.040	0.0207	0.0177	19.99	15.00	2.01	2.56	2.23	2.83	45	150	42	0.054	1.18	yes
100	495	0.080	0.040	0.0207	0.0172	19.99	15.00	2.29	2.97	2.46	3.19	45	150	42	0.054	1.18	yes
500	694	0.080	0.040	0.0207	0.0164	19.99	15.00	2.73	3.68	2.80	3.77	45	150	42	0.054	1.18	yes
500+	974	0.080	0.040	0.0207	0.0155	19.98	15.00	3.22	4.58	3.12	4.44	45	150	42	0.054	1.18	yes

D<sub>84</sub> mobilizes in middle of culvert between 50 and 60 cfs  
(<1.5-yr flood)

### Sediment mobility

(Excel Example – Shear Stress – XS 4.5 Middle 16' Wide Box Culvert)

Hydraulics											Particle Mobility/Stability						
Recurrence Interval	Discharge, Q (ft <sup>3</sup> /s)	Floodplain value	Channel value	Channel slope, S <sub>c</sub>	Energy slope, S <sub>e</sub>	Total flow width, W <sub>t</sub> (ft)	Channel width, W <sub>c</sub> (ft)	Total hydraulic radius, R <sub>t</sub> (ft)	Channel Hydraulic Radius, R <sub>c</sub> (ft)	Total boundary shear stress, τ <sub>b</sub> (lb/ft <sup>2</sup> ) <sup>a</sup>	Channel boundary shear stress, τ <sub>c</sub> (lb/ft <sup>2</sup> ) <sup>b</sup>	D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose φ	Shield's entrainment for τ <sub>c,0.050</sub> <sup>c</sup>	Critical Shear Stress to Entrain D <sub>84</sub> Particle Size, τ <sub>c,D84</sub> (lb/ft <sup>2</sup> ) <sup>d</sup>	D <sub>84</sub> Particle Mobile (yes/no)
<b>C. Adjusted stream simulation design channel for 20'x8' Conspan culvert with a channel-bed gradient of 0.0207 (mid culv)</b>																	
○	50	0.080	0.040	0.0207	0.0207	11.01	11.01	0.88	0.88	1.14	1.14	50	165	41	0.052	1.25	no
1.5	60	0.080	0.040	0.0207	0.0207	12.05	12.05	0.93	0.93	1.20	1.20	50	165	41	0.052	1.25	no
	90	0.080	0.040	0.0207	0.0206	14.54	14.54	1.06	1.06	1.37	1.37	50	165	41	0.052	1.25	yes
2	110	0.080	0.040	0.0207	0.0207	20.00	15.00	0.91	1.18	1.17	1.52	50	165	41	0.052	1.25	yes
5	195	0.080	0.040	0.0207	0.0191	20.00	15.00	1.34	1.67	1.60	2.00	50	165	41	0.052	1.25	yes
10	242	0.080	0.040	0.0207	0.0186	20.00	15.00	1.53	1.91	1.77	2.22	50	165	41	0.052	1.25	yes
50	390	0.080	0.040	0.0207	0.0177	19.99	15.00	2.01	2.56	2.23	2.83	50	165	41	0.052	1.25	yes
100	495	0.080	0.040	0.0207	0.0172	19.99	15.00	2.29	2.97	2.46	3.19	50	165	41	0.052	1.25	yes
500	694	0.080	0.040	0.0207	0.0164	19.99	15.00	2.73	3.68	2.80	3.77	50	165	41	0.052	1.25	yes
500+	974	0.080	0.040	0.0207	0.0155	19.98	15.00	3.22	4.58	3.12	4.44	50	165	41	0.052	1.25	yes

With 10% increase in bed material size,  
D<sub>84</sub> mobilizes in middle of culvert between 60 and 90 cfs  
(~1.5-yr flood)

## Sediment mobility

(Excel Ex – Shear Stress – 20' Conspan Culvert Adjustment +10%)

Ref Reach (XS2):  $D_{84}$  mobile at 55 cfs ( $<Q_{1.5}$ )  
 Initial Bed:  $D_{84}$  mobile at 75 cfs ( $\sim Q_{1.5}$ )  
 Adjusted Bed:  $D_{84}$  mobile at 75 cfs ( $\sim Q_{1.5}$ )

D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose $\phi$	Description
45	150	42	initial reference reach particle size
49.5	165.0	42	adjusted particle sizes in design channel
		<b>10</b>	<b>percent particle size adjustment</b>

## Sediment stability

(Excel Example – Shear Stress – XS 2 Ref Reach )

