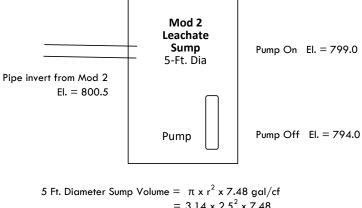
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SCS ENGINE	ERS	Sheet No. 4	of	5
		Calc. No.		
		Rev. No.		
ob No. 25222260.00	Job: Columbia Dry Ash Disposal Facility	By: MJT	Date: 8/31	1/23
Client: Wisconsin Power & Light	Subject: Sump Volume and Pump Capacity	Chk'd: KRG	Date: 9/1/	/23

Sump Volumes

Sump (For Module 2)



$$= 3.14 \times 2.5^{2} \times 7.48$$

$$= 3.14 \times 2.5^{2} \times 7.48$$

$$= 147 \quad \text{gal/ft}$$

$$= 376 \quad \text{gal.ft} \times 5 \quad \text{ft}$$

$$= 734 \quad \text{gal}$$

Calculations:

	Available Volume in Mod 2 Sump (gals)	Leachate Generation Rate (gal/day)	Fill Time (hours)
Module 2 (Current Conditions)	734	1,711	10.3
Module 2 (Final Conditions)	734	293	60.2

Sump filling times provide adequate pump rest time during active conditions with open and closed conditions.

 $I:\ 25222260.00\ Data\ and\ Calculations\ Leachate\ [E4-5_Mod\ 1\ and\ 2\ Leachate\ Sump\ Sizing\ and\ Pump\ Cycle\ Time.xls] Page\ 4$

Sheet No.	5 of 5
Calc. No.	
Rev. No.	
By: MJT	Date: 8/31/23
Chk'd: KRG	Date: 9/1/23

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Client: Wisconsin Power & Light Subject: Sump Volume and Pump Capacity

Calculations:

Determine preliminary pumping rates and times based on assumed sump filling rate

Filling rate (gpm) = Sump Volume (gal) / Sump Fill Time (hr) / 60 min/hr

Dewatering Time (hr) = Sump Volume (gal) / (Pumping Rate - Filling Rate (gpm)) / 60 min/hr

Pump Rest Time (hr) = Sump Fill Time (hr)

Mod 1 Sump - (assume pump will pump at 17 gpm with all pumps running, see pump sizing calculation)

	Available Sump Volume, gal	Sump Fill Time, hrs	Filling Rate, gpm	Pumping Rate, gpm	Dewatering Time, hrs	Pump Rest Time, hrs
Mod 1 (Current Conditions)	587	8.9	1.1	1 <i>7</i> .0	0.6	8.9
Mod 1 (Final Conditions)	587	22.9	0.4	17.0	0.6	22.9

Mod 2 Sump - (assume pump will pump at 17 gpm with all pumps running, see pump sizing calculation)

	Available Sump Volume, gal	Sump Fill Time, hrs	Filling Rate, gpm	Pumping Rate, gpm	Dewatering Time, hrs	Pump Rest Time, hrs
Mod 2 (Current Conditions)	587	10.3	1.0	17.0	0.6	10.3
Mod 2 (Final Conditions)	587	60.2	0.2	17.0	0.6	60.2

Conclusion:

The required pumping rates are readily achievable with available pumps, such as EPG Model 5 series.

Sheet No.	1
Calc. No.	
Rev. No.	
By: MJT	Date : 8/21/2023
Chrid KDC	Date: 8/24/23

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Client: Wisconsin Power & Light Subject: Leachate Storage Capacity Chk'd KRG Date: 8/24/23

Leachate Generated at the Site During Closed Conditions

Purpose:

To determine the amount of leachate storage required when the site is closed and final cover is installed. Leachate will be collected and pumped to a storage tank.

Approach:

Determine the area of final cover, use 1 inch/year for leachate generation per NR 512.12(3)(b).

Determine the leachate generated per day and the tank volume needed to store 4 days of leachate.

Assumptions:

The site is fully constructed (Modules 1 - 6, and 10 - 13 are constructed)

There is final cover on all Modules with a leachate generation rate of 1 inch/year

Calculations:

Entire Area of Filling (Final Cover):

Mod 1	8.28	acres	Mod 5	4.12	acres	Mod 11	3.48	acres
Mod 2	4.05	acres	Mod 6	3.79	acres	Mod 12	4.65	acres
Mod 3	3.98	acres	Mod 10	3.42	acres	Mod 13	2.48	acres
	4.05							

Mod 4 4.05 acres

Total: 42.30 acres

Final Cover (Volume per day)

42.30 acres X 1
$$\frac{\text{in}}{\text{yr}}$$
 X $\frac{1}{12}$ $\frac{\text{ft}}{\text{in}}$ X 43,560 $\frac{\text{ft}^2}{\text{acre}}$ X $\frac{1}{365}$ $\frac{\text{yr}}{\text{365}}$ days

= 420.7 $\frac{\text{ft}^3}{\text{day}}$ X 7.48 $\frac{\text{gal}}{\text{ft}^3}$ = 3,147 $\frac{\text{gallons}}{\text{day}}$ X 4 days

= 12,587 gallons

Results: 4 days of leachate production when the site is 100% covered in final cover requires a 15,000 gallon storage tank.

SCS ENGINEER	S	Sheet No.	1 of 5
		Calc. No.	
		Rev. No.	
Job No. 25222260.00	Job: Columbia Dry Ash Disposal Facility	By: MJT	Date: 8/22/23
Client: Wisconsin Power & Light	Subject: Leachate Loadout Facility Pump Design	Chk'd: KRG	Date: 8/24/23

Purpose:

To size the leachate loadout facility pump.

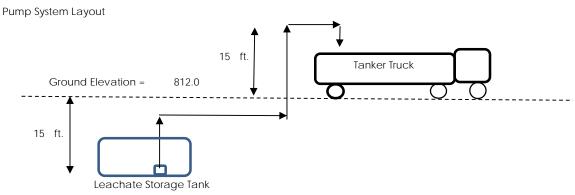
Approach:

Determine the total dynamic head that the pump will operate against under various pumping rates.

The total dynamic head is a combination of the static head and friction losses from fittings and flow through the pipe.

Plot the resulting system curve on a representative pump curve to determine the suitability of a pump.

Calculations:



Assume 2.5 in. diameter pipe Pipe Length = 75 ft.

Static Head

Tank Pump: Pump Off Elevation = 797.0 Assumes: 5 feet of cover over tank feet tank diameter

Loadout Gantry Elev. = 827.0

> Assumes: Gantry is 15 feet above the ground surface

Static Head 827.0 797.0 30.0 feet

Results:

The Goulds Pump Model 2GFK3212G, 3.2 hp, 200V will pump at 138 gpm. (See Sheet 5)

Sheet No. 2 of 5 Calc. No. Rev. No. By: MJT Date: 8/22/23 Subject: Leachate Loadout Facility Pump Design Chk'd: KRG Date: 8/24/23

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Calculations (cont.):

Client: Wisconsin Power & Light

Fittings and Equivalent Length (see Sheet 4)

From Pump to Loadout	Eq.	Length	(ft)		Total Eq. Length (ft)
90° Elbows, 2.5"	3	Χ	10	=	30
Union Fitting, 2.5"	1	Χ	4	=	4
Check Valve, 2.5"	1	Χ	15	=	15
			Tota	ıl:	49
Actual Length (2.5" Steel)					75
	Ed	guivale	nt Lena	124	

Flowrate	Static Head	Equivalent Length	Head Loss	Head Loss	Total Dynamic
(GPM)	(Ft)	(Ft)	Per 100 Ft.	(Ft)	Head (Ft)
2.5" Pipe (Pump	,				
75	30	124	3.88	4.81	34.8
100	20	124	/ /1	8.20	38.2
100	30	124	6.61	8.20	38.2
125	30	124	10.01	12.41	42.4
150	30	124	14.01	17.37	47.4

Sheet No. 3 of 5

Calc. No.

Rev. No.

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

By: MJT Date: 8/22/23

Client: Wisconsin Power & Light Subject: Leachate Loadout Facility Pump Design

Chk'd: KRG Date: 8/24/23

Engineering & Design Data



FLOW VELOCITY & FRICTION LOSS

											S	DR	13.	5												
Floor Rate Gallorral Minute)	entric films	Velocity (No.)	Fristion Head Loss (fl costs)	Friction Pressure (psi/ 1968)	Wiscity (file)	Friction Head Loss (R cates) 1008)	Priction Pressure (psi) 100m	Velocity (file)	Friction Head Loss (ft states)	Friction Pressure (psi/ (00h)	Writesity (file)	Friction Head Loss (R materi 10010)	Friction Pressure (psV (40m)	Valocity (f.h)	Friction Head Loss (ft water 1008)	Friction Pressure (psil/ 1998)	Valority (th)	Friction Head Lose (R cated 1000)	Friction Prinsular goal/ (seep)	Unlocity (Bits)	Friction Head Love (N water/ 1000)	Friction Pressure (pell 1966)	Velocity (file)	Friction Head Loss (It system 100H)	Friction Pressure (said (004)	
GPM			1/2"			3/4"			17			1-1/4"			1-1/2"			2"			2-1/2"			3		GPM
3	0.002	0.88	103	0.86	0.64	0.34	0.16																		П	4
2	0.004	1.69	2.06	0.83	437	0.68	0.29	0.02	0.40	0.17	0.42	0.10	0.00	0.32	0.009	0.098	0.50	0.08	0.018							2
5	0.011	422	11.58	5.01	268	3 82	168	1.69	1.24	0.54	105	059	0.17	0.80	0.50	0.088	0.51	0.025	0.033	036	0.088	0.016	0.24	0.02	0.000	5
7	9,016	5.81	2124	9.20	375	7.01	3,00	287	220	0.89	1.47	0.72	0.31	1,12	0.37	0.19	0.72	0.125	0.054	0.43	0.63	0.023	033	0.00	0.012	7
10	0.022	8,44	80.46	17,52	5,35	13.84	5.78	3.37	433	1.87	510	107	0.69	(00)	971	0.51	3.02	0.54	0.10	0.70	0.09	0.088	0.47	0.04	0.047	10
15.	0.033		4"		100	28.27	12.24	5.06	9.18	3.87	11.15	591	1.26	2.40	150	0.65	157	0.50	932	rot	920	0.087	0.70	0.00	0.036	15
20	0.045	9,57	0.04	0.017	10.70	48 17	20.86	6.74	15.64	677	424	1.96	2.85	320	2.55	110	2.04	0.85	0.37	133	0.84	0 (5)	0.84	0.13	0.066	20
25	0.004	0.71	0.06	0.056		5"		9.0	22.65	10.24	526	7.49	324	1.00	3.65	157	2.65	128	0.56	1.74	0.61	0.22	1.17	0.190.	387	25
30	9.067	0.85	0.00	111135	0.66	0.03	0.012	1011	88.16	1435	831	10.50	4.65	4.00	5.40.	234	8.05	130	0.78	9.09	071	0.83	140	0.27	0.12	30
35	9.078	0.89	000	0.048	065	0.04	0.017	-		. 1	736	1597	6.05	5 (6)	7.10	3.0	3.57	2.40	1.04	2.44	0.95	0.41	1.64	656	0.16	35
40	0.089	114	0.14	0.060	074	0.05	0.002				841	17:30	7.75	0:40	920	5.98	4.0E	207	133	2.78	101	0.52	lss	0.46	0.20	40
45	0.100	1.28	0.17	D-074	0.84	0.00	0.004		5"		9.46	2:38	9.64	7.20	11.44	495	459	3.82	(8	3 (3	1.51	0.65	333	0.62	0.25	45
50	9.111	5.42	025	2001	0.95	0.07	0.000	0.00	0.03	0.013	10.02	27.06	35.71	8.00	1891	2002	\$10	4.04	TOI	5.45	1.88	0.75	230	0.70	9.86	50
60	0.134	1.70	0.28	0.18	315	0.10	0.042	0.79	0.04	0.017				100	1930	\$44	612	8.50	\$81	4.18	Q.57	6.0	10	0.08	0.45	60
70	0.156	1,00	0.28	0.16	130	0.14	0.061	09)	0.06	0.026			_				3.04	8.05	3.75	497	345	1.48	129	1.31	0.57	70
75	0.167	215	0.44	0.19	1.40	0.18	0.059	0.88	0.07	0.000							7.85	9.80	428	6.22	388	1.83	3.42	1.49	0.6	75
80	0.178	227	0.45	0.21	148	0.(8	£000	F-05	0.08	0.038							9 16	00	410	6-57	437	(8)	37¢	18	0/3	80
90	6.261	288	13.0	0.26	1.67	0.22	0.095	1.18	0.09	0.089							9.18	0.78	5.97	8.27	5.44	236	433	2.09	6.90	90
100	0.223	284	0.74	0.82	148	0.27	0.12	121	0.11	0.049							(0.20	1675	735	6.96	881	2.88	4.70	264	T to	100
125	5.279	8.85	1.18	0.40	230	0.60	D:10	1.84	0.47	0.074										1170	10.01	433	5.00	234	1.68	125
150	0.334	4.26	1.68	0.68	279	0.5%	0.26	6.95	0.04	0110									T	10.44	48.01	6.07	7.64	6.97	2.50	150
175	0.310	4.97.	210	0.91	8.26	075	0.38	290	0.92	0.18													100	7 15	8 10	175
200	0.448	5.88	2.69	1.16	3.72	0.98	0.42	282	0.41	0.15													9.39	916	3.96	200
250	8 457	3.10	407	1.76	4.68	1.48	918	3.20	0.62	0.28													1174	19.86	6.00	250
300	0.569	8.52	509	2.46	5.60	2.03	0.88	9,93	0.87	0.28																300
350	0.780	9.04.	7.68	9.29	8.69	2.70	1.17	4.69	1.10	0.50																350
400	0.391	f138	9.70	4.00	7.44	3.46	150	5.24	1.49	0.64																400
450	1.003				9.97	431	1.87	590	1.84	0.80															=1	450
500	1,114			-1	938	534	227	8.58	2.23	0.97															7	500
750		-				_		9.85	4.73	2.05															_	750
1000	2.223							13.11	1.06	349																1000

NOTE: Speace*recomments that From Velocities be maintained at or bylow 5 teet per functed in large diameter giping systems (i.e. 5" diameter and larger) to minimals the potential for hydraulic Shock. For additional information Friction loss data based on uniting mean wall dimensions to determine average ID, actual for may very.

 Sheet No.
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 Calc. No.
 Rev. No.

 By: MJT
 Date: 8/22/23

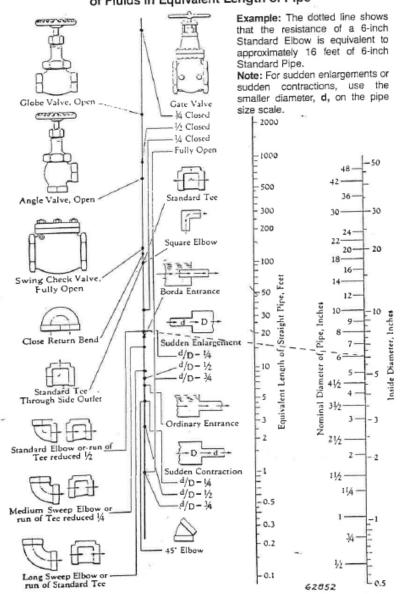
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 Chk'd: KRG
 Date: 8/24/23

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Client: Wisconsin Power & Light Subject: Leachate Loadout Facility Pump Design

FRICTION-WATER-PIPE FITTINGS

Friction of Water (Continued) Resistance of Valves and Fittings to Flow of Fluids in Equivalent Length of Pipe



From Crane Co. Technical Paper No. 409. Data based on the above chart are satisfactory for most applications; for more detailed data and information refer to pages 3-110 to page 3-120 which are based on Crane Co. Technical Paper No. 410. 3-12:

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 Calc. No.
 Rev. No.
 Date: 8/22/23

Date: 8/24/23

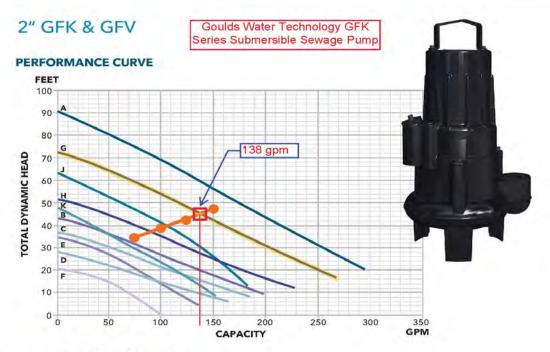
Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Client: Wisconsin Power & Light Subject: Leachate Loadout Facility Pump Design

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Chk'd: KRG



PRODUCT SPECIFICATIONS

GWT Part No.	Impeller Type	Curve	Voltage	HP	Rated Current	GWT Part No.	Impeller Type	Curve	Voltage	HP	Rated Current
2GFV1212F	Vortex	F	200	1.2	3.8	2GFK1712C	K	C	200	1.7	5.1
2GFV1213F	Vortex	F	230	1.2	4	2GFK1713C	K.	С	230	1.7	5
2GFV1214F	Vortex	F	460	1.2	2	2GFK1714C	K	С	460	1.7	2.5
2GFV1712E	Vortex	E	200	1.7	5.1	2GFK1712B	K.	В	200	1.7	5.1
2GFV1713E	Vortex	E	230	1.7	5	2GFK1713B	K	В	230	1.7	5
2GFV1714E	Vortex	E	460	1.7	2.5	2GFK1714B	K	В	460	1.7	2.5
2GFV3212K	Vortex	K	200	3.2	9.8	2GFK2412H	K	Н	200	2.4	7.7
2GFV3213K	Vortex	K	230	3.2	8.8	2GFK2413H	K	Н	230	2.4	7.2
2GFV3214K	Vortex	K	460	3.2	4.4	2GFK2414H	K	H	460	2.4	3.6
2GFK1212D	K	D	200	1.2	3.8	2GFK3212G	K	C.	500		9.8
2GFK1213D	К	D	230	1.2	4	2GFK3213G	K	C	230	3.2	8,8
2GFK1214D	K	D	460	1.2	-	2GFK3214G	K	G	460	3.2	4.4
2GFV3812J	Vortex	J	200	3.8	12	2GFK3812A	К	A	200	3.8	12
2GFV3813J	Vortex	J	230	3.8	10.2	2GFK3813A	K.	А	230	3.8	10.2
2GFV3814J	Vortex	1	460	3.8	5.1	2GFK3814A	K	A	460	3.8	5.1

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 Calc. No.
 1

 Rev. No.
 1

 By: MJT
 Date: 8/21/2023

Job No. 25222260.00 Job: Columbia Dry Ash Disposal Facility

Client: Wisconsin Power & Light Subject: Leachate Generated over 4 Days Chk'd: KRG Date: 8/24/23

Leachate Generated at the Site During Open Conditions

Purpose:

To determine the volume of leachate generated over 4 days during open conditions and determine if the storage capacity of the Leachate Storage Pond is sufficient to contain the leachate.

Approach:

Estimate the leachate generated over 4 days assuming a leachate generation rate of 6 inches per year.

Determine the volume of storage available in the Leachate/Surface Water Pond based on the existing contours.

Assumptions:

The site is fully constructed (Modules 1 - 6, and 10 - 13 are constructed)

There is final cover on a portion of Module 1, which does not generate leachate.

There is final cover on the west side of Modules 2 through 4, which does not generate leachate.

The remainder of the site is considered open conditions, with a leachate generation rate of 6 inches/year.

The Leachate/Surface Water Pond can contain 194,617 cubic feet of water. (1,455,735 gallons)

The Leachate/Surface Water Pond has 0.5 feet of water in the pond prior to the start of storage and the top elevation of the pond storage area is at elevation 796.97 ft.

Calculations:

Areas of open conditions:

Mod 1	2.62	acres	Mod 5	4.12	acres	Mod 11	3.48	acres
Mod 2	2.38	acres	Mod 6	3.79	acres	Mod 12	4.65	acres
Mod 3	2.41	acres	Mod 10	3.42	acres	Mod 13	2.48	acres
Mod 4	2.65	acres						

Total: 32.00 acres

Leachate Generated in 4 days:

32.00 acres X 6
$$\frac{\text{in}}{\text{yr}}$$
 X $\frac{1}{12}$ $\frac{\text{ft}}{\text{in}}$ X 43,560 $\frac{\text{ft}^2}{\text{acre}}$ X $\frac{1}{365}$ days

$$= 1,909 \quad \frac{\text{ft}^3}{\text{day}} \quad \text{X} \qquad 7.48 \quad \frac{\text{gal}}{\text{ft}^3} \quad = \quad 14,283 \quad \frac{\text{gallons}}{\text{day}} \quad \text{X} \quad \text{4} \quad \text{days}$$

= 57,132 gallons

Results: The Leachate/Surface Water Pond has adequate volume to contain 4 days of leachate generation.

SCS ENGINEE	RS	Sheet No.	1/1
	Calc. No.		
		Rev. No.	
Job No. 25220183.00	Job: Columbia Dry Ash Disposal Facility	By: KRG	Date: 2/17/22
Client: Wisconsin Power & Light	Subject: Hydraulic Capacity of LCS Pipe	Chk'd: MRS	Date: 4/8/22

Pipe Capacity Calculation

Purpose:

To confirm the leachate conveyance pipe is adequately sized for the flows expected.

Approach:

Estimate the amount of leachate that will be conveyed to the leachate collection pipe based on the leachate collection rate of 6 inches per year (NR 504.06(5)(j)1).

(Reference Leachate Sump Volume and Pump Capacity Calculations for this part of calculation.)

Use Manning's equation to estimate the maximum flow capacity of the pipe.

Assumptions:

Leachate flow from Modules 4, 5, and 6 flows through Module 11 to a collection sump.

From Leachate Sump Sizing and Pump Capacity Calculations, the estimated peak average leachate collection rate during active life **446.3** gal/acre/day.

The largest drainage basin area to a leachate collection pipe is approximately

16.39 acres

6" dia. leachate collection pipes are proposed

The inner diameter of 6" dia. HDPE SDR 11 pipe is

5.348 = 0.45 ft

Calculations:

Convert estimated peak average leachate flow to cfs:

446 gal/acre/day x 16.39 acres/[(

1,440 min/day x

60 sec/min x 7.48 gal/cf] =

0.111

0.01 cfs

Using Mannings equation, calculate pipe flow capacity: $Q_{max} = (1.49/n) \times A \times R^{2/3} \times S^{1/2}$

where:

 $Q_{max} = Maximum flow rate$

n = Mannings coefficient = 0.010 plastic pipe

 $A = Area of pipe = (\pi) \times D^2/4$

0.16 sf 6" dia. .pipe

R = Hydraulic radius = Area / Wetted perimeter = D/4 =

S = slope = 0.005 ft/ft (0.5%)

 $Q_{max} = (1.49/.01) \times (A) \times (R^{0.667}) \times (S^{0.5})$

 $Q_{max} = 0.38$ cfs

Results:

The flow capacity of the $\mathbf{6}^{\text{"}}$ dia. leachate collection pipe is

0.38 cfs. The worst-case flow rate is

0.01 cfs.

Therefore, the leachate collection pipe is adequately designed to accommodate the maximum flow rates expected to be generated at the site.

SCS	ENGINEERS	SHEET NO.		1	
Job No.	25222260.00	CALC. NO.			
Job:	Columbia Energy Center	REV. NO.		2	
Client	WPL	BY	SJL	DATE	8/28/23
Subject	Module 12/13 - Leachate/Surface Water Pond Evaluation	CHK'D.	RJG	DATE	8/30/23

Purpose:

The purpose of the leachate/surface water pond evaluation is to determine the following based on the as-built leachate/surface water pond top of liner elevation of 796.97 (see Background section below):

- The maximum amount of open area during each filling phase in order to maintain the peak water elevation resulting from the 25-year, 24-hour storm event at the maximum allowable 796.97.
- Based on the amount of allowable open area determined from the above, determine the maximum starting water elevations in the leachate/surface water pond to accommodate 1, 2, 5, and 10-year, 24-hour storm events without overtopping.

Background:

- During construction of Module 2, the top of the leachate/surface water pond liner was determined to be at elevation 796.97.
- Previous calculations submitted to the WDNR on January 30, 2018 and March 10, 2021, evaluated the leachate/surface water pond capacity based on the as-built pond liner elevation.
- A similar evaluation was performed for Module 3 and 4 construction and then Module 5 and 6 construction that produced a chart of maximum leachate/surface water pond starting elevations vs. rainfall storage capacity.
- Module 10 and 11 were constructed in 2002 and CCR placement began in 2023.
- Module 12 and 13 will be constructed if additional airspace is required before the plant is closed
- The Filling Phases 0 4 were previously submitted and approved by WDNR in 2022 as part
 of the Plan Modification Request Plan of Operations Update that covered modules up to
 Module 10-11.

Approach:

- Use the previously developed HydroCAD storm water model to model the below four filling scenarios.
 - 1. Filling Phase 5 Assumes portions of Module 11 and 12 are contributing to the leachate/surface water pond while material is placed from the pond closure and the plant. See **Figure 1** for filling grades and contributing area.
 - 2. Filling Phase 6 Assumes portions of Module 12 and 13 are contributing to the leachate/surface water pond while material is placed from the pond closure and the plant. See **Figure 2** for filling grades and contributing area.

Assumptions:

- CCR surfaces and intermediate cover areas were assumed to be impermeable (CN=98).
- The top of pond liner elevation is 796.97 (see Background section).
- Time of Concentration is 20 minutes for open areas.

Results:

1. Maximum allowable open area and contact water sump area during filling of Module 12 and 13 is 8.51 acres.

SCS	ENGINEERS	SHEET NO.		2	
Job No.	25222260.00	CALC. NO.			
Job:	Columbia Energy Center	REV. NO.		2	
Client	WPL	BY	SJL	DATE	8/28/23
Subject	Module 12/13 - Leachate/Surface Water Pond Evaluation	CHK'D.	RJG	DATE	8/30/23

2. Filling Phase 5:

- The contributing area of landfill to the leachate/surface water pond is 6.24 acres for the leachate/surface water pond to accommodate the runoff from a 25-year, 24-hour storm without overtopping. This acreage includes the open landfill area plus the contact water sump area.
- The remainder of landfill would need to be closed/covered with final or intermediate cover and routed away from the pond.
- **Figure 1** shows a proposed filling sequence, and **Figure 1a** shows the various operating levels of the leachate/surface water pond to accommodate the various storm events with the additional cover in place.

3. Filling Phase 6:

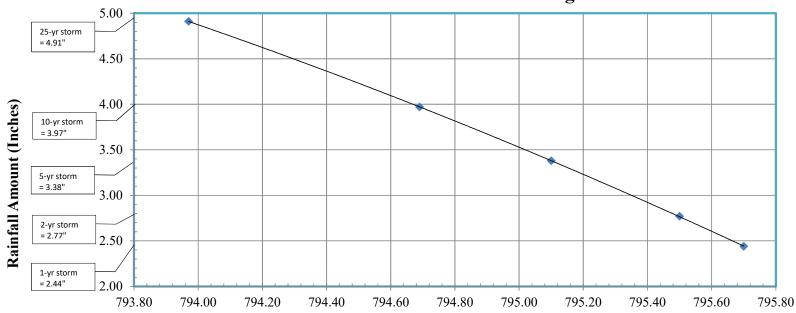
- The contributing area of landfill to the leachate/surface water pond is 8.475 acres for the leachate/surface water pond to accommodate the runoff from a 25-year, 24-hour storm without overtopping. This acreage includes the open landfill area plus the contact water sump area.
- The remainder of landfill would need to be closed/covered with final or intermediate cover and routed away from the pond.
- **Figure 2** shows a proposed filling sequence, and **Figure 2a** shows the various operating levels of the leachate/surface water pond to accommodate the various storm events with the additional cover in place.

The HydroCAD reports for the maximum open contributing area, each scenario modeled are attached.

I:\25222260.00\Data and Calculations\Leachate\230728_Leachate_Surface Water Pond Evaluation_P00 Calc writeup.docx



Figure 1A Columbia Energy Center Phase 5 Filling- Open Landfill Area Leachate/Surface Water Pond Maximum Starting Water Elevation



Leachate/Surface Water Pond Maximum Starting Water Elevation (ft)

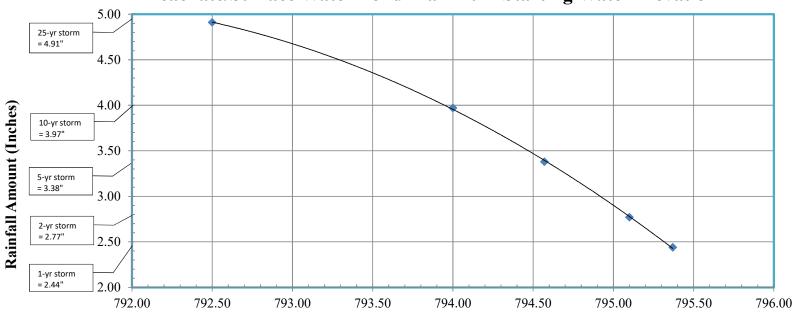
Notes/Assumptions:

- 1. Maximum starting water elevations based on 2011 Mod 2 as-built survey which determined the top of pond liner elevation = 796.97.
- 2. Maximum starting water elevation assumes no freeboard.
- 3. Previously developed HydroCAD model utilized with curve number for intermediate cover areas and ash surfaces assumed at CN = 98.
- 4. HydroCAD model assumes drainage areas contributing to pond include (Figure 1):
 - Landfill open area plus contact water sump area = 6.24 acres.
 - Leachate/Surface Water Pond Area, 2.98 acres.
- 5. Maximum open area per HydroCAD model during filling is 8.51 acres.

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Figure 2A
Columbia Energy Center
Phase 6 Filling- Open Landfill Area
Leachate/Surface Water Pond Maximum Starting Water Elevation



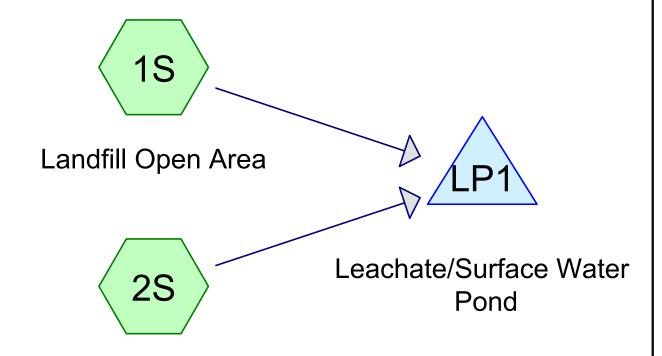
Leachate/Surface Water Pond Maximum Starting Water Elevation (ft)

Notes/Assumptions:

- 1. Maximum starting water elevations based on 2011 Mod 2 as-built survey which determined the top of pond liner elevation = 796.97.
- 2. Maximum starting water elevation assumes no freeboard.
- 3. Previously developed HydroCAD model utilized with curve number for intermediate cover areas and ash surfaces assumed at CN = 98.
- 4. HydroCAD model assumes drainage areas contributing to pond include (Figure 2):
 - Landfill open area plus contact water sump area = 8.475 acres.
 - Leachate/Surface Water Pond Area, 2.98 acres.
- 5. Maximum open area per HydroCAD model during filling is 8.51 acres.

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Maximum Open Area



Leachate/Surface Water Pond

Phase 12 Filling

Phase 13 Filling









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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	25-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	4.91	2

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91" Printed 8/17/2023

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: Landfill Open Area Runoff Area=8.510 ac 100.00% Impervious Runoff Depth=4.67"

Tc=20.0 min CN=98 Runoff=34.73 cfs 3.314 af

Subcatchment2S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=4.67"

Tc=0.0 min CN=98 Runoff=21.19 cfs 1.161 af

Pond LP1: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,946 cf Inflow=39.03 cfs 4.475 af

Outflow=0.00 cfs 0.000 af

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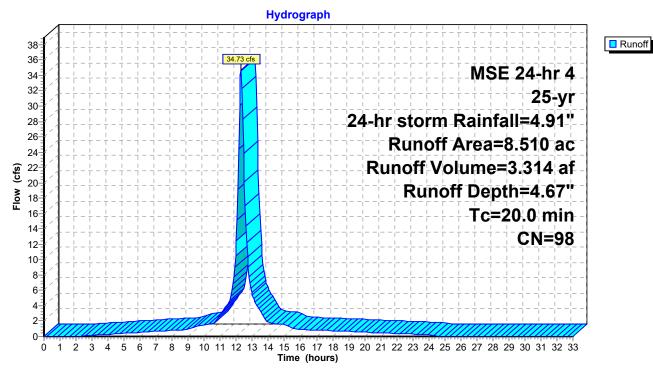
Summary for Subcatchment 1S: Landfill Open Area

3.314 af, Depth= 4.67" Runoff 34.73 cfs @ 12.28 hrs, Volume= Routed to Pond LP1: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

_	Area	(ac)	CN	Desc	cription					
*	8.	510	98	Mod	od 2 - 11 Open Area					
	8.510 100.00% Impervious Area						a ·			
		Leng		•	•		Description			
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)				
	20.0						Direct Entry, Estimated			

Subcatchment 1S: Landfill Open Area



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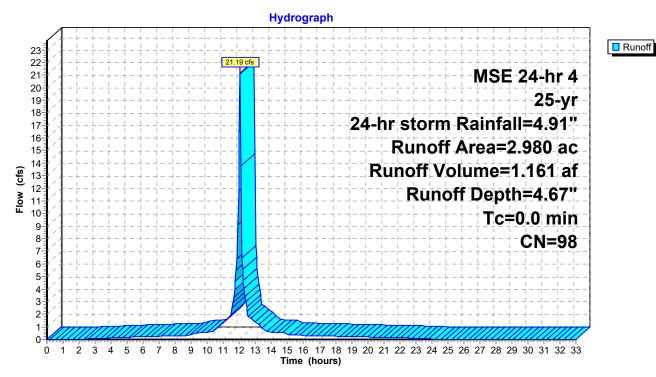
Summary for Subcatchment 2S: Leachate/Surface Water Pond

1.161 af, Depth= 4.67" Runoff 21.19 cfs @ 12.04 hrs, Volume= Routed to Pond LP1: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

	Area	(ac)	CN	Desc	cription						
*	2.	980	98	Lead	achate Surface Water Pond						
	2.	2.980 100.00% Impervious Area									
	Тс	Leng	th	Slope	Velocity	Capacity	Description				
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	0.0						Direct Entry,				

Subcatchment 2S: Leachate/Surface Water Pond



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Summary for Pond LP1: Leachate/Surface Water Pond

Inflow Area = 11.490 ac,100.00% Impervious, Inflow Depth = 4.67" for 25-yr, 24-hr storm event

Inflow 39.03 cfs @ 12.27 hrs, Volume= 4.475 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

Starting Elev= 792.50' Surf.Area= 11,070 sf Storage= 3,030 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,611 sf Storage= 197,946 cf (194,915 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (194,617 cf above start)

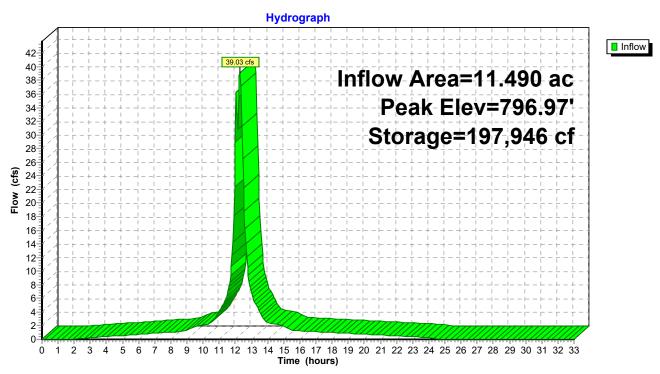
Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

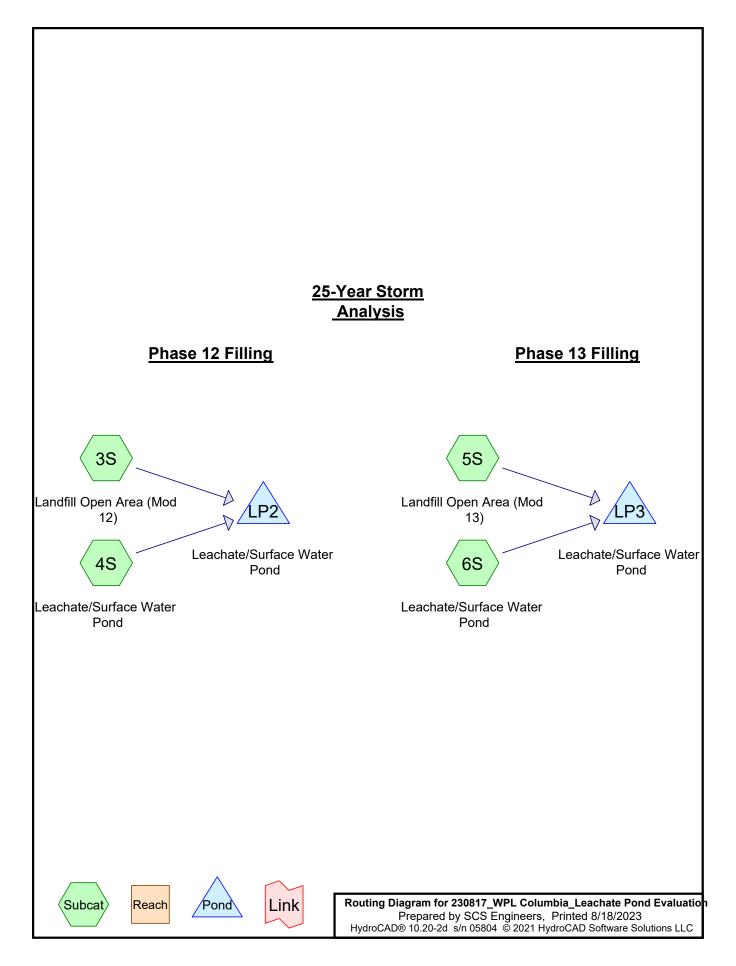
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage	Storage Description				
#1	792.00'	00' 405,390 cf C		Custom Stage Data (Prismatic)Listed below (Recalc)				
Elevation (feet)			c.Store	Cum.Store				

Elevation	Sun Area	inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
792.00	1,051	0	0
794.00	41,126	42,177	42,177
796.00	56,885	98,011	140,188
798.00	66,581	123,466	263,654
800.00	75,155	141,736	405,390

Pond LP1: Leachate/Surface Water Pond





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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
 1	25-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	4.91	2

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment3S: Landfill Open Area Runoff Area=6.240 ac 100.00% Impervious Runoff Depth=4.67"

Tc=20.0 min CN=98 Runoff=25.47 cfs 2.430 af

Subcatchment4S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=4.67"

Tc=0.0 min CN=98 Runoff=21.19 cfs 1.161 af

Subcatchment5S: Landfill Open Area Runoff Area=8.475 ac 100.00% Impervious Runoff Depth=4.67"

Tc=20.0 min CN=98 Runoff=34.59 cfs 3.301 af

Subcatchment6S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=4.67"

Tc=0.0 min CN=98 Runoff=21.19 cfs 1.161 af

Pond LP2: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,360 cf Inflow=32.30 cfs 3.591 af

Outflow=0.00 cfs 0.000 af

Pond LP3: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,353 cf Inflow=38.89 cfs 4.461 af

Outflow=0.00 cfs 0.000 af

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

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Summary for Subcatchment 3S: Landfill Open Area (Mod 12)

Runoff = 25.47 cfs @ 12.28 hrs, Volume=

2.430 af, Depth= 4.67"

Routed to Pond LP2: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

_	Area	(ac)	CN	Desc	cription		
*	6.	240	98	Mod	12 Open /	Area	
	6.240 100.00% Impervious Area						1
	Тс	Leng	th	Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	20.0						Direct Entry, Estimated

Summary for Subcatchment 4S: Leachate/Surface Water Pond

Runoff = 21.19 cfs @ 12.04 hrs, Volume=

1.161 af, Depth= 4.67"

Routed to Pond LP2: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

	Area	(ac)	CN	Desc	cription									
*	2.	980	98	Lead	eachate Surface Water Pond									
	2.980 100.00% Impervious Area													
	Тс	Leng	th	Slope	Velocity	Capacity	Description							
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)								
	0.0						Direct Entry,							