Hydraulics											Particle Mobility/Stability						
Recur- rence Interval	Dis- charge, Q (ft <sup>3</sup> /s)	Flood- plain n value	Chan- nel n value	Chan- nel slope, S <sub>c</sub>	Energy slope, S <sub>e</sub>	Total flow width, W <sub>t</sub> (ft)	Chan- nel flow width, W <sub>c</sub> (ft)	Total hydraulic radius, R <sub>t</sub> (ft)	Channel Hydraulic Radius, R <sub>c</sub> (ft)	Total boundary shear stress, $\tau_b$ (lb/ft <sup>2</sup> ) <sup>a</sup>	Channel boundary shear stress, $\tau_c$ (lb/ft <sup>2</sup> ) <sup>b</sup>	D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose $\phi$	Shield's entrain- ment for $\tau_{c0.050}^c$	Critical Shear Stress to Entrain D <sub>84</sub> Particle Size, $\tau_{c0.084}$ (lb/ft <sup>2</sup> ) <sup>d</sup>	D <sub>84</sub> Particle Stable yes/no
<b>A. Reference reach cross section: XS2</b>																	
	50.0	0.080	0.040	0.0222	0.0194	25.55	9.99	0.52	0.85	0.63	<b>1.02</b>	474	528	42	0.054	<b>8.93</b>	yes
	60.0	0.080	0.040	0.0222	0.0200	26.69	10.56	0.58	0.88	0.73	<b>1.10</b>	474	528	42	0.054	<b>8.93</b>	yes
1.5	90.0	0.080	0.040	0.0222	0.0228	29.24	11.86	0.71	0.96	1.00	<b>1.36</b>	474	528	42	0.054	<b>8.93</b>	yes
2	110.0	0.080	0.040	0.0222	0.0225	30.96	12.73	0.79	1.01	1.11	<b>1.43</b>	474	528	42	0.054	<b>8.93</b>	yes
5	195.0	0.080	0.040	0.0222	0.0211	46.16	14.30	0.88	1.30	1.16	<b>1.72</b>	474	528	42	0.054	<b>8.93</b>	yes
10	242.0	0.080	0.040	0.0222	0.0224	47.92	14.30	0.98	1.43	1.37	<b>2.00</b>	474	528	42	0.054	<b>8.93</b>	yes
50	390.0	0.080	0.040	0.0222	0.0211	53.67	14.30	1.27	1.84	1.68	<b>2.43</b>	474	528	42	0.054	<b>8.93</b>	yes
100	495.0	0.080	0.040	0.0222	0.0215	56.81	14.30	1.43	2.07	1.92	<b>2.78</b>	474	528	42	0.054	<b>8.93</b>	yes
500	694.0	0.080	0.040	0.0222	0.0204	61.25	14.30	1.73	2.49	2.21	<b>3.16</b>	474	528	42	0.054	<b>8.93</b>	yes
500+	974.0	0.080	0.040	0.0222	0.0201	65.23	14.30	2.10	2.96	2.63	<b>3.71</b>	474	528	42	0.054	<b>8.93</b>	yes

**D<sub>84</sub> (21 inches or 528 mm) for key pieces stable at all flows.**

## Sediment stability

(Excel Example – Shear Stress – XS 4.5 16' Box Culvert)

Hydraulics											Particle Mobility/Stability						
Recur- rence Interval	Dis- charge, Q (ft <sup>3</sup> /s)	Flood- plain n value	Chan- nel n value	Chan- nel slope, S <sub>c</sub>	Energy slope, S <sub>e</sub>	Total flow width, W <sub>t</sub> (ft)	Chan- nel flow width, W <sub>c</sub> (ft)	Total hydraulic radius, R <sub>t</sub> (ft)	Channel Hydraulic Radius, R <sub>c</sub> (ft)	Total boundary shear stress, τ <sub>t</sub> (lb/ft <sup>2</sup> ) <sup>a</sup>	Channel boundary shear stress, τ <sub>c</sub> (lb/ft <sup>2</sup> ) <sup>b</sup>	D <sub>50</sub> (mm)	D <sub>84</sub> (mm)	Angle of repose φ	Shield's entrain- ment for τ* <sub>0.05</sub> <sup>c</sup>	Critical Shear Stress to Entrain D <sub>84</sub> Particle Size, τ <sub>c,084</sub> (lb/ft <sup>2</sup> ) <sup>d</sup>	D <sub>84</sub> Particle Stable yes/no
<b>B. Preliminary stream simulation design channel for 20'x8' Conspan culvert with a channel-bed gradient of 0.0207 (mid culv)</b>																	
	50.0	0.080	0.040	0.0207	0.0207	11.01	11.01	0.88	0.88	1.14	1.14	474	528	42	0.054	8.93	yes
	60.0	0.080	0.040	0.0207	0.0207	12.05	12.05	0.93	0.93	1.20	1.20	474	528	42	0.054	8.93	yes
1.5	90.0	0.080	0.040	0.0207	0.0206	14.54	14.54	1.06	1.06	1.37	1.37	474	528	42	0.054	8.93	yes
2	110.0	0.080	0.040	0.0207	0.0207	20.00	15.00	0.91	1.18	1.17	1.52	474	528	42	0.054	8.93	yes
5	195.0	0.080	0.040	0.0207	0.0191	20.00	15.00	1.34	1.67	1.60	2.00	474	528	42	0.054	8.93	yes
10	242.0	0.080	0.040	0.0207	0.0186	20.00	15.00	1.53	1.91	1.77	2.22	474	528	42	0.054	8.93	yes
50	390.0	0.080	0.040	0.0207	0.0177	19.99	15.00	2.01	2.56	2.23	2.83	474	528	42	0.054	8.93	yes
100	495.0	0.080	0.040	0.0207	0.0172	19.99	15.00	2.29	2.97	2.46	3.19	474	528	42	0.054	8.93	yes
500	694.0	0.080	0.040	0.0207	0.0164	19.99	15.00	2.73	3.68	2.80	3.77	474	528	42	0.054	8.93	yes
500+	974.0	0.080	0.040	0.0207	0.0155	19.98	15.00	3.22	4.58	3.12	4.44	474	528	42	0.054	8.93	yes

D<sub>84</sub> (21 inches or 528 mm) for key pieces stable at all flows.