Summary for Subcatchment 5S: Landfill Open Area (Mod 13)

Runoff = 34.59 cfs @ 12.28 hrs, Volume= 3.301 af, Depth= 4.67" Routed to Pond LP3 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

_	Area	(ac)	CN	Desc	cription		
*	8.	475	98	Mod	13 Open A	Area	
	8.	475	a				
	Тс	Lengt	h S	Slope	Velocity	Capacity	Description
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)	
	20.0	•	•				Direct Entry, Estimated

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Summary for Subcatchment 6S: Leachate/Surface Water Pond

Runoff 21.19 cfs @ 12.04 hrs, Volume= 1.161 af. Depth= 4.67"

Routed to Pond LP3: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

_	Area	(ac)	CN	Desc	cription								
*	2.	980	98	Lead	Leachate Surface Water Pond								
	2.980 100.00% Impervious Area												
		Leng		Slope	,	. ,	Description						
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)							
	0.0						Direct Entry,						

Summary for Pond LP2: Leachate/Surface Water Pond

Inflow Area = 9.220 ac,100.00% Impervious, Inflow Depth = 4.67" for 25-yr, 24-hr storm event

32.30 cfs @ 12.06 hrs, Volume= Inflow 3.591 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

Starting Elev= 793.97' Surf.Area= 40,525 sf Storage= 40,952 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,565 sf Storage= 197,360 cf (156,408 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (156,695 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage		Storage	Storage Description				
#1	792.00'	405	,390 cf	Custor	n Stage Data (Pr	ismatic)Listed below (Recalc)			
Elevation (feet)		f.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)				
792.00		1,051		0	0				
794.00	4	1,126	4	42,177	42,177				
796.00	5	6,885	(98,011	140,188				
798.00	6	6,581	12	23,466	263,654				
800.00	75,155		14	41,736	405,390				

Summary for Pond LP3: Leachate/Surface Water Pond

11.455 ac,100.00% Impervious, Inflow Depth = 4.67" for 25-yr, 24-hr storm event Inflow Area =

38.89 cfs @ 12.27 hrs, Volume= Inflow 4.461 af

0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min Outflow 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 25-yr, 24-hr storm Rainfall=4.91"

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798.00

800.00

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Starting Elev= 792.50' Surf.Area= 11,070 sf Storage= 3,030 cf Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,564 sf Storage= 197,353 cf (194,323 cf above start) Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (194,617 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

123,466

141,736

Center-of-Mass det. time= (not calculated: no outflow)

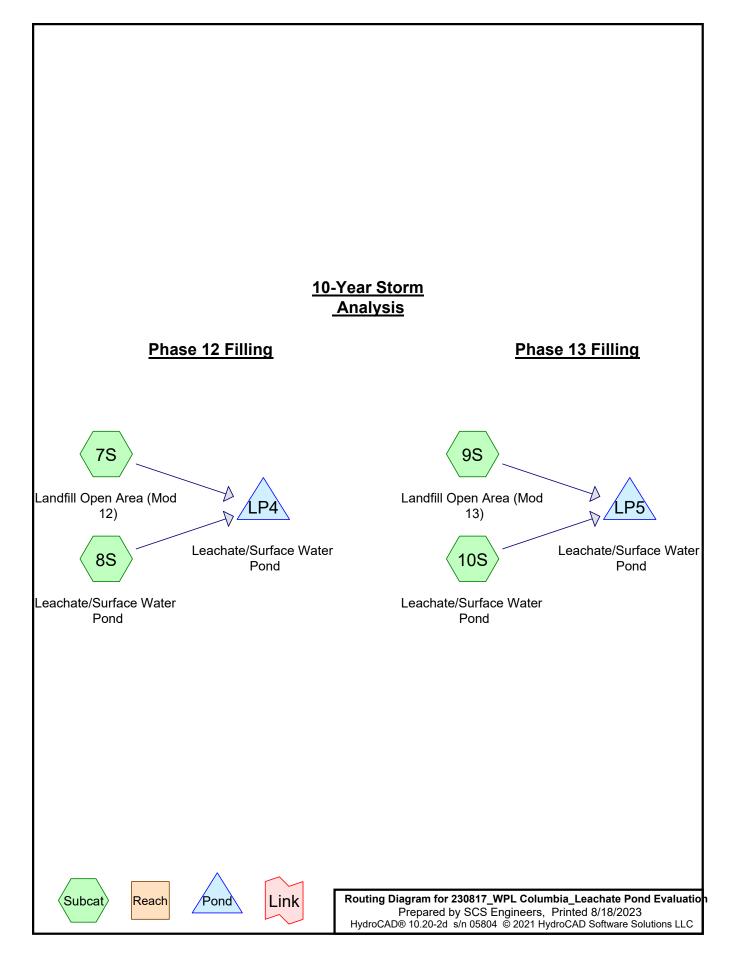
66,581

75,155

Volume	Invert	Avail.Storag	e Storage	Description		
#1	792.00' 405,390 cf		cf Custom	Custom Stage Data (Prismatic)Listed below (Re		
Elevation (feet)			Inc.Store ubic-feet)	Cum.Store (cubic-feet)		
792.00	1	1,051	0	0		
794.00	41	1,126	42,177	42,177		
796.00	56	5,885	98,011	140,188		

263,654

405,390



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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	10-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	3.97	2

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment7S: Landfill Open Area Runoff Area=6.240 ac 100.00% Impervious Runoff Depth=3.74"

Tc=20.0 min CN=98 Runoff=20.53 cfs 1.942 af

Subcatchment8S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=3.74"

Tc=0.0 min CN=98 Runoff=17.09 cfs 0.928 af

Subcatchment9S: Landfill Open Area Runoff Area=8.475 ac 100.00% Impervious Runoff Depth=3.74"

Tc=20.0 min CN=98 Runoff=27.88 cfs 2.638 af

Subcatchment10S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=3.74"

Tc=0.0 min CN=98 Runoff=17.09 cfs 0.928 af

Pond LP4: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,440 cf Inflow=26.02 cfs 2.870 af

Outflow=0.00 cfs 0.000 af

Pond LP5: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,491 cf Inflow=31.34 cfs 3.566 af

Outflow=0.00 cfs 0.000 af

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

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Summary for Subcatchment 7S: Landfill Open Area (Mod 12)

Runoff = 20.53 cfs @ 12.29 hrs, Volume=

1.942 af. Depth= 3.74"

Routed to Pond LP4: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

_	Area	(ac)	CN	Desc	cription			
*	6.	240	98	Mod	12 Open /	Area		
6.240 100.00% Impervious Area								
	Тс	Leng	th	Slope	Velocity	Capacity	Description	
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	20.0						Direct Entry, Estimated	

Summary for Subcatchment 8S: Leachate/Surface Water Pond

Runoff = 17.09 cfs @ 12.04 hrs, Volume=

0.928 af, Depth= 3.74"

Routed to Pond LP4: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

_	Area	(ac)	CN	Desc	cription									
*	2.	980	98	Lead	eachate Surface Water Pond									
2.980 100.00% Impervious Area														
	Тс	Leng	th :	Slope	Velocity	Capacity	Description							
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)								
	0.0						Direct Entry,							

Summary for Subcatchment 9S: Landfill Open Area (Mod 13)

Runoff = 27.88 cfs @ 12.29 hrs, Volume= 2.638 af, Depth= 3.74" Routed to Pond LP5 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

	Area	(ac)	CN	Desc	cription			
*	8.	475	98	Mod	13 Open A	Area		
8.475 100.00% Impervious Area								
	Tc (min)	Length (feet)		Slope (ft/ft)	,	Capacity (cfs)	Description	
_	20.0	(100	ι)	(IVIL)	(10360)	(015)	Direct Entry, Estimated	

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Summary for Subcatchment 10S: Leachate/Surface Water Pond

Runoff 17.09 cfs @ 12.04 hrs, Volume= 0.928 af. Depth= 3.74"

Routed to Pond LP5: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

	Area	(ac)	CN	N Description										
*	2.	980	98	Lead	eachate Surface Water Pond									
	2.980 100.00% Impervious Area													
		Leng		•	•		Description							
_	(min) (feet) (ft/ft) (ft/sec) (cfs)													
	0.0						Direct Entry,							

Summary for Pond LP4: Leachate/Surface Water Pond

Inflow Area = 9.220 ac,100.00% Impervious, Inflow Depth = 3.74" for 10-yr, 24-hr storm event

26.02 cfs @ 12.06 hrs, Volume= Inflow 2.870 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs Starting Elev= 794.69' Surf.Area= 46,563 sf Storage= 72,430 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,571 sf Storage= 197,440 cf (125,010 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (125,218 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage		Storage	Storage Description				
#1	792.00'	405	,390 cf	Custor	n Stage Data (Pr	ismatic)Listed below (Recalc)			
Elevation (feet)		f.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)				
792.00		1,051		0	0				
794.00	4	1,126	4	42,177	42,177				
796.00	5	6,885	(98,011	140,188				
798.00	6	6,581	12	23,466	263,654				
800.00	75,155		141,736		405,390				

Summary for Pond LP5: Leachate/Surface Water Pond

11.455 ac,100.00% Impervious, Inflow Depth = 3.74" for 10-yr, 24-hr storm event Inflow Area =

Inflow 31.34 cfs @ 12.27 hrs, Volume= 3.566 af

0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min Outflow 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

230817_WPL Columbia_Leachate Pond EvaluMSE 24-hr 4 10-yr, 24-hr storm Rainfall=3.97"

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796.00

798.00

800.00

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Starting Elev= 794.00' Surf.Area= 41,126 sf Storage= 42,177 cf
Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,575 sf Storage= 197,491 cf (155,314 cf above start)
Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (155,470 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

98,011

123,466

141,736

Center-of-Mass det. time= (not calculated: no outflow)

56,885

66,581

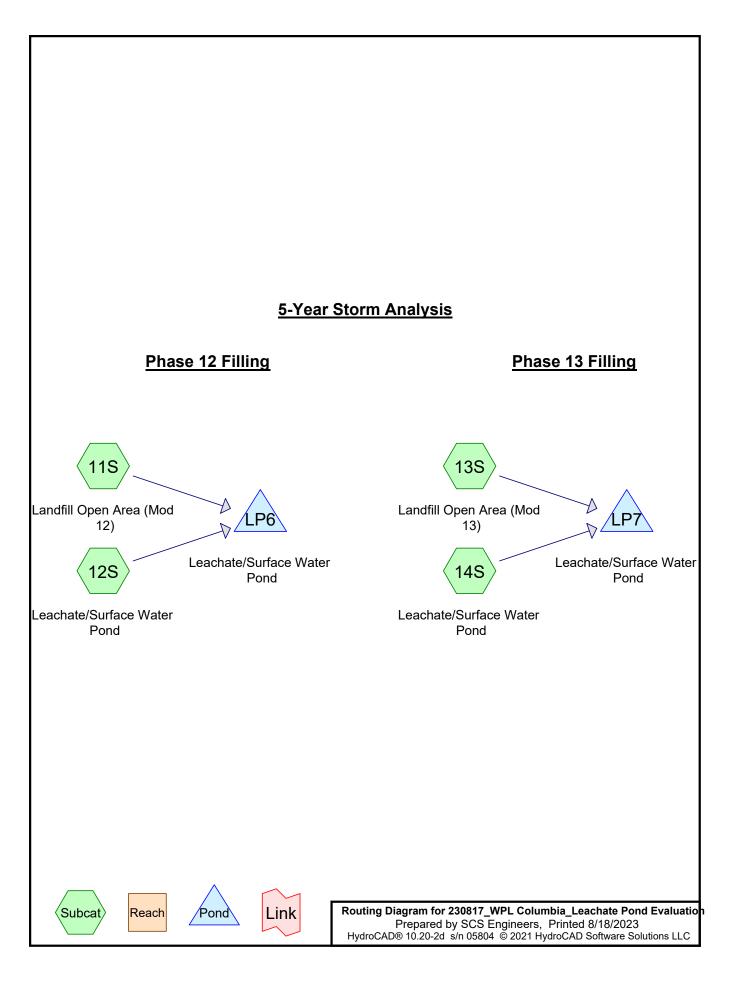
75,155

Volume	Invert	Avail.S	Avail.Storage		Storage Description				
#1	792.00'	405,	390 cf	Custon	n Stage Data (P	rismatic)Listed below (Recalc)			
Elevation				.Store	Cum.Store				
(feet) 792.00	00 1,051		(cubic-feet) 0		(cubic-feet) 0				
794.00			4	2,177	42,177				

140,188

263,654

405,390



230817_WPL Columbia_Leachate Pond Evaluation Prepared by SCS Engineers HydroCAD® 10.20-2d s/n 05804 © 2021 HydroCAD Software Solutions LLC

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Rainfall Events Listing (selected events)

Eve	nt#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
		Name				(hours)		(inches)	
	1	5-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	3.38	2

230817_WPL Columbia_Leachate Pond Evalual ISE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment11S: Landfill Open Area	Runoff Area=6.240 ac	100.00% Impervious	Runoff Depth=3.15"

Tc=20.0 min CN=98 Runoff=17.42 cfs 1.636 af

Subcatchment12S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=3.15"

Tc=0.0 min CN=98 Runoff=14.51 cfs 0.781 af

Subcatchment13S: Landfill Open Area Runoff Area=8.475 ac 100.00% Impervious Runoff Depth=3.15"

Tc=20.0 min CN=98 Runoff=23.66 cfs 2.222 af

Subcatchment14S: Leachate/Surface Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=3.15"

Tc=0.0 min CN=98 Runoff=14.51 cfs 0.781 af

Pond LP6: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,502 cf Inflow=22.07 cfs 2.418 af

Outflow=0.00 cfs 0.000 af

Pond LP7: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,748 cf Inflow=26.60 cfs 3.004 af

Outflow=0.00 cfs 0.000 af

230817_WPL Columbia_Leachate Pond Evaluat/ISE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

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Summary for Subcatchment 11S: Landfill Open Area (Mod 12)

Runoff = 17.42 cfs @ 12.29 hrs, Volume=

1.636 af, Depth= 3.15"

Routed to Pond LP6: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

_	Area	(ac)	CN	Desc	cription		
*	6.	240	98	Mod	12 Open /	Area	
	6.	1					
	Тс	Leng	th	Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	20.0						Direct Entry, Estimated

Summary for Subcatchment 12S: Leachate/Surface Water Pond

Runoff = 14.51 cfs @ 12.04 hrs, Volume=

0.781 af, Depth= 3.15"

Routed to Pond LP6: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

	Area	(ac)	CN	Desc	cription							
*	2.	980	98	Lead	eachate Surface Water Pond							
	2.	980		100.	00% Impe	rvious Area	a					
	Тс	Leng	th	Slope	Velocity	Capacity	Description					
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
	0.0						Direct Entry,					

Summary for Subcatchment 13S: Landfill Open Area (Mod 13)

Runoff = 23.66 cfs @ 12.29 hrs, Volume= 2.222 af, Depth= 3.15" Routed to Pond LP7 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

	Area	(ac)	CN	Desc	cription				
*	8.	475	98	Mod	Mod 13 Open Area				
	8.	1							
	Тс	Leng	th :	Slope	Velocity	Capacity	Description		
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)			
	20.0						Direct Entry, Estimated		

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Summary for Subcatchment 14S: Leachate/Surface Water Pond

Runoff 14.51 cfs @ 12.04 hrs, Volume=

0.781 af. Depth= 3.15"

Routed to Pond LP7: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

_	Area	(ac)	CN	Desc	Description								
*	2.	980	98	Lead	eachate Surface Water Pond								
	2.980 100.00% Impervious Area												
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
_	0.0	(100	, c)	(1011)	(10300)	(013)	Direct Entry,						

Summary for Pond LP6: Leachate/Surface Water Pond

Inflow Area = 9.220 ac,100.00% Impervious, Inflow Depth = 3.15" for 5-yr, 24-hr storm event

22.07 cfs @ 12.06 hrs, Volume= Inflow 2.418 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs Starting Elev= 795.10' Surf.Area= 49,793 sf Storage= 92,183 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,576 sf Storage= 197,502 cf (105,320 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (105,464 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.S	torage	Storage	e Description	
#1	792.00'	405	,390 cf	Custor	n Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)		f.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
792.00		1,051		0	0	
794.00	4	1,126	4	42,177	42,177	
796.00	5	6,885	(98,011	140,188	
798.00	6	6,581	12	23,466	263,654	
800.00	7	5,155	14	41,736	405,390	

Summary for Pond LP7: Leachate/Surface Water Pond

11.455 ac,100.00% Impervious, Inflow Depth = 3.15" for 5-yr, 24-hr storm event Inflow Area =

Inflow 26.60 cfs @ 12.27 hrs, Volume= 3.004 af

0.00 hrs, Volume= Outflow 0.00 cfs @ 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

230817_WPL Columbia_Leachate Pond Evalual SE 24-hr 4 5-yr, 24-hr storm Rainfall=3.38"

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794.00

796.00

798.00

800.00

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Starting Elev= 794.57' Surf.Area= 45,617 sf Storage= 66,899 cf Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,596 sf Storage= 197,748 cf (130,850 cf above start) Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (130,748 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

42,177

98,011

123,466

141,736

Center-of-Mass det. time= (not calculated: no outflow)

41,126

56,885

66,581

75,155

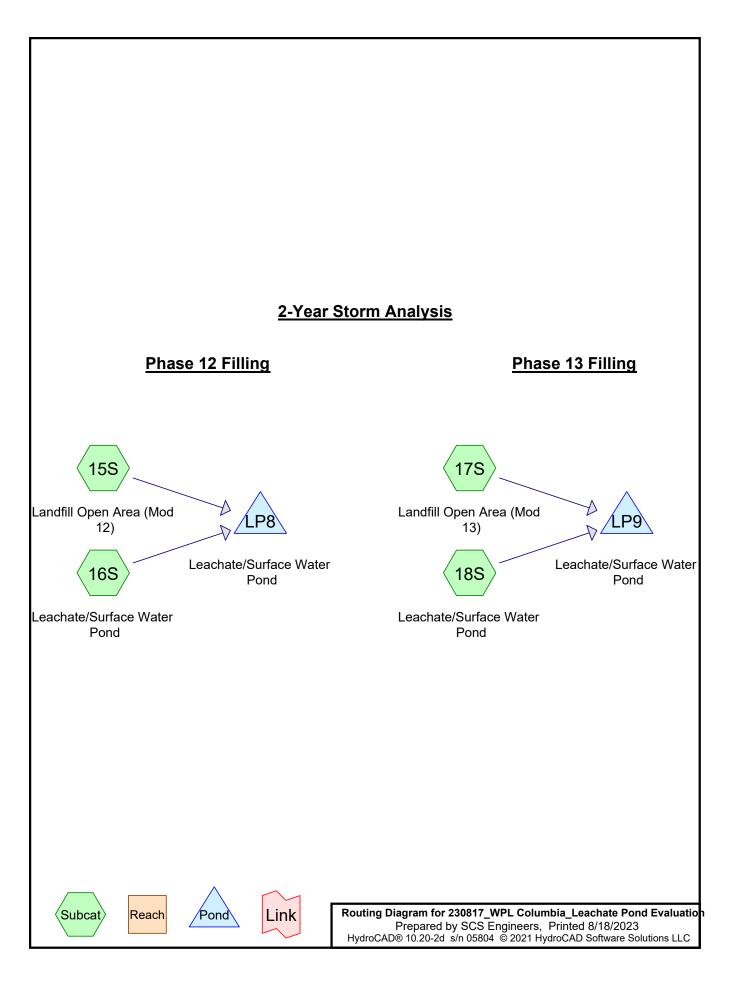
Volume	Invert	Avail.Storage	Storage	Description				
#1	792.00'	405,390 cf	Custom	Custom Stage Data (Prismatic)Listed below (Recalc)				
Elevation (feet)			c.Store ic-feet)	Cum.Store (cubic-feet)				
792.00		1,051	0	0				

42.177

140,188

263,654

405,390



230817_WPL Columbia_Leachate Pond Evaluation
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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	2.77	2

230817 WPL Columbia Leachate Pond Evalual SE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77" Printed 8/18/2023

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment15S: Landfill Open Area Runoff Area=6.240 ac 100.00% Impervious Runoff Depth=2.54"

Tc=20.0 min CN=98 Runoff=14.19 cfs 1.320 af

Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=2.54" Subcatchment16S: Leachate/Surface

Tc=0.0 min CN=98 Runoff=11.83 cfs 0.631 af

Runoff Area=8.475 ac 100.00% Impervious Runoff Depth=2.54" Subcatchment17S: Landfill Open Area

Tc=20.0 min CN=98 Runoff=19.28 cfs 1.793 af

Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=2.54" Subcatchment 18S: Leachate/Surface

Tc=0.0 min CN=98 Runoff=11.83 cfs 0.631 af

Pond LP8: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,715 cf Inflow=17.97 cfs 1.951 af

Outflow=0.00 cfs 0.000 af

Pond LP9: Leachate/SurfaceWater Pond Peak Elev=796.97' Storage=197,769 cf Inflow=21.68 cfs 2.424 af

Outflow=0.00 cfs 0.000 af

230817_WPL Columbia_Leachate Pond Evalual SE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

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Summary for Subcatchment 15S: Landfill Open Area (Mod 12)

Runoff = 14.19 cfs @ 12.29 hrs, Volume=

1.320 af, Depth= 2.54"

Routed to Pond LP8: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

Area (ac) CN Description									
*	6.	240	98	Mod	12 Open /	Area			
	6.240 100.00% Impervious Area								
		Leng		•	•		Description		
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)			
	20.0						Direct Entry, Estimated		

Summary for Subcatchment 16S: Leachate/Surface Water Pond

Runoff = 11.83 cfs @ 12.04 hrs, Volume=

0.631 af, Depth= 2.54"

Routed to Pond LP8: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

	Area	(ac)	CN	Desc	cription							
*	2.	980	98	Lead	eachate Surface Water Pond							
	2.	980		100.	00% Impe	rvious Area	a					
	Тс	Leng	th	Slope	Velocity	Capacity	Description					
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
	0.0						Direct Entry,					

Summary for Subcatchment 17S: Landfill Open Area (Mod 13)

Runoff = 19.28 cfs @ 12.29 hrs, Volume= 1.793 af, Depth= 2.54" Routed to Pond LP9 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

	Area	(ac)	CN	Desc	cription				
*	8.	475	98	Mod	lod 13 Open Area				
	8.	475		100.	00% Impe	a e e e e e e e e e e e e e e e e e e e			
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	20.0	·					Direct Entry, Estimated		

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Summary for Subcatchment 18S: Leachate/Surface Water Pond

Runoff 11.83 cfs @ 12.04 hrs, Volume= 0.631 af. Depth= 2.54"

Routed to Pond LP9: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

_	Area	(ac)	CN	Desc	cription							
*	2.	980	98	Lead	eachate Surface Water Pond							
	2.980 100.00% Impervious Area											
		Leng		Slope	,	. ,	Description					
	(min)) (feet) (ft/ft) (ft/sec) (cfs)										
	0.0						Direct Entry,					

Summary for Pond LP8: Leachate/Surface Water Pond

Inflow Area = 9.220 ac,100.00% Impervious, Inflow Depth = 2.54" for 2-yr, 24-hr storm event

17.97 cfs @ 12.06 hrs, Volume= Inflow 1.951 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs Starting Elev= 795.50' Surf.Area= 52,945 sf Storage= 112,730 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,593 sf Storage= 197,715 cf (84,985 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (84,917 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.S	torage	Storage	e Description	
#1	792.00'	405	,390 cf	Custor	n Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)		f.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
792.00		1,051		0	0	
794.00	4	1,126	4	42,177	42,177	
796.00	5	6,885	(98,011	140,188	
798.00	6	6,581	12	23,466	263,654	
800.00	7	5,155	14	41,736	405,390	

Summary for Pond LP9: Leachate/Surface Water Pond

11.455 ac,100.00% Impervious, Inflow Depth = 2.54" for 2-yr, 24-hr storm event Inflow Area =

Inflow 21.68 cfs @ 12.27 hrs, Volume= 2.424 af

0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min Outflow 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

230817 WPL Columbia Leachate Pond Evalual/ISE 24-hr 4 2-yr, 24-hr storm Rainfall=2.77"

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794.00

796.00

798.00

800.00

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Starting Elev= 795.10' Surf.Area= 49,793 sf Storage= 92,183 cf Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,597 sf Storage= 197,769 cf (105,586 cf above start) Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (105,464 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

41,126

56,885

66,581

75,155

Volume	Invert	Avail.Storage	e Storage	Storage Description			
#1	792.00'	405,390 c	f Custom	Custom Stage Data (Prismatic)Listed below (Recalc)			
Elevation (feet)			nc.Store bic-feet)	Cum.Store (cubic-feet)			
792.00		1,051	0	0			

42,177

140,188

263,654

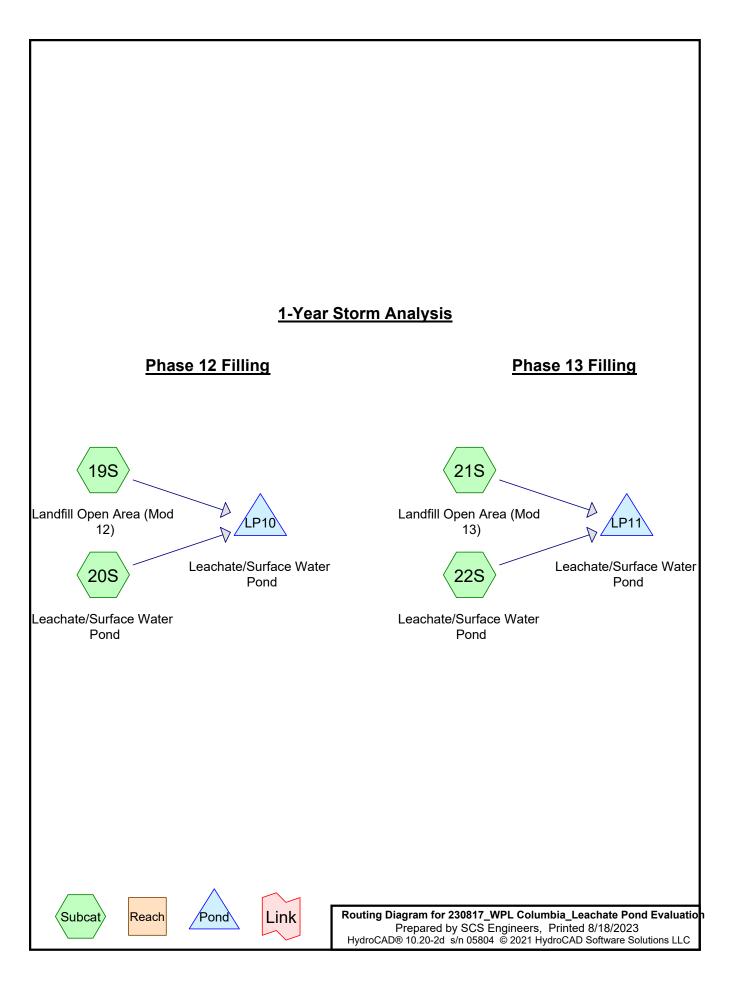
405,390

42,177

98,011

123,466

141,736



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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMO	2
	Name				(hours)		(inches)		
1	1-yr, 24-hr storm	MSE 24-hr	4	Default	24.00	1	2.44	2	_

230817_WPL Columbia_Leachate Pond Evaluat/ISE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

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Outflow=0.00 cfs 0.000 af

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Time span=0.00-33.00 hrs, dt=0.05 hrs, 661 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 19S: Landfill Open Area	Runoff Area=6.240 ac 100.00% Impervious Runoff Depth=2.21" Tc=20.0 min CN=98 Runoff=12.44 cfs 1.150 af
Subcatchment20S: Leachate/Surface	Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=2.21" Tc=0.0 min CN=98 Runoff=10.38 cfs 0.549 af
Subcatchment21S: Landfill Open Area	Runoff Area=8.475 ac 100.00% Impervious Runoff Depth=2.21" Tc=20.0 min CN=98 Runoff=16.90 cfs 1.562 af
Subcatchment22S: Leachate/Surface	Runoff Area=2.980 ac 100.00% Impervious Runoff Depth=2.21" Tc=0.0 min CN=98 Runoff=10.38 cfs 0.549 af
Pond LP10: Leachate/SurfaceWater	Peak Elev=796.97' Storage=197,478 cf Inflow=15.75 cfs 1.699 af Outflow=0.00 cfs 0.000 af
Pond LP11: Leachate/SurfaceWater	Peak Elev=796.97' Storage=197,853 cf Inflow=19.01 cfs 2.111 af

230817_WPL Columbia_Leachate Pond Evalual SE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

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Summary for Subcatchment 19S: Landfill Open Area (Mod 12)

Runoff = 12.44 cfs @ 12.29 hrs, Volume= 1.150 af, Depth= 2.21"

Routed to Pond LP10: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

	Area	(ac)	CN	Desc	cription		
*	* 6.240 98 Mod 12 Open Area						
	6.240 100.00% Impervious Area						
Tc Length Slope Velocity Capacity				•	•		Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	20.0						Direct Entry, Estimated

Summary for Subcatchment 20S: Leachate/Surface Water Pond

Runoff = 10.38 cfs @ 12.04 hrs, Volume= 0.549 af, Depth= 2.21" Routed to Pond LP10 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

	Area	(ac)	CN	Desc	cription				
*	2.	980	98	B Leachate Surface Water Pond					
	2.980 100.00% Impervious Area								
	Тс	Leng	th	Slope	Velocity	Capacity	Description		
	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)			
	0.0						Direct Entry,		

Summary for Subcatchment 21S: Landfill Open Area (Mod 13)

Runoff = 16.90 cfs @ 12.29 hrs, Volume= 1.562 af, Depth= 2.21" Routed to Pond LP11 : Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

	Area	(ac)	CN	Desc	cription		
*	8.	475	98	Mod	13 Open A	Area	
	8.	475		100.	00% Impe	rvious Area	a e e e e e e e e e e e e e e e e e e e
	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	20.0	·					Direct Entry, Estimated

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Summary for Subcatchment 22S: Leachate/Surface Water Pond

Runoff 10.38 cfs @ 12.04 hrs, Volume= 0.549 af. Depth= 2.21"

Routed to Pond LP11: Leachate/Surface Water Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs MSE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

_	Area	(ac)	CN	Desc	cription					
*	2.	980	98	Lead	Leachate Surface Water Pond					
	2.980 100.00% Impervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					Description					
_	0.0	(100	, c)	(1011)	(10300)	(013)	Direct Entry,			

Summary for Pond LP10: Leachate/Surface Water Pond

Inflow Area = 9.220 ac,100.00% Impervious, Inflow Depth = 2.21" for 1-yr, 24-hr storm event

15.75 cfs @ 12.06 hrs, Volume= Inflow 1.699 af

Outflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs Starting Elev= 795.70' Surf.Area= 54,521 sf Storage= 123,477 cf

Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,574 sf Storage= 197,478 cf (74,001 cf above start)

Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (74,170 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.S	torage	Storage	e Description	
#1	792.00'	405	,390 cf	Custon	n Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)		f.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
792.00		1,051		0	0	
794.00	4	1,126	4	42,177	42,177	
796.00	5	6,885	(98,011	140,188	
798.00	6	6,581	12	23,466	263,654	
800.00	7	75,155	14	41,736	405,390	

Summary for Pond LP11: Leachate/Surface Water Pond

11.455 ac,100.00% Impervious, Inflow Depth = 2.21" for 1-yr, 24-hr storm event Inflow Area =

19.01 cfs @ 12.27 hrs, Volume= 2.111 af Inflow

0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min Outflow 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-33.00 hrs, dt= 0.05 hrs

230817_WPL Columbia_Leachate Pond Evaluated SE 24-hr 4 1-yr, 24-hr storm Rainfall=2.44"

Prepared by SCS Engineers

Printed 8/18/2023

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Starting Elev= 795.37' Surf.Area= 51,921 sf Storage= 105,914 cf Peak Elev= 796.97' @ 25.15 hrs Surf.Area= 61,604 sf Storage= 197,853 cf (91,939 cf above start) Flood Elev= 796.97' Surf.Area= 61,588 sf Storage= 197,647 cf (91,733 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description	
#1	792.00'	405,390 cf	Custom Stage Data (Prismatic)Listed below (Recalc)	
Elevation	Surf.	Area Inc	c.Store Cum.Store	

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
792.00	1,051	0	0
794.00	41,126	42,177	42,177
796.00	56,885	98,011	140,188
798.00	66,581	123,466	263,654
800.00	75,155	141,736	405,390

E.5 **Volume Calculations**

Client: Wisconsin Power and Light Co

Job No. 25222260.00

Sheet No.	1 of 1
Calc. No.	_
Rev. No.	
By: MJT	Date: 8/31/2023
Chkid: DEC	Data: 9/21/2022

Purpose: To determine the following airspace volumes

1. Airspace in Dry Ash Disposal Facility (ADF) with Module 1-6 and Modules 10 through 11 constructed

Subject: 2023 ADF Plan of Op Update Airspace Calcs

- 2. Airspace in Dry Ash Disposal Facility (ADF) with Module 1-6 and Modules 10 through 12 constructed
- 3. Airspace in Dry Ash Disposal Facility (ADF) with Module 1-6 and Modules 10 through 13 constructed

Approach: Use AutoCAD Civil 3D software to compare surfaces from the following data:

Job: Columbia Energy Center

- The total airspace in Phase 1 Modules 1 6 and Phase 2 Modules 10 through 11 was calculated using AutoCAD Civil 3D surfaces. As-built base grades (Figure 1) are based on construction documented grades Waste grades are based on the 2022 Plan of Operation Modification Request proposed waste grades (Figure 2).
- 2. The total airspace in Phase 1 Modules 1 6 and Phase 2 Modules 10 through 12 was calculated using AutoCAD Civil 3D surfaces. As-built base grades (Figure 3) are based on construction documented grades and proposed base grades for Module 12. Waste grades are based on the 2023 Plan of Operation Modification Request, Addendum No. 2 proposed waste grades (Figure 4).
- 3. The total airspace in Phase 1 Modules 1 6 and Phase 2 Modules 10 through 13 was calculated using AutoCAD Civil 3D surfaces. As-built base grades (Figure 5) are based on construction documented grades and proposed base grades for Modules 12-13. Waste grades are based on the 2023 Plan of Operation Modification Request, Addendum No. 2 proposed waste grades (Figure 6).

Results:

Total airspace in Modules 1-6 and Modules 10-11 = 2,596,262 cubic yards (see attached AutoCAD Civil 3D report).

Total airspace in Modules 1-6 and Modules 10-12 = 3,207,520 cubic yards (see attached AutoCAD Civil 3D report).

Total airspace in Modules 1-6 and Modules 10-13 = 3,630,075 cubic yards (see attached AutoCAD Civil 3D report).

Surface Report

Project Name:

I:\25222260.00\Drawings\Civil\Airspace

2023\Total Airspace Vol.dwg

Report Date: 8/30/2023 1:28:26 PM

Client: Alliant

Project Description: Columbia Plan of Operation Update

Prepared by: KP

Linear Units: foot Area Units: squareFoot Volume Units: cubicYard

Surface: Vol - Total Airspace (Mod 10-11)

Description: total airspace volume surface from base grades to waste grades through Module 11. Subtract 1'

drainage layer for final airspace.

Area 2D: 1568771.199 Area 3D: 1637412.816 Elevation Max: 98.458 Elevation Min: -12.273 Number of Points: 13204 Number of Triangles: 25588

Surface: Vol - Total Airspace (Mod 12)

Description: total airspace volume surface from base grades to waste grades through Module 12. Subtract 1'

drainage layer for final airspace.

Area 2D: 1725844.456 Area 3D: 1798272.141 Elevation Max: 111.031 Elevation Min: -4.612

Number of Points: 25608 Number of Triangles: 50458

Surface: Vol - Total Airspace (Mod 13)

Description: total airspace volume surface from base grades to waste grades through Module 12. Subtract 1'

drainage layer for final airspace.

Area 2D: 1833694.680 Area 3D: 1910725.684 Elevation Max: 128.895 Elevation Min: -4.655 Number of Points: 22476 Number of Triangles: 44223

Volume Surface: Vol - Total Airspace (Mod 10-11)

Description: total airspace volume surface from base grades to waste grades through Module 11. Subtract 1'

drainage layer for final airspace.

Volume Cut: 740.176 Volume Fill: 2642283.786 Volume Total: 2641543.610

Drainage Layer = 45,282 cubic yards Compare Surface: Composite Final Waste Grades

Base Surface: Composite Base Grades (through

CCR Volume = 2,641,544 cubic yards - 45,282 cubic yards = 2,596,262 cubic yards

Mod 11)

Volume Surface: Vol - Total Airspace (Mod 12)

Description: total airspace volume surface from base grades to waste grades through Module 12. Subtract 1' drainage layer for final airspace.

Volume Cut: 52.314

Compare Surface: Pr-Waste Grades (Mod 2-12

Capped)

Base Surface: Composite Base Grades (through

Mod 12)

Volume Fill: 3259010.655 Volume Total: 3258958.342

Drainage Layer = 51,438 cubic yards

CCR Volume = 3,258,958 cubic yards - 51,438 cubic yards =

3,207,520 cubic yards

Volume Surface: Vol - Total Airspace (Mod 13)

Description: total airspace volume surface from base grades to waste grades through Module 12. Subtract 1' drainage layer for final airspace.

Volume Cut: 52.908

Compare Surface: Pr-Waste Grades (Mod 2-13

Capped)

Base Surface: Composite Base Grades (through

Mod 13)

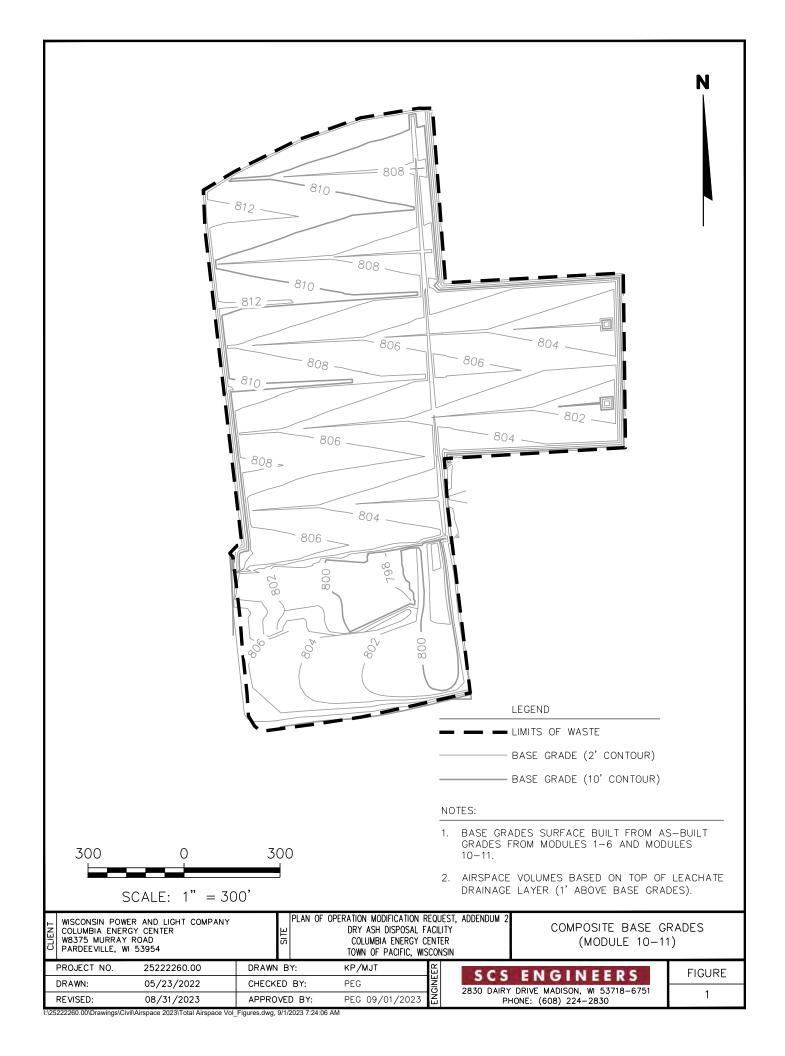
Volume Fill: 3685558.884 Volume Total: 3685505.977

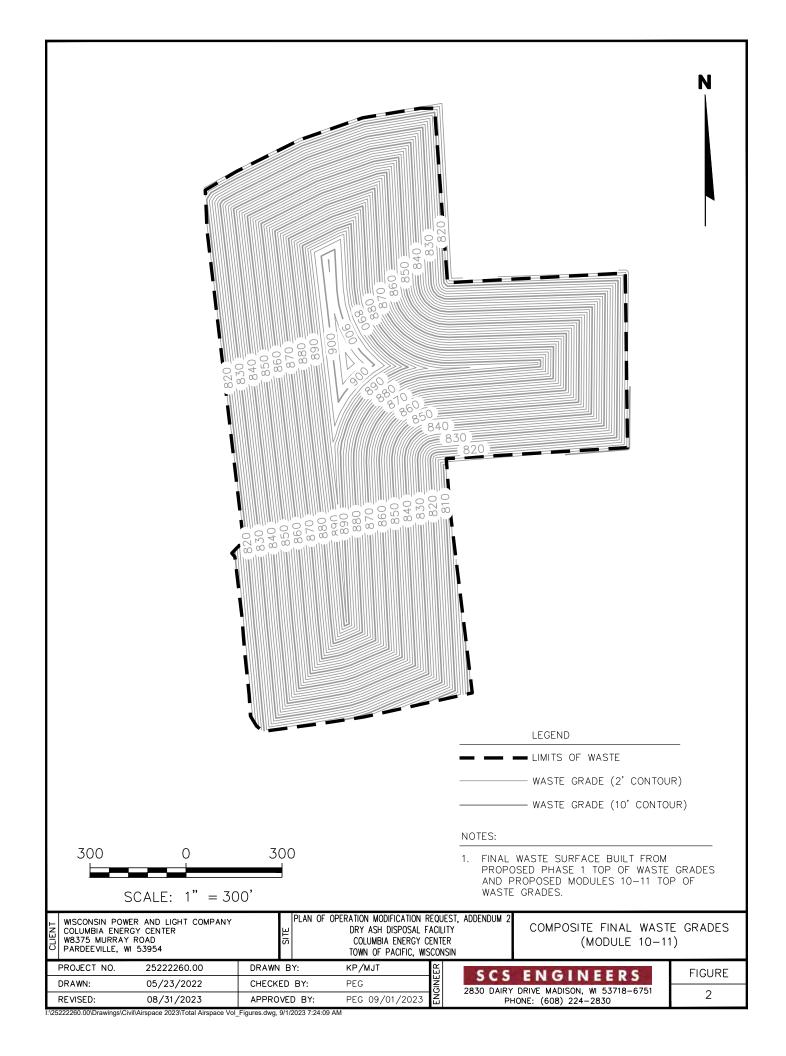
Drainage Layer = 55,431 cubic yards

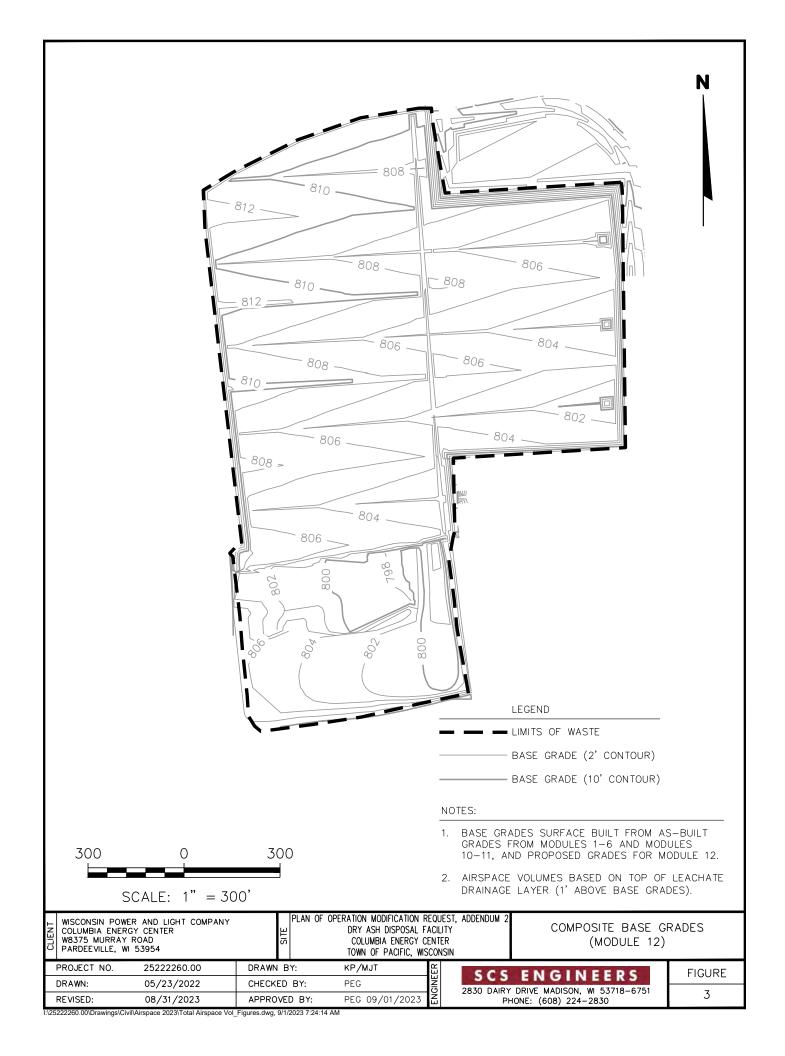
Drainage Layer - 55,451 cubic yards

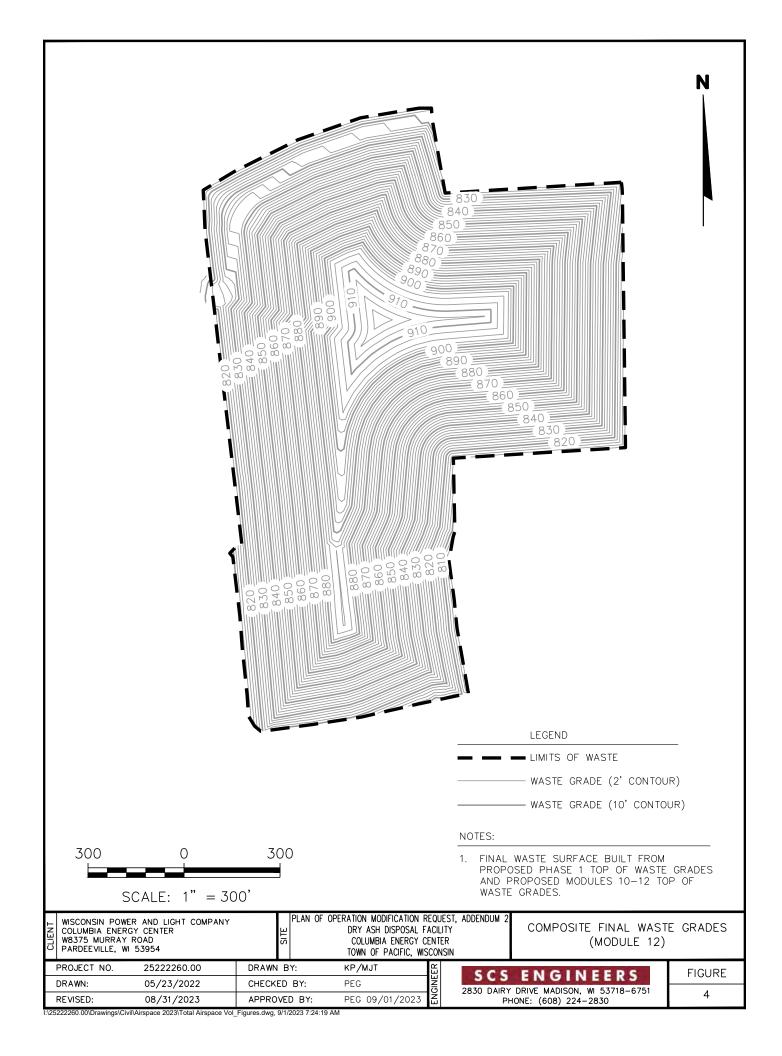
CCR Volume = 3,685,506 cubic yards - 55,431 cubic yards =

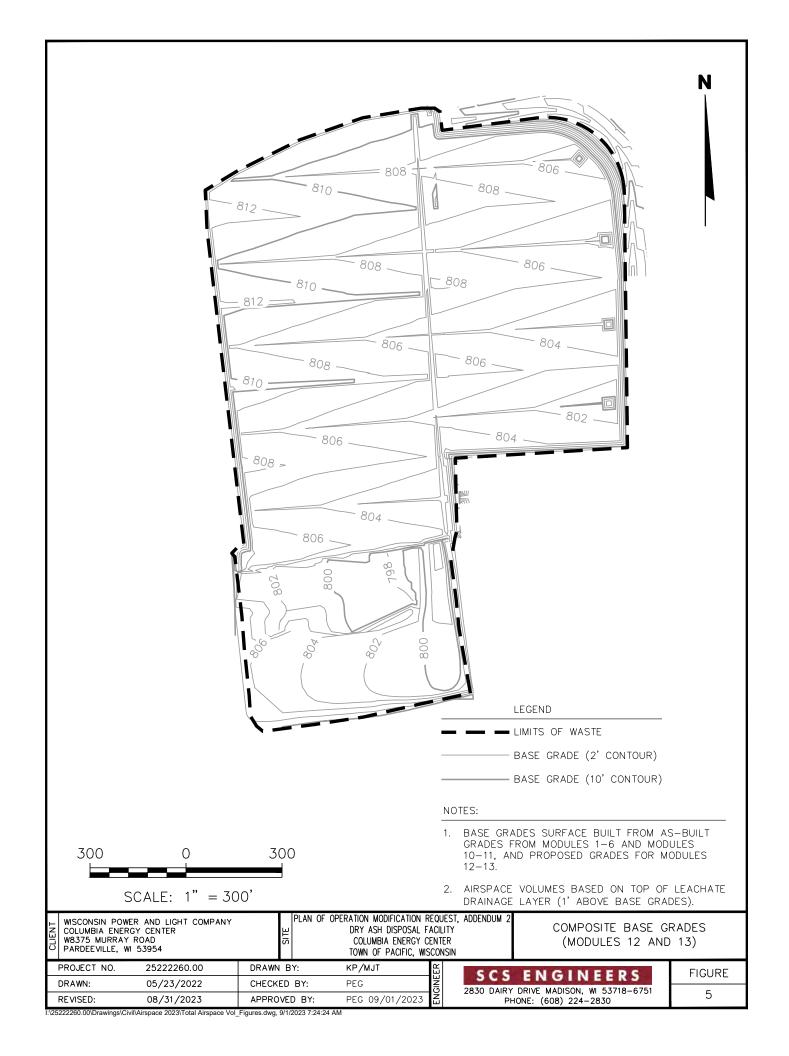
3,630,075 cubic yards

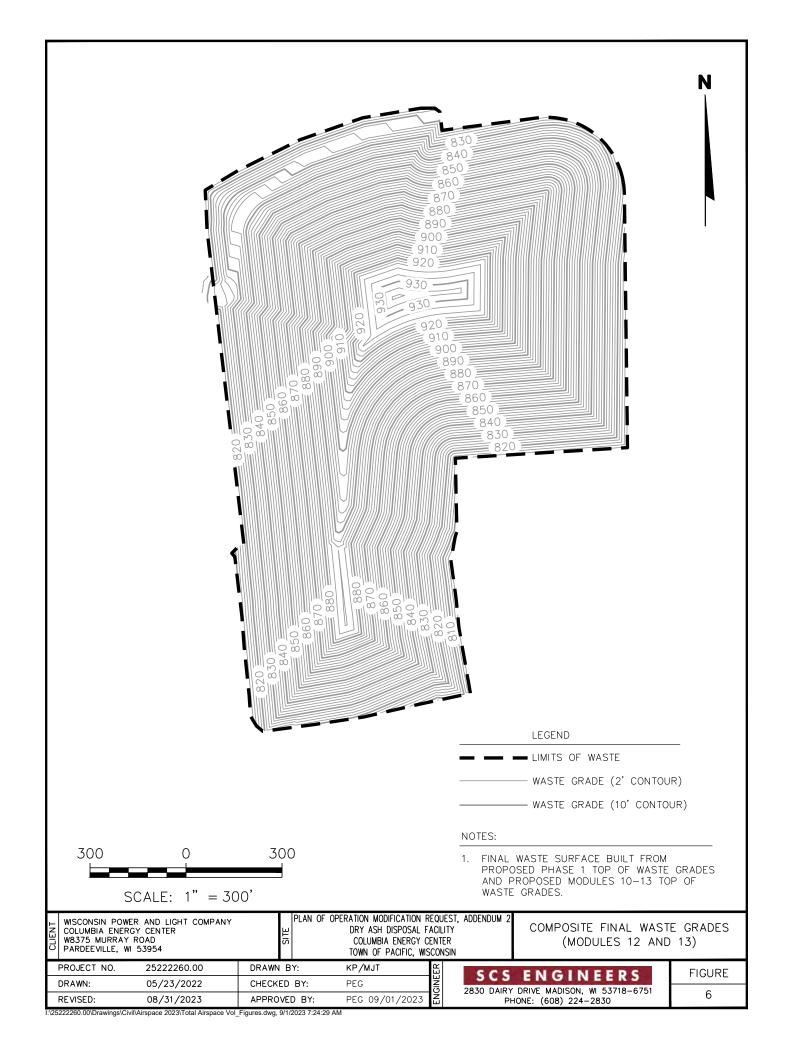












E.6	Stormwater Calculations

E.6.1	Module 12 Stormwater Calculations

SHEET NO.		1 of 4
CALC. NO.		
REV. NO.		
ВҮ	SJL	DATE 8/28/23
CHK,D	WBH	DATE 8/30/23

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

Storm Water Management Calculations

Purpose:

The purpose of the storm water runoff calculations is to demonstrate that the existing storm water sedimentation basin and proposed storm water management features included in the Module 12 Plan Modification Request can accommodate and safely convey the runoff from a 25-year, 24-hour storm event and 100-year, 24-hour storm event during post closure conditions.

Items addressed in these calculations:

- Swales
- Culverts
- Diversion Berms
- Downslope Flumes & Energy Dissipators
- Rock Chutes
- Discharge Aprons
- Sedimentation Basin
- North Infiltration Area

The proposed storm water management conditions are shown on **Figure 1**. The calculations support the capacity check of the following existing storm water management feature:

Feature **Purpose Design Method** Convey storm water runoff from HydroCAD runoff modeling and Swales adjacent areas to culverts and offsite Swale Calculation during post construction conditions Convey storm water from the final HydroCAD runoff modeling and Culverts cover perimeter swales during post **HY-8 Culvert Model** construction conditions Diversion Berms Reduce storm water runoff from final HydroCAD runoff modeling and Diversion Berm Calculations cover slopes and to divert water to perimeter swales during post construction conditions Downslope Flumes & Convey storm water from diversion HydroCAD runoff modeling and **Energy Dissipators** berms down slope to swales and offsite Downslope Flume Calculations drainage features during post construction conditions **Rock Chutes** Erosion protection and convey storm HydroCAD runoff modeling and water from energy dissipators to Rock Chute Calculation existing swale during post construction conditions Erosion protection from culvert HydroCAD runoff modeling and Discharge Aprons discharge at culvert outlets Riprap Apron Calculation To safely handle 25-year, 24-hour HydroCAD runoff modeling Sedimentation Basin storm event without overtopping the 100-year, 24-hour spillway. To safely handle 25-year, 24-hour and North Infiltration Area HydroCAD runoff modeling 100-year, 24-hour storm events without overtopping or backing up the inlet pipe.

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CALC. NO.			
REV. NO.			
ВҮ	SJL	DATE 8/28/23	
CHK,D	MPH	DATE 8/30/23	

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

Approach:

Hydrograph Generation

HydroCAD was used to model the storm water management system and develop the hydrographs using TR-20 methodologies. The model is designed to simulate the surface runoff response of a watershed to a precipitation event. Input parameters for the model include precipitation depth for the design storm events from NOAA ATLAS 14, contributing drainage areas, runoff curve numbers, and time of concentration.

Swale Sizing

The proposed swales were sized for the 25-year, 24-hour storm event. A WisDOT HEC-15 spreadsheet based on Manning's equation was used to calculate the depth of flow and velocity in the swales using the swale geometry and peak flow in the swales (as determined by the Hydrograph Generation models).

Culvert Sizing

Culverts were sized for the 25-year, 24-hour storm event using the HY-8 computer model developed by the US Department of Transportation, Federal Highway Administration.

Diversion Berms

Diversion berms were sized for the 25-year, 24-hour storm event. A WisDOT HEC-15 spreadsheet based on Manning's Equation was used to calculate the depth of flow and velocity in the swale using the swale geometry and peak flow for the storm event (as determined by the Hydrograph Generation Calculations).

Downslope Flumes and Energy Dissipators Sizing

Flumes and energy dissipators were sized for the 25-year, 24-hour storm event. Manning's equation and the orifice equation were used to size the flumes. Energy dissipators were sized using tables from the reference book "Hydraulic Design of Energy Dissipators for Culverts and Channels" US Department of Transportation, Federal Highway Administration, July 2006.

Rock Chute Sizing

Rock chutes were sized for the 25-year, 24-hour storm event. Rock Chutes were sized based on the flow to each culvert location. The Iowa NRCS Rock Chute Design spreadsheet was used to size the chute and riprap.

Discharge Apron Sizing

Riprap aprons were sized for the 25-year, 24-hour storm event using equations in Section 5.2 – Riprap Blanket of WisDOT FDM 13-35-5. The riprap aprons were sized based on the flow to the culvert location. The riprap stone sizing was used to specify the thickness and geometry of the riprap discharge apron.

Sedimentation Basin Sizing

Route the proposed construction and existing drainage runoff through the sedimentation basin to confirm the basin can handle the 25-year, 24-hour storm event and to safely pass the 100-year, 24-hour storm event. HydroCAD was used to model the runoff flow through the basin outfall (as determined by the Hydrograph Generation model).

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	CALC. NO.			
	REV. NO.			
	BY	SJL	DATE 8/28/23	
-	CHK'D.	MRH	DATE 8/30/23	

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

North Infiltration Area Verification

The depressional area located north of the Module 12 Plan Modification construction area acts as a infiltration area and accepts portions of the drainage runoff. Route the proposed construction and existing drainage runoff flowing to the North Infiltration Area to confirm the area can handle the 25-year, 24-hour and 100-year, 24-hour storm events without overtopping or backing up the inlet pipe. HydroCAD was used to model the runoff flow into this area (as determined by the Hydrograph Generation model).

Key Assumptions:

- Drainage areas and time of concentration flow paths are as shown on Figure 1 for Post Construction Conditions.
- An MSE4 rainfall distribution was used based on NRCS Wisconsin rainfall distribution regions.

The precipitation depth for the 25-year, 24-hour storm was assumed to be <u>4.91 inches</u>, based on NOAA ATLAS 14 Point Precipitation Frequency Estimates (NOAA's National Weather Service Hydrometeorological Design Studies Center Precipitation Frequency Data Server).

The precipitation depth for the 100-year, 24-hour storm was assumed to be <u>6.59 inches</u>, based on NOAA ATLAS 14 Point Precipitation Frequency Estimates.

• Runoff curve numbers were based on tables presented in Urban Hydrology for Small Watersheds, and were assumed as follows and as listed in the modeling.

Cover Type	CN	
Final Cover	69 – Pasture/grassland/range in good condition,	
	hydrologic soil group (HSG) (B/C assumed mid value	
	between each soil group)	
Pasture, grassland or range	39 – Pasture/grassland/range, Good, HSG A	
Gravel	96 – Gravel, HSG A	
Water Surface	98 – Water Surface, HSG A	

- Type A soil group for non-disturbed areas outside the landfill as soils are loamy sand.
- Other assumptions are included with the calculations attached to this appendix.

Results:

Hydrograph Generation

The hydrograph modeling results for the 25-year and 100-year, 24-hour storm events are included in the Post Construction Conditions Hydrograph Generation section.

Swale Sizing

The proposed swales will be constructed as shown on the Drawings. The swales have the capacity to safely convey the both the 25-year, 24-hour storm events and maintain a minimum 0.5 foot of freeboard. Refer to the Swale Sizing section.

Appropriate erosion control product was selected based on the velocities and shear stress in the swales. Refer to the Swale Sizing section below for the evaluation.

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CALC. NO.			
REV. NO.			
BY	SJL	DATE 8/28/23	
CHK'D	MRH	DATE 8/30/23	

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

Culvert Sizing

Culverts will be as shown in the Drawings. The culverts have the capacity to safely convey the 25-year, 24-hour storm event. Refer to the Culvert Sizing Section for the detailed calculations.

Diversion Berm Sizing

The proposed final berms will be constructed as shown on the Drawings. The diversion berms will contain the runoff from the 25-year, 24-hour storm event. Refer to the Diversion Berm Design section.

Downslope Flume and Energy Dissipator Sizing

The downslope flumes and energy dissipaters will be constructed as shown on the Drawings. The downslope flumes are designed to contain the runoff from the 25-year, 24-hour storm event. Energy dissipators at the bottom of the downslope flumes have been designed to handle the peak velocities. Refer to the Downslope Flume and Energy Dissipator Sizing section below for detailed calculations.

Rock Chute Sizing

The proposed rock chutes will be constructed as shown in the Drawings. The rock chutes will accommodate the runoff from the 25-year, 24-hour storm event. Refer to the Rock Chute Sizing section.

Discharge Apron Sizing

The proposed riprap aprons will be constructed as shown in the Drawings. The aprons will accommodate the runoff from the 25-year, 24-hour storm event. Refer to Discharge Apron Sizing for design calculations.

Sedimentation Basin Sizing

The existing sedimentation basin has the capacity to safely contain the 25-year, 24-hour storm event and safely pass the 100-year, 24-hour storm event through the emergency spillway.

As shown in the HydroCAD model, the water elevation in both basin areas for each storm event is provided below:

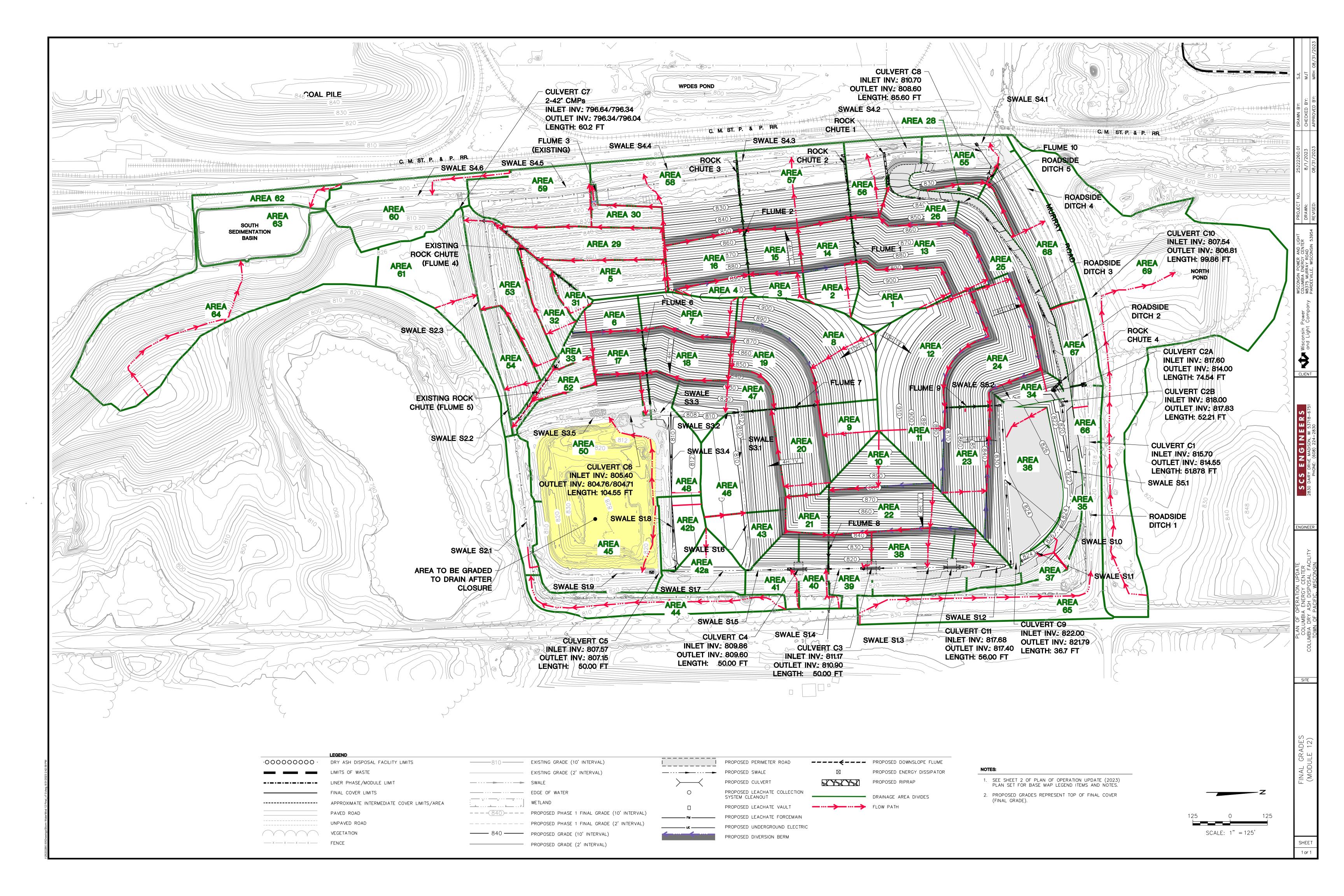
Basin Area	Basin	Basin	Peak	Peak
	Crest	Spillway	Elevation 25-	Elevation 100-
	Elevation	Elevation	year storm	year storm
	(ft MSL)	(ft MSL)	(ft MSL)	(ft MSL)
Existing Sedimentation Basin	794.00	793.00	792.18	793.23

North Infiltration Area Verification

The North Infiltration Area can safely contain the 25-year, 24-hour storm event and the 100-year, 24-hour storm event without overtopping or backing up the inlet pipe at Murray Road.

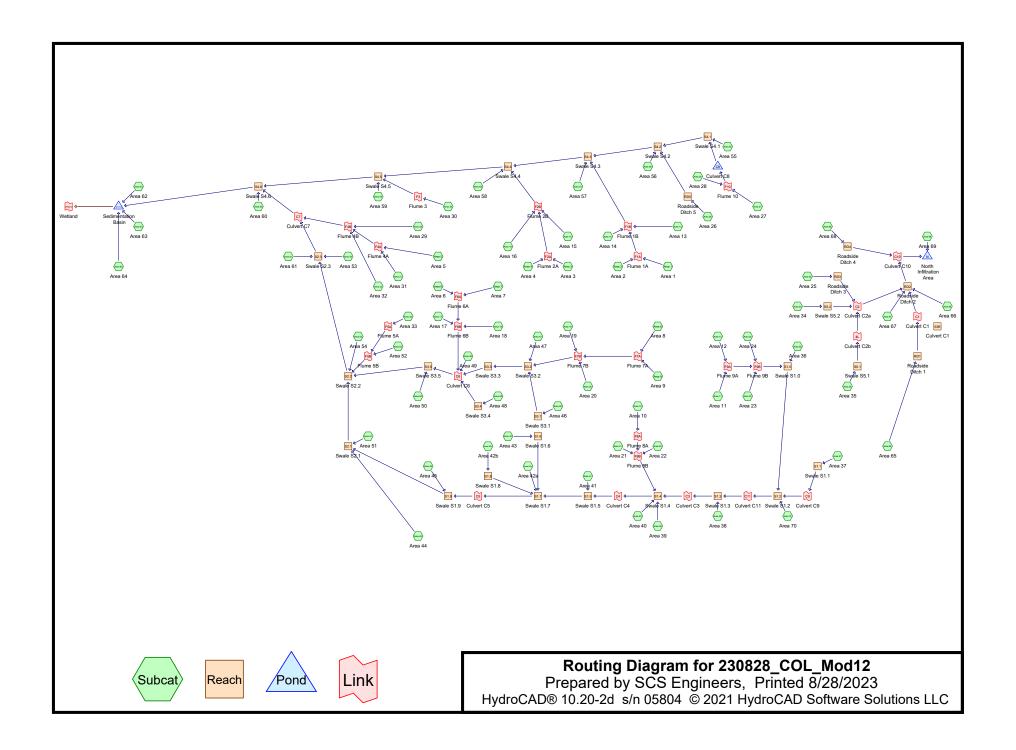
As shown in the HydroCAD model, the water elevation in both basin areas for each storm event is provided below:

Basin Area	Basin Crest Elevation (ft MSL)	Basin Inlet Pipe Elevation (ft MSL)	Peak Elevation 25- year storm (ft MSL)	Peak Elevation 100- year storm (ft MSL)	
North Basin	810.00	806.81	803.89	805.24	



Post Construction Conditions Hydrograph Generation

- 25-year, 24-hour Storm Event
- 100-year, 24-hour Storm Event



230828_COL_Mod12
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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	25-yr, 24-hr	MSE 24-hr	4	Default	24.00	1	4.91	2
2	100-yr, 24-hr	MSE 24-hr	4	Default	24.00	1	6.59	2

Page 3

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentArea 1: Area 1	Runoff Area=1.141 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=500' Tc=6.5 min CN=69 Runoff=3.32 cfs 0.180 af
SubcatchmentArea 10: Area 10	Runoff Area=0.791 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=354' Tc=6.4 min CN=69 Runoff=2.31 cfs 0.125 af
SubcatchmentArea 11: Area 11	Runoff Area=0.885 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=288' Tc=6.4 min CN=69 Runoff=2.59 cfs 0.140 af
SubcatchmentArea 12: Area 12	Runoff Area=2.206 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=628' Tc=6.7 min CN=69 Runoff=6.39 cfs 0.348 af
SubcatchmentArea 13: Area 13	Runoff Area=1.610 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=712' Tc=5.3 min CN=69 Runoff=4.83 cfs 0.254 af
SubcatchmentArea 14: Area 14	Runoff Area=0.626 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=333' Tc=4.5 min CN=69 Runoff=1.94 cfs 0.099 af
SubcatchmentArea 15: Area 15	Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=331' Tc=4.4 min CN=69 Runoff=1.93 cfs 0.098 af
SubcatchmentArea 16: Area 16	Runoff Area=0.943 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=464' Tc=5.9 min CN=69 Runoff=2.80 cfs 0.149 af
SubcatchmentArea 17: Area 17	Runoff Area=0.571 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=315' Tc=4.4 min CN=69 Runoff=1.78 cfs 0.090 af
SubcatchmentArea 18: Area 18	Runoff Area=0.990 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=424' Tc=4.7 min CN=69 Runoff=3.04 cfs 0.156 af
SubcatchmentArea 19: Area 19	Runoff Area=1.179 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=385' Tc=4.7 min CN=69 Runoff=3.62 cfs 0.186 af
SubcatchmentArea 2: Area 2	Runoff Area=0.557 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=303' Tc=4.7 min CN=69 Runoff=1.71 cfs 0.088 af
SubcatchmentArea 20: Area 20	Runoff Area=1.057 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=419' Tc=4.7 min CN=69 Runoff=3.24 cfs 0.167 af
SubcatchmentArea 21: Area 21	Runoff Area=0.434 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=374' Tc=4.5 min CN=69 Runoff=1.35 cfs 0.068 af
SubcatchmentArea 22: Area 22	Runoff Area=1.442 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=489' Tc=4.8 min CN=69 Runoff=4.40 cfs 0.227 af
SubcatchmentArea 23: Area 23	Runoff Area=1.252 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=435' Tc=4.7 min CN=69 Runoff=3.84 cfs 0.197 af

SubcatchmentArea 24: Area 24

Page 4

Runoff Area=1.846 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=701' Tc=5.4 min CN=69 Runoff=5.53 cfs 0.291 af

SubcatchmentArea 25: Area 25 Runoff Area=1.552 ac 0.00% Impervious Runoff Depth=2.64" Flow Length=123' Slope=0.2500 '/' Tc=3.9 min CN=78 Runoff=6.95 cfs 0.341 af

SubcatchmentArea 26: Area 26

Runoff Area=0.616 ac 0.00% Impervious Runoff Depth=2.13"

Flow Length=109' Slope=0.2500 '/' Tc=3.8 min CN=72 Runoff=2.25 cfs 0.109 af

SubcatchmentArea 27: Area 27 Runoff Area=0.149 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=211' Tc=1.2 min CN=69 Runoff=0.52 cfs 0.023 af

SubcatchmentArea 28: Area 28 Runoff Area=0.126 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=59' Slope=0.2500 '/' Tc=2.5 min CN=69 Runoff=0.44 cfs 0.020 af

SubcatchmentArea 29: Area 29

Runoff Area=1.228 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=647' Tc=5.7 min CN=69 Runoff=3.66 cfs 0.194 af

SubcatchmentArea 3: Area 3 Runoff Area=0.348 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=298' Tc=4.8 min CN=69 Runoff=1.06 cfs 0.055 af

SubcatchmentArea 30: Area 30

Runoff Area=0.427 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=362' Tc=3.8 min CN=69 Runoff=1.38 cfs 0.067 af

SubcatchmentArea 31: Area 31

Runoff Area=0.223 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=296' Tc=3.7 min CN=69 Runoff=0.72 cfs 0.035 af

SubcatchmentArea 32: Area 32 Runoff Area=0.655 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=642' Tc=5.0 min CN=69 Runoff=1.98 cfs 0.103 af

SubcatchmentArea 33: Area 33

Runoff Area=0.237 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=348' Tc=4.0 min CN=69 Runoff=0.76 cfs 0.037 af

SubcatchmentArea 34: Area 34

Runoff Area=0.424 ac 0.00% Impervious Runoff Depth=2.29"

Flow Length=78' Slope=0.2500 '/' Tc=3.1 min CN=74 Runoff=1.73 cfs 0.081 af

SubcatchmentArea 35: Area 35

Runoff Area=0.993 ac 0.00% Impervious Runoff Depth=2.46"
Flow Length=126' Tc=4.4 min CN=76 Runoff=4.05 cfs 0.204 af

SubcatchmentArea 36: Area 36

Runoff Area=2.594 ac 0.00% Impervious Runoff Depth=3.19"
Flow Length=183' Tc=2.2 min CN=84 Runoff=14.85 cfs 0.689 af

SubcatchmentArea 37: Area 37

Runoff Area=0.306 ac 0.00% Impervious Runoff Depth=0.66"
Flow Length=133' Tc=4.5 min CN=50 Runoff=0.24 cfs 0.017 af

SubcatchmentArea 38: Area 38 Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=1.52" Flow Length=156' Slope=0.2500 '/' Tc=4.1 min CN=64 Runoff=3.58 cfs 0.181 af

SubcatchmentArea 39: Area 39

Runoff Area=0.742 ac 0.00% Impervious Runoff Depth=1.45"
Flow Length=145' Tc=4.3 min CN=63 Runoff=1.74 cfs 0.090 af

230828 COL Mod12

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91" Printed 8/28/2023

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SubcatchmentArea 4: Area 4 Runoff Area=0.288 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=296' Tc=4.0 min CN=69 Runoff=0.92 cfs 0.045 af

SubcatchmentArea 40: Area 40

Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.00"

Flow Length=168' Tc=9.6 min CN=56 Runoff=0.72 cfs 0.051 af

SubcatchmentArea 41: Area 41 Runoff Area=0.739 ac 0.00% Impervious Runoff Depth=1.00"

Flow Length=141' Tc=7.2 min CN=56 Runoff=0.98 cfs 0.061 af

SubcatchmentArea 42a: Area 42a Runoff Area=0.871 ac 0.00% Impervious Runoff Depth=1.59"

Flow Length=144' Slope=0.0500 '/' Tc=7.8 min CN=65 Runoff=2.00 cfs 0.116 af

SubcatchmentArea 42b: Area 42b Runoff Area=0.712 ac 0.00% Impervious Runoff Depth=2.05"

Flow Length=102' Slope=0.0500 '/' Tc=7.3 min CN=71 Runoff=2.19 cfs 0.122 af

SubcatchmentArea 43: Area 43 Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=2.14 cfs 0.121 af

SubcatchmentArea 44: Area 44 Runoff Area=1.416 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=941' Slope=0.0260 '/' Tc=22.0 min CN=39 Runoff=0.06 cfs 0.022 af

SubcatchmentArea 45: Area 45 Runoff Area=2.792 ac 0.00% Impervious Runoff Depth=1.32"

Flow Length=419' Tc=14.7 min CN=61 Runoff=3.88 cfs 0.306 af

SubcatchmentArea 46: Area 46 Runoff Area = 2.044 ac 0.00% Impervious Runoff Depth = 1.89"

Flow Length=138' Tc=4.0 min CN=69 Runoff=6.52 cfs 0.322 af

SubcatchmentArea 47: Area 47 Runoff Area=0.457 ac 0.00% Impervious Runoff Depth=1.97"

Flow Length=122' Slope=0.2500 '/' Tc=3.9 min CN=70 Runoff=1.53 cfs 0.075 af

SubcatchmentArea 48: Area 48 Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=2.21"

Flow Length=98' Slope=0.0500 '/' Tc=7.2 min CN=73 Runoff=3.99 cfs 0.220 af

SubcatchmentArea 49: Area 49 Runoff Area=0.079 ac 0.00% Impervious Runoff Depth=2.13"

Flow Length=143' Tc=5.1 min CN=72 Runoff=0.27 cfs 0.014 af

SubcatchmentArea 5: Area 5 Runoff Area=0.986 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=608' Tc=6.6 min CN=69 Runoff=2.87 cfs 0.155 af

SubcatchmentArea 50: Area 50 Runoff Area=3.726 ac 0.00% Impervious Runoff Depth=2.05"

Flow Length=391' Tc=11.1 min CN=71 Runoff=9.80 cfs 0.636 af

SubcatchmentArea 51: Area 51 Runoff Area=0.698 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=100' Slope=0.0600 '/' Tc=6.8 min CN=39 Runoff=0.03 cfs 0.011 af

SubcatchmentArea 52: Area 52 Runoff Area=0.475 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=425' Tc=4.5 min CN=69 Runoff=1.48 cfs 0.075 af

SubcatchmentArea 53: Area 53 Runoff Area = 1.618 ac 0.00% Impervious Runoff Depth = 1.18"

Flow Length=384' Tc=4.2 min CN=59 Runoff=2.98 cfs 0.160 af

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SubcatchmentArea 54: Area 54 Runoff Area=0.826 ac 0.00% Impervious Runoff Depth=1.12" Flow Length=126' Slope=0.2500 '/' Tc=3.9 min CN=58 Runoff=1.45 cfs 0.077 af

SubcatchmentArea 55: Area 55

Runoff Area=1.089 ac 0.00% Impervious Runoff Depth=1.12"
Flow Length=158' Tc=4.3 min CN=58 Runoff=1.86 cfs 0.102 af

SubcatchmentArea 56: Area 56 Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=1.18"

Flow Length=221' Tc=4.4 min CN=59 Runoff=2.17 cfs 0.118 af

SubcatchmentArea 57: Area 57

Runoff Area=2.220 ac 0.00% Impervious Runoff Depth=0.88"
Flow Length=240' Tc=4.6 min CN=54 Runoff=2.69 cfs 0.162 af

SubcatchmentArea 58: Area 58

Runoff Area=2.476 ac 0.00% Impervious Runoff Depth=1.12"
Flow Length=263' Tc=4.7 min CN=58 Runoff=4.13 cfs 0.231 af

SubcatchmentArea 59: Area 59 Runoff Area=1.683 ac 0.00% Impervious Runoff Depth=0.82"

Flow Length=146' Slope=0.2345 '/' Tc=4.1 min CN=53 Runoff=1.90 cfs 0.115 af

SubcatchmentArea 6: Area 6 Runoff Area=0.504 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=258' Tc=3.4 min CN=69 Runoff=1.66 cfs 0.079 af

SubcatchmentArea 60: Area 60 Runoff Area=2.001 ac 0.00% Impervious Runoff Depth=0.33"

Flow Length=318' Tc=20.8 min CN=43 Runoff=0.24 cfs 0.055 af

SubcatchmentArea 61: Area 61 Runoff Area=2.177 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=149' Tc=8.2 min CN=39 Runoff=0.09 cfs 0.033 af

SubcatchmentArea 62: Area 62 Runoff Area=0.594 ac 0.00% Impervious Runoff Depth=0.66"

Flow Length=147' Slope=0.0544 '/' Tc=7.6 min CN=50 Runoff=0.39 cfs 0.032 af

SubcatchmentArea 63: Area 63 Runoff Area=1.509 ac 100.00% Impervious Runoff Depth=4.67"

Tc=0.0 min CN=98 Runoff=10.73 cfs 0.588 af

SubcatchmentArea 64: Area 64 Runoff Area=5.227 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=701' Tc=7.9 min CN=39 Runoff=0.22 cfs 0.079 af

SubcatchmentArea 65: Area 65 Runoff Area=3.035 ac 0.00% Impervious Runoff Depth=0.51"

Flow Length=886' Slope=0.0068 '/' Tc=38.9 min CN=47 Runoff=0.61 cfs 0.128 af

SubcatchmentArea 66: Area 66 Runoff Area=0.409 ac 0.00% Impervious Runoff Depth=1.18"

Flow Length=52' Slope=0.1154 '/' $Tc=3.1 \ min$ CN=59 Runoff=0.81 cfs 0.040 af

SubcatchmentArea 67: Area 67 Runoff Area=0.755 ac 0.00% Impervious Runoff Depth=0.71"

Flow Length=86' Slope=0.2326 '/' Tc=3.5 min CN=51 Runoff=0.71 cfs 0.045 af

SubcatchmentArea 68: Area 68 Runoff Area=1.671 ac 0.00% Impervious Runoff Depth=0.71"

Flow Length=126' Tc=4.0 min CN=51 Runoff=1.52 cfs 0.099 af

SubcatchmentArea 69: Area 69 Runoff Area=9.875 ac 0.00% Impervious Runoff Depth=0.29"

Flow Length=1,337' Tc=16.6 min CN=42 Runoff=0.99 cfs 0.238 af

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SubcatchmentArea 7: Area 7Runoff Area=0.936 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=541' Tc=5.3 min CN=69 Runoff=2.81 cfs 0.148 af

SubcatchmentArea 70: Area 70

Runoff Area = 0.694 ac 0.00% Impervious Runoff Depth = 1.67"

Flow Length = 127' Slope = 0.2500 '/' Tc = 3.9 min CN = 66 Runoff = 1.94 cfs 0.096 af

SubcatchmentArea 8: Area 8Runoff Area=2.114 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=371' Tc=6.6 min CN=69 Runoff=6.16 cfs 0.333 af

SubcatchmentArea 9: Area 9Runoff Area=0.810 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=292' Tc=6.3 min CN=69 Runoff=2.37 cfs 0.128 af

Reach 53R: Culvert C1Avg. Flow Depth=0.00' Max Vel=0.00 fps
24.0" Round Pipe n=0.012 L=51.9' S=0.0222 '/' Capacity=36.48 cfs Outflow=0.00 cfs 0.000 af

Reach RD1: Roadside Ditch 1 Avg. Flow Depth=0.10' Max Vel=1.34 fps Inflow=0.61 cfs 0.128 af n=0.030 L=440.6' S=0.0188 '/' Capacity=47.16 cfs Outflow=0.60 cfs 0.128 af

Reach RD2: Roadside Ditch 2 Avg. Flow Depth=0.40' Max Vel=2.70 fps Inflow=11.19 cfs 0.838 af n=0.030 L=433.0' S=0.0162 '/' Capacity=72.77 cfs Outflow=10.57 cfs 0.838 af

Reach RD3: Roadside Ditch 3 Avg. Flow Depth=0.62' Max Vel=3.79 fps Inflow=6.95 cfs 0.341 af n=0.030 L=821.0' S=0.0288 '/' Capacity=20.76 cfs Outflow=5.77 cfs 0.341 af

Reach RD4: Roadside Ditch 4 Avg. Flow Depth=0.12' Max Vel=1.05 fps Inflow=1.52 cfs 0.099 af n=0.030 L=495.6' S=0.0090 '/' Capacity=54.26 cfs Outflow=0.92 cfs 0.099 af

Reach RD5: Roadside Ditch 5 Avg. Flow Depth=0.38' Max Vel=3.65 fps Inflow=2.25 cfs 0.109 af n=0.030 L=288.0' S=0.0531 '/' Capacity=28.18 cfs Outflow=2.01 cfs 0.109 af

Reach S1.0: Swale S1.0Avg. Flow Depth=0.93' Max Vel=2.44 fps Inflow=30.93 cfs 1.665 af n=0.030 L=551.3' S=0.0039 '/' Capacity=118.85 cfs Outflow=26.03 cfs 1.665 af

Reach S1.1: Swale S1.1Avg. Flow Depth=0.03' Max Vel=0.99 fps Inflow=0.24 cfs 0.017 af n=0.030 L=98.0' S=0.0396 '/' Capacity=376.93 cfs Outflow=0.22 cfs 0.017 af

Reach S1.2: Swale S1.2Avg. Flow Depth=0.57' Max Vel=4.60 fps Inflow=27.11 cfs 1.778 af n=0.030 L=170.0' S=0.0242 '/' Capacity=294.55 cfs Outflow=26.46 cfs 1.778 af

Reach S1.3: Swale S1.3Avg. Flow Depth=0.55' Max Vel=4.94 fps Inflow=27.98 cfs 1.960 af n=0.030 L=212.6' S=0.0293 '/' Capacity=324.28 cfs Outflow=27.48 cfs 1.960 af

Reach S1.4: Swale S1.4Avg. Flow Depth=0.73' Max Vel=4.09 fps Inflow=32.89 cfs 2.521 af n=0.030 L=72.2' S=0.0144 '/' Capacity=227.36 cfs Outflow=32.85 cfs 2.521 af

Reach S1.5: Swale S1.5Avg. Flow Depth=1.02' Max Vel=2.73 fps Inflow=33.64 cfs 2.583 af n=0.030 L=148.0' S=0.0045 '/' Capacity=126.50 cfs Outflow=33.34 cfs 2.583 af

Reach S1.6: Swale S1.6Avg. Flow Depth=0.20' Max Vel=1.12 fps Inflow=2.14 cfs 0.121 af n=0.030 L=179.7' S=0.0050 '/' Capacity=134.06 cfs Outflow=1.95 cfs 0.121 af

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Reach S1.7: Swale S1.7	Avg. Flow Depth=1.04' Max Vel=3.05 fps Inflow=38.89 cfs 2.941 af n=0.030 L=252.0' S=0.0054 '/' Capacity=139.68 cfs Outflow=38.11 cfs 2.941 af
Reach S1.8: Swale S1.8	Avg. Flow Depth=0.31' Max Vel=1.88 fps Inflow=2.19 cfs 0.122 af n=0.030 L=245.8' S=0.0099'/' Capacity=90.14 cfs Outflow=1.99 cfs 0.122 af
Reach S1.9: Swale S1.9	Avg. Flow Depth=0.99' Max Vel=3.48 fps Inflow=41.96 cfs 3.247 af n=0.030 L=422.0' S=0.0075 '/' Capacity=163.67 cfs Outflow=40.72 cfs 3.247 af
Reach S2.1: Swale S2.1	Avg. Flow Depth=1.34' Max Vel=3.22 fps Inflow=40.74 cfs 3.279 af n=0.030 L=389.0' S=0.0054 '/' Capacity=97.05 cfs Outflow=39.80 cfs 3.279 af
Reach S2.2: Swale S2.2	Avg. Flow Depth=1.30' Max Vel=3.34 fps Inflow=66.42 cfs 6.023 af n=0.030 L=411.0' S=0.0049 '/' Capacity=152.61 cfs Outflow=65.44 cfs 6.023 af
Reach S2.3: Swale S2.3	Avg. Flow Depth=1.00' Max Vel=4.74 fps Inflow=66.33 cfs 6.216 af n=0.030 L=307.0' S=0.0130 '/' Capacity=249.72 cfs Outflow=65.84 cfs 6.216 af
Reach S3.1: Swale S3.1	Avg. Flow Depth=0.37' Max Vel=1.62 fps Inflow=6.52 cfs 0.322 af n=0.030 L=357.0' S=0.0050 '/' Capacity=133.76 cfs Outflow=5.46 cfs 0.322 af
Reach S3.2: Swale S3.2	Avg. Flow Depth=0.76' Max Vel=2.46 fps Inflow=20.49 cfs 1.211 af n=0.030 L=34.0' S=0.0050 '/' Capacity=133.95 cfs Outflow=20.35 cfs 1.211 af
Reach S3.3: Swale S3.3	Avg. Flow Depth=0.58' Max Vel=3.39 fps Inflow=20.35 cfs 1.211 af n=0.030 L=200.0' S=0.0130 '/' Capacity=215.99 cfs Outflow=19.21 cfs 1.211 af
Reach S3.4: Swale S3.4	Avg. Flow Depth=0.47' Max Vel=1.99 fps Inflow=3.99 cfs 0.220 af n=0.030 L=283.0' S=0.0071 '/' Capacity=76.21 cfs Outflow=3.61 cfs 0.220 af
Reach S3.5: Swale S3.5	Avg. Flow Depth=1.27' Max Vel=2.27 fps Inflow=39.25 cfs 2.554 af n=0.030 L=318.5' S=0.0024 '/' Capacity=93.14 cfs Outflow=37.05 cfs 2.554 af
Reach S4.1: Swale S4.1	Avg. Flow Depth=0.14' Max Vel=1.62 fps Inflow=2.74 cfs 0.145 af n=0.030 L=240.0' S=0.0153 '/' Capacity=70.22 cfs Outflow=2.38 cfs 0.145 af
Reach S4.2: Swale S4.2	Avg. Flow Depth=0.26' Max Vel=2.12 fps Inflow=6.29 cfs 0.372 af n=0.030 L=259.3' S=0.0127 '/' Capacity=63.88 cfs Outflow=5.84 cfs 0.372 af
Reach S4.3: Swale S4.3	Avg. Flow Depth=0.73' Max Vel=1.82 fps Inflow=18.75 cfs 1.155 af n=0.030 L=362.9' S=0.0027 '/' Capacity=108.12 cfs Outflow=16.68 cfs 1.155 af
Reach S4.4: Swale S4.4	Avg. Flow Depth=0.75' Max Vel=2.26 fps Inflow=22.06 cfs 1.732 af n=0.030 L=495.6' S=0.0040 '/' Capacity=132.85 cfs Outflow=21.14 cfs 1.732 af
Reach S4.5: Swale S4.5	Avg. Flow Depth=0.59' Max Vel=3.07 fps Inflow=22.26 cfs 1.915 af n=0.030 L=411.1' S=0.0097 '/' Capacity=465.89 cfs Outflow=21.79 cfs 1.915 af
Reach S4.6: Swale S4.6	Avg. Flow Depth=1.24' Max Vel=5.00 fps Inflow=89.57 cfs 8.673 af n=0.030 L=537.0' S=0.0112 '/' Capacity=499.25 cfs Outflow=88.39 cfs 8.673 af

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MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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Reach S5.1: Swale S5.1 Avg. Flow Depth=0.20' Max Vel=2.00 fps Inflow=4.05 cfs 0.204 af

n=0.030 L=428.0' S=0.0154'/' Capacity=235.24 cfs Outflow=3.50 cfs 0.204 af

Avg. Flow Depth=0.10' Max Vel=1.89 fps Inflow=1.73 cfs 0.081 af Reach S5.2: Swale S5.2

n=0.030 L=183.6' S=0.0331 '/' Capacity=344.73 cfs Outflow=1.51 cfs 0.081 af

Pond C8: Culvert C8 Peak Elev=811.21' Storage=0.000 af Inflow=0.95 cfs 0.043 af

12.0" Round Culvert n=0.012 L=85.6' S=0.0245'/' Outflow=0.96 cfs 0.043 af

Pond N: North Infiltration Area Peak Elev=803.89' Storage=20,733 cf Inflow=11.60 cfs 1.175 af

Outflow=1.40 cfs 1.175 af

Pond Sed Pond: Sedimentation Basin Peak Elev=792.17' Storage=175,109 cf Inflow=90.06 cfs 9.372 af 6.223 af Primary=10.22 cfs 3.149 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=15.75 cfs 9.372 af

Link 4L: Culvert C2b Inflow=3.50 cfs 0.204 af

Primary=3.50 cfs 0.204 af

Link C1: Culvert C1 Inflow=0.60 cfs 0.128 af

Primary=0.60 cfs 0.128 af

Inflow=11.21 cfs 0.937 af Link C10: Culvert C10

Primary=11.21 cfs 0.937 af

Link C11: Culvert C11 Inflow=26.46 cfs 1.778 af

Primary=26.46 cfs 1.778 af

Link C2: Culvert C2a Inflow=10.41 cfs 0.626 af

Primary=10.41 cfs 0.626 af

Link C3: Culvert C3 Inflow=27.48 cfs 1.960 af

Primary=27.48 cfs 1.960 af

Link C4: Culvert C4 Inflow=32.85 cfs 2.521 af

Primary=32.85 cfs 2.521 af

Link C5: Culvert C5 Inflow=38.11 cfs 2.941 af

Primary=38.11 cfs 2.941 af

Link C6: Culvert C6 Inflow=30.03 cfs 1.918 af

Primary=30.03 cfs 1.918 af

Link C7: Culvert C7 Inflow=68.10 cfs 6.703 af

Primary=68.10 cfs 6.703 af

Inflow=0.22 cfs 0.017 af Link C9: Culvert C9

Primary=0.22 cfs 0.017 af

Link F10: Flume 10 Inflow=0.95 cfs 0.043 af

Primary=0.95 cfs 0.043 af

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Link F1A: Flume 1A	Inflow=4.96 cfs 0.268 af	

Link F1A: Flume 1A	Inflow=4.96 cfs 0.268 af Primary=4.96 cfs 0.268 af
Link F1B: Flume 1B	Inflow=11.65 cfs 0.620 af Primary=11.65 cfs 0.620 af
Link F2A: Flume 2A	Inflow=1.98 cfs 0.100 af Primary=1.98 cfs 0.100 af
Link F2B: Flume 2B	Inflow=6.58 cfs 0.347 af Primary=6.58 cfs 0.347 af
Link F3: Flume 3	Inflow=1.38 cfs 0.067 af Primary=1.38 cfs 0.067 af
Link F4A: Flume 4A	Inflow=3.47 cfs 0.191 af Primary=3.47 cfs 0.191 af
Link F4B: Flume 4B	Inflow=9.10 cfs 0.488 af Primary=9.10 cfs 0.488 af
Link F5A: Flume 5A	Inflow=0.76 cfs 0.037 af Primary=0.76 cfs 0.037 af
Link F5B: Flume 5B	Inflow=2.23 cfs 0.112 af Primary=2.23 cfs 0.112 af
Link F6A: Flume 6A	Inflow=4.38 cfs 0.227 af Primary=4.38 cfs 0.227 af
Link F6B: Flume 6B	Inflow=9.20 cfs 0.473 af Primary=9.20 cfs 0.473 af
Link F7A: Flume 7A	Inflow=8.53 cfs 0.461 af Primary=8.53 cfs 0.461 af
Link F7B: Flume 7B	Inflow=15.13 cfs 0.814 af Primary=15.13 cfs 0.814 af
Link F8A: Flume 8A	Inflow=2.31 cfs 0.125 af Primary=2.31 cfs 0.125 af
Link F8B: Flume 8B	Inflow=7.92 cfs 0.421 af Primary=7.92 cfs 0.421 af
Link F9A: Flume 9A	Inflow=8.97 cfs 0.487 af Primary=8.97 cfs 0.487 af
Link F9B: Flume 9B	Inflow=18.15 cfs 0.976 af Primary=18.15 cfs 0.976 af
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230828_COL_Mod12

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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Link Wetland: Wetland Inflow=10.22 cfs 3.149 af Primary=10.22 cfs 3.149 af

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#### **Summary for Subcatchment Area 1: Area 1**

Runoff = 3.32 cfs @ 12.14 hrs, Volume= 0.180 af, Depth= 1.89"

Routed to Link F1A: Flume 1A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	1.	141 6	9 Past	ture/grassl	and/range,	Fair, HSG B
_	1.	141	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.7	60	0.1000	0.27		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.8	40	0.2500	0.36		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	16	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.9	384	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
_						n= 0.030 Earth, grassed & winding
	6.5	500	Total			

#### **Summary for Subcatchment Area 10: Area 10**

Runoff = 2.31 cfs @ 12.14 hrs, Volume= 0.125 af, Depth= 1.89"

Routed to Link F8A: Flume 8A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription			
0.791 69 Pasture/grassland/range, Fair, HSG B							
_	0.	791	100.	00% Pervi	ous Area		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.2	71	0.1000	0.28		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	1.4	29	0.2500	0.34		Sheet Flow,	
	0.4	00	0.2500	2.50		Grass: Short n= 0.150 P2= 2.77"	
	0.4	88	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
_	0.4	166	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
	6.4	354	Total				

6.4 354 Tota

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#### **Summary for Subcatchment Area 11: Area 11**

Runoff = 2.59 cfs @ 12.14 hrs, Volume= 0.140 af, Depth= 1.89"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Des	cription		
0.	885 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	885	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	71	0.1000	0.28		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.4	29	0.2500	0.34		Sheet Flow,
0.6	119	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.2	69	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
6.4	288	Total			

#### **Summary for Subcatchment Area 12: Area 12**

Runoff = 6.39 cfs @ 12.14 hrs, Volume= 0.348 af, Depth= 1.89"

Routed to Link F9A: Flume 9A

	Area	(ac) C	N Desc	cription		
_	2.	206 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	2.	206	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	2.7	40	0.1000	0.25	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	2.5	60	0.2500	0.39		Sheet Flow,
	0.2	EC	0.0500	2.50		Grass: Short n= 0.150 P2= 2.77"
	0.3	56	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	1.2	472	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
			0.0200	3.7 1	30.01	Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
_						n= 0.030 Earth, grassed & winding
	6.7	628	Total			

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#### **Summary for Subcatchment Area 13: Area 13**

Runoff = 4.83 cfs @ 12.13 hrs, Volume= 0.254 af, Depth= 1.89"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription		
	1.	610 6	9 Past	ure/grassl	Fair, HSG B	
	1.	610	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.1	31	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	1.4	581	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	5.3	712	Total			

# Summary for Subcatchment Area 14: Area 14

Runoff = 1.94 cfs @ 12.11 hrs, Volume= 0.099 af, Depth= 1.89"

Routed to Link F1B: Flume 1B

	Area	(ac) C	N Des	cription		
	0.	626 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	626	100.	00% Pervi	ous Area	
Tc Length Slope Velocity Capac (min) (feet) (ft/ft) (ft/sec) (c						Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.5	183	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	4.5	333	Total			

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# **Summary for Subcatchment Area 15: Area 15**

Runoff = 1.93 cfs @ 12.11 hrs, Volume= 0.098 af, Depth= 1.89"

Routed to Link F2B: Flume 2B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	ı (ac) C	N Des	cription		
	0.620	69 Past	ure/grassl	and/range,	Fair, HSG B
	).620	100.	00% Pervi	ous Area	
Tc (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.4	331	Total			

# **Summary for Subcatchment Area 16: Area 16**

Runoff = 2.80 cfs @ 12.14 hrs, Volume= 0.149 af, Depth= 1.89"

Routed to Link F2B: Flume 2B

Area	(ac) C	N Desc	cription		
0.	943 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	943	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	100	0.1500	0.35		Sheet Flow,
0.5	95	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	269	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5.9	464	Total			

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# **Summary for Subcatchment Area 17: Area 17**

Runoff = 1.78 cfs @ 12.11 hrs, Volume= 0.090 af, Depth= 1.89"

Routed to Link F6B: Flume 6B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription			
0.571 69 Pasture/grassland/range, Fair, HSG B							
	0.	571	100.	00% Pervi	ous Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	3.8	100	0.2500	0.43		Sheet Flow,	
	0.2	44	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	0.4	171	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
	1 1	215	Total				

4.4 315 Total

# **Summary for Subcatchment Area 18: Area 18**

Runoff = 3.04 cfs @ 12.12 hrs, Volume= 0.156 af, Depth= 1.89"

Routed to Link F6B: Flume 6B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	0.	990 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	990	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.8	100	0.2500	0.43		Sheet Flow,
	0.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.7	278	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
_	17	121	Total			<u>-</u>

4.7 424 Total

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#### **Summary for Subcatchment Area 19: Area 19**

Runoff = 3.62 cfs @ 12.12 hrs, Volume= 0.186 af, Depth= 1.89"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription		
1.	.179 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	.179	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.3	60	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.6	225	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	385	Total			

# Summary for Subcatchment Area 2: Area 2

Runoff = 1.71 cfs @ 12.12 hrs, Volume= 0.088 af, Depth= 1.89"

Routed to Link F1A: Flume 1A

Area	(ac) C	N Desc	cription		
0	.557 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0	.557	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	100	0.1950	0.39		Sheet Flow,
0.1	22	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	303	Total			

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# **Summary for Subcatchment Area 20: Area 20**

Runoff = 3.24 cfs @ 12.12 hrs, Volume= 0.167 af, Depth= 1.89"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	1.	057 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	057	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.3	57	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	262	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
-	47	<u>⊿</u> 10	Total			11 0.000 Earth, gradood & Willamig

#### 4.7 419 Total

# **Summary for Subcatchment Area 21: Area 21**

Runoff = 1.35 cfs @ 12.11 hrs, Volume= 0.068 af, Depth= 1.89"

Routed to Link F8B: Flume 8B

	Area	(ac) C	N Desc	cription		
	0.	434 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	434	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.1	15	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	259	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
_						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	4.5	374	Total			

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#### **Summary for Subcatchment Area 22: Area 22**

Runoff = 4.40 cfs @ 12.12 hrs, Volume= 0.227 af, Depth= 1.89"

Routed to Link F8B: Flume 8B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription		
1.	.442 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	.442	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	49	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.8	340	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.8	489	Total			

# **Summary for Subcatchment Area 23: Area 23**

Runoff = 3.84 cfs @ 12.12 hrs, Volume= 0.197 af, Depth= 1.89"

Routed to Link F9B: Flume 9B

Area	(ac) C	N Des	cription					
1	.252 6	39 Past	ure/grassl	and/range,	Fair, HSG B			
1	1.252 100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
3.8	100	0.2500	0.43		Sheet Flow,			
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
0.7	285	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
4.7	435	Total						

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#### **Summary for Subcatchment Area 24: Area 24**

Runoff = 5.53 cfs @ 12.13 hrs, Volume= 0.291 af, Depth= 1.89"

Routed to Link F9B: Flume 9B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	a (ac) C	N Des	cription		
•	1.846 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.846	100.	00% Pervi	ous Area	
To (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	43	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.4	558	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5.4	701	Total			

### **Summary for Subcatchment Area 25: Area 25**

Runoff = 6.95 cfs @ 12.10 hrs, Volume= 0.341 af, Depth= 2.64"

Routed to Reach RD3: Roadside Ditch 3

_	Area	(ac) C	N Des	cription		
						Fair, HSG B
_	0.	528 9	96 Grav	∕el surface	, HSG A	
1.552 78 Weighted Average						
1.552 100.00% Pervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
Ī	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	23	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
-	3.9	123	Total			<u> </u>

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# **Summary for Subcatchment Area 26: Area 26**

Runoff = 2.25 cfs @ 12.10 hrs, Volume=

0.109 af, Depth= 2.13"

Routed to Reach RD5: Roadside Ditch 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	Description								
	0.			•	•	Fair, HSG B						
_	0.	073	96 Grav	vel surface	, HSG A							
0.616 72 Weighted Average 0.616 100.00% Pervious Area												
0.616 100.00% Pervious Area												
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	3.8	100	0.2500	0.43		Sheet Flow,						
						Grass: Short n= 0.150 P2= 2.77"						
	0.0	9	0.2500	3.50		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
_	3.8	109	Total			·						

#### **Summary for Subcatchment Area 27: Area 27**

Runoff = 0.52 cfs @ 12.08 hrs, Volume= 0.023 af, Depth= 1.89"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription		
0.	.149 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	.149	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	12	0.2500	0.28	•	Sheet Flow,
0.5	199	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
1.2	211	Total			

# **Summary for Subcatchment Area 28: Area 28**

Runoff = 0.44 cfs @ 12.09 hrs, Volume= 0.020 af, Depth= 1.89"

Routed to Link F10: Flume 10

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Area (ac) CN Description									
0.126 69 Pasture/grassland/range, Fair, HSG B									
	0.126 100.00% Pervious Area								
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.5	59	0.2500	0.39		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"	

#### **Summary for Subcatchment Area 29: Area 29**

Runoff = 3.66 cfs @ 12.13 hrs, Volume= 0.19

0.194 af, Depth= 1.89"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

 Area	(ac) C	N Des	cription		
 1.	228 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	228	100.	00% Pervi	ous Area	
 Tc (min)	Length Slope (feet) (ft/ft)		Velocity (ft/sec)	Capacity (cfs)	Description
 3.8	100	0.2500	0.43		Sheet Flow,
0.0	6	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
 1.9	541	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding
5.7	647	Total			

# **Summary for Subcatchment Area 3: Area 3**

Runoff = 1.06 cfs @ 12.12 hrs, Volume= 0.055 af, Depth= 1.89"

Routed to Link F2A: Flume 2A

Area (ac)	CN	Description
0.348	69	Pasture/grassland/range, Fair, HSG B
0.348		100.00% Pervious Area

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	100	0.1950	0.39		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	36	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.4	162	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
					n= 0.030 Earth, grassed & winding
4.8	298	Total			

#### **Summary for Subcatchment Area 30: Area 30**

Runoff = 1.38 cfs @ 12.10 hrs, Volume=

0.067 af, Depth= 1.89"

Routed to Link F3: Flume 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription		
	0.	427 6	9 Past	ture/grassl	and/range,	Fair, HSG B
_	0.	427	100.	00% Pervi	ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.9	70	0.2500	0.40		Sheet Flow,
	0.8	227	0.0200	4.80	23.38	Grass: Short n= 0.150 P2= 2.77" <b>Trap/Vee/Rect Channel Flow, Existing Diversion Berm</b> Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding
	0.1	65	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
	3.8	362	Total		-	

# **Summary for Subcatchment Area 31: Area 31**

Runoff = 0.72 cfs @ 12.10 hrs, Volume= 0.035 af, Depth= 1.89"

Routed to Link F4A: Flume 4A

Area (ac)	CN	Description				
0.223	69	Pasture/grassland/range, Fair, HSG B				
0.223		100.00% Pervious Area				

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	77	0.2500	0.41		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.5	157	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow,
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
					n= 0.030 Earth, grassed & winding
0.1	62	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
3.7	296	Total			

# **Summary for Subcatchment Area 32: Area 32**

Runoff = 1.98 cfs @ 12.12 hrs, Volume= 0.103 af, Depth= 1.89"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription				
	0.	655 6	9 Past	ure/grassl	and/range,	Fair, HSG B		
	0.	655	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	. ,		Capacity (cfs)	Description		
_	3.8	100	0.2500	0.43	,	Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1 11		0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	8.0	314	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm		
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'		
	0.0	0.47	0.0500	40.00	444.40	n= 0.030 Earth, grassed & winding		
	0.3	217	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume		
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'		
_						n= 0.078 Riprap, 12-inch		
	5.0	642	Total					

# **Summary for Subcatchment Area 33: Area 33**

Runoff = 0.76 cfs @ 12.11 hrs, Volume= 0.037 af, Depth= 1.89"

Routed to Link F5A: Flume 5A

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	Area	(ac) C	N Des	cription			
	0.	237 6	9 Past	ture/grassl	and/range,	Fair, HSG B	
0.237 100.00% Pervious Area							
	Tc Leng		Slope (ft/ft)	, , ,		·	
	3.3	83	0.2500	0.42	, ,	Sheet Flow,	
					Grass: Short n= 0.150 P2= 2.77"		
	0.5	138	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm	
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'	
	0.2	107	0.2500	12.26	444 49	n= 0.030 Earth, grassed & winding	
	0.2	127	0.2500	12.20	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'	
						n= 0.078 Riprap, 12-inch	
-	4.0	348	Total				

#### **Summary for Subcatchment Area 34: Area 34**

Runoff = 1.73 cfs @ 12.10 hrs, Volume= 0.081 af, Depth= 2.29"

Routed to Reach S5.2: Swale S5.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area (ac) CN Description  0.346 69 Pasture/grassland/range, Fair, HSG B									
0.078 96 Gravel surface, HSG A									
_	0.424 74 Weighted Average								
	0.424 100.00% Pervious Area								
	Tc	Length	Slope	Velocity	Capacity	Description			
(min) (feet) (ft/ft) (ft/sec) (cfs)									
	3.1	78	0.2500	0.41		Sheet Flow,			
						Grass: Short	n= 0 150	P2= 2 77"	

# **Summary for Subcatchment Area 35: Area 35**

Runoff = 4.05 cfs @ 12.11 hrs, Volume= 0.204 af, Depth= 2.46" Routed to Reach S5.1 : Swale S5.1

Area (ac)	CN	Description					
0.745	0.745 69 Pasture/grassland/range, Fair, HSG B						
0.248	96	Gravel surface, HSG A					
0.993	76	Weighted Average					
0.993		100.00% Pervious Area					

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.3	70	0.1736	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	30	0.0050	0.58		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.1	12	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	4.4	126	Total			

#### **Summary for Subcatchment Area 36: Area 36**

Runoff = 14.85 cfs @ 12.08 hrs, Volume= 0.689 af, Depth= 3.19" Routed to Reach S1.0 : Swale S1.0

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription					
	1.	120 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
1.474 96 Gravel surface, HSG A									
	2.	594 8	34 Weid	hted Aver	age				
	2.	594		, 00% Pervi					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	1.5	100	0.0140	1.11		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 2.77"			
	0.2	32	0.0140	2.40		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.5	40	0.0050	1.44		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.0	11	0.5000	4.95		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	2.2	183	Total						

#### **Summary for Subcatchment Area 37: Area 37**

Runoff = 0.24 cfs @ 12.14 hrs, Volume= 0.017 af, Depth= 0.66"

Routed to Reach S1.1: Swale S1.1

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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	Area	(ac) C	N Desc	cription		
	0.	249 3	9 Past	ure/grassl	and/range,	Good, HSG A
_	0.	.057 9	96 Grav	el surface	, HSG A	
	0.	306 5	50 Weig	ghted Aver	age	
	0.	306	100.	00% Pervi	ous Area	
	_		0.1			B
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.4	72	0.1736	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	8.0	28	0.0050	0.57		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.2	19	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	4.5	133	Total			

#### **Summary for Subcatchment Area 38: Area 38**

Runoff = 3.58 cfs @ 12.11 hrs, Volume= 0.181 af, Depth= 1.52"

Routed to Reach S1.3: Swale S1.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription			
0.395 39 Pasture/grassland/range, Good, HSG A							
0.886 69 Pasture/grassland/range, Fair, HSG						Fair, HSG B	
_	0.149 96 Gravel surface, HSG A						
	1.430 64 Weighted Average						
	1.	430	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.8	100	0.2500	0.43		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.3	56	0.2500	3.50		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	4.1	156	Total				

#### **Summary for Subcatchment Area 39: Area 39**

Runoff = 1.74 cfs @ 12.11 hrs, Volume= 0.090 af, Depth= 1.45"

Routed to Reach S1.4: Swale S1.4

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	Area	(ac) C	N Des	cription					
	0.225 39 Pasture/grassland/range, Good, HSG A								
0.436 69 Pasture/grassland/range, Fair, HSG B									
_	0.	081 9	<u>6 Grav</u>	∕el surface	, HSG A				
	0.742 63 Weighted Average								
	0.	742	100.	00% Pervi	ous Area				
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.3	23	0.0050	1.44		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.2	22	0.0833	2.02		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
Ī	4.3	145	Total						

# **Summary for Subcatchment Area 4: Area 4**

Runoff = 0.92 cfs @ 12.11 hrs, Volume= 0.0

0.045 af, Depth= 1.89"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription				
	0.288 69 Pasture/grassland/range, Fair, HSG B							
	0.	288	100.	00% Pervi	ous Area			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
	3.4	49	0.0820	0.24		Sheet Flow,		
	0.6	247	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
	4.0	296	Total					

# **Summary for Subcatchment Area 40: Area 40**

Runoff = 0.72 cfs @ 12.19 hrs, Volume= 0.051 af, Depth= 1.00" Routed to Reach S1.4 : Swale S1.4

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	Area	(ac) C	N Desc	cription		
	0.	317 3	9 Past	ure/grassl	and/range,	Good, HSG A
	0.	243 6				Fair, HSG B
_	0.	<u>060 9</u>	<u>6 Grav</u>	<u>el surface</u>	, HSG A	
				ghted Aver		
	0.	620	100.	00% Pervi	ous Area	
	_		01		0 "	D
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.2	81	0.0245	0.16		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.0	19	0.2500	0.31		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	29	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	20	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	19	0.1053	2.27		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	9.6	168	Total			

# **Summary for Subcatchment Area 41: Area 41**

Runoff = 0.98 cfs @ 12.16 hrs, Volume= 0.061 af, Depth= 1.00"

Routed to Reach S1.5 : Swale S1.5

Area	(ac) C	CN Des	cription		
0	.389				Good, HSG A
0	.270	69 Past	ture/grassl	and/range,	Fair, HSG B
0	.080	96 Grav	vel surface	, HSG A	
0	.739	56 Wei	ghted Aver	age	
0	.739		.00% Pervi		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
4.5	49	0.0408	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
2.1	47	0.2500	0.37		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	4	0.0050	0.39		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 2.77"
0.2	19	0.0050	1.44		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.2	22	0.1136	2.36		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	141	Total			

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# Summary for Subcatchment Area 42a: Area 42a

Runoff = 2.00 cfs @ 12.16 hrs, Volume= 0.116 af, Depth= 1.59"

Routed to Reach S1.7: Swale S1.7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac)	CN	Desc	cription				
0	0.249 39 Pasture/grassland/range, Good, HSG A							
0.489 69 Pasture/grassland/range, Fair, HSG B								
0	.133	96	Grav	el surface	, HSG A			
0	0.871 65 Weighted Average							
0	.871		100.	00% Pervi	ous Area			
Tc	Lengt	h S	Slope	Velocity	Capacity	Description		
(min)	(feet	()	(ft/ft)	(ft/sec)	(cfs)			
7.3	10	0.0	0500	0.23		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
0.5	4	4 0.0	0500	1.57		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
7.8	14	4 To	otal					

# Summary for Subcatchment Area 42b: Area 42b

Runoff = 2.19 cfs @ 12.15 hrs, Volume= 0.122 af, Depth= 2.05"

Routed to Reach S1.8: Swale S1.8

	Area	(ac) C	N Des	cription			
	0.655 69 Pasture/grassland/range, Fair, HSG B						
	0.057 96 Gravel surface, HSG A						
_					•		
	0.	712 7	′1 Wei	ghted Aver	age		
	0.	712	100.	00% Pervi	ous Area		
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u> </u>	
	7.3	100	0.0500	0.23		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.0	2	0.0500	1.57		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
_	7.3	102	Total			<u> </u>	

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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# **Summary for Subcatchment Area 43: Area 43**

Runoff = 2.14 cfs @ 12.15 hrs, Volume= 0.121

0.121 af, Depth= 1.89"

Routed to Reach S1.6: Swale S1.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription				
0.769 69 Pasture/grassland/range, Fair, HSG B								
_	0.769 100.00% Pervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
-	7.3	100	0.0500	0.23	, ,	Sheet Flow,		
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	7 7	139	Total	•				

#### Summary for Subcatchment Area 44: Area 44

Runoff = 0.06 cfs @ 13.20 hrs, Volume= 0.022 af, Depth= 0.18"

Routed to Reach S2.1 : Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription		
	1.	416 3	9 Past	ure/grassl	and/range,	Good, HSG A
_	1.	416	100.	00% Pervi	ous Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	9.5	100	0.0260	0.18		Sheet Flow,
	0.0	404	0.0000	4.40		Grass: Short n= 0.150 P2= 2.77"
	2.9	194	0.0260	1.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
_	9.6	647	0.0260	1.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	22.0	941	Total			

# Summary for Subcatchment Area 45: Area 45

Runoff = 3.88 cfs @ 12.25 hrs, Volume= 0.306 af, Depth= 1.32"

Routed to Reach S1.9: Swale S1.9

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Area	(ac) C	N Desc	cription		
0.	797 3	9 Past	ure/grassl	and/range,	Good, HSG A
					Fair, HSG B
0.	057 9	<u>6 Grav</u>	∕el surface	, HSG A	
			ghted Aver		
2.	792	100.	00% Pervi	ous Area	
_		01			
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.3	100	0.0500	0.23		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.3	119	0.0500	1.57		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.1	22	0.1905	3.06		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
6.0	178	0.0050	0.49		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
14.7	419	Total			

#### **Summary for Subcatchment Area 46: Area 46**

Runoff = 6.52 cfs @ 12.11 hrs, Volume= 0.322 af, Depth= 1.89"

Routed to Reach S3.1 : Swale S3.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription					
	2.044 69 Pasture/grassland/range, Fair, HSG B								
	2.	044	100.	00% Pervi	ous Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	3.8	100	0.2500	0.43	· · · · · ·	Sheet Flow,			
	0.2	38	0.2632	3.59		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	4.0	138	Total						

### **Summary for Subcatchment Area 47: Area 47**

Runoff = 1.53 cfs @ 12.11 hrs, Volume= 0.075 af, Depth= 1.97"

Routed to Reach S3.2: Swale S3.2

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	Area	(ac) C	N Des	Description							
	0.	435	69 Past	Pasture/grassland/range, Fair, HSG B							
	0.	022	96 Grav	el surface	, HSG A						
	0.457 70 Weighted Average										
	0.	457	100.	00% Pervi	ous Area						
	Tc	Length	•	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	3.8	100	0.2500	0.43		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.77"					
	0.1	22	0.2500	3.50		Shallow Concentrated Flow,					
_						Short Grass Pasture Kv= 7.0 fps					
	3.9	122	Total								

### **Summary for Subcatchment Area 48: Area 48**

Runoff = 3.99 cfs @ 12.15 hrs, Volume= 0.220 af, Depth= 2.21"

Routed to Reach S3.4 : Swale S3.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac)	CN	Desc	Description							
1.	.031	69	Past	ure/grassla	and/range,	Fair, HSG B					
0.163 96 Gravel surface, HSG A											
1.	1.194 73 Weighted Average										
1.	.194		100.	00% Pervi	ous Area						
Tc	Lengt	h S	Slope	Velocity	Capacity	Description					
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)						
7.2	9	8 0.	0500	0.23		Sheet Flow,					
						Grass: Short	n= 0.150	P2= 2.77"			

# **Summary for Subcatchment Area 49: Area 49**

Runoff = 0.27 cfs @ 12.12 hrs, Volume= 0.014 af, Depth= 2.13"

Routed to Link C6 : Culvert C6

 Area (ac)	CN	Description					
0.070	69	Pasture/grassland/range, Fair, HSG B					
 0.009	96	Gravel surface, HSG A					
 0.079	72	Weighted Average					
0.079		100.00% Pervious Area					

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т.	Longth	Clana	\/alaaity	Canacity	Description
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.5	90	0.2500	0.43		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.2	10	0.0500	0.14		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.1	10	0.0500	1.57		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.2	15	0.0050	1.44		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.1	18	0.1390	2.61		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
5.1	143	Total	·	·	

#### **Summary for Subcatchment Area 5: Area 5**

Runoff = 2.87 cfs @ 12.14 hrs, Volume= 0.155 af, Depth= 1.89"

Routed to Link F4A: Flume 4A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	0.986 69 Pasture/grassland/range,		and/range,	Fair, HSG B		
	0.986		100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	2.2	31	0.1000	0.24	,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	2.8	69	0.2500	0.40		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.0	9	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.5	419	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
	0.4	00	0.0500	40.00	444.40	n= 0.030 Earth, grassed & winding
	0.1	80	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
_						n= 0.078 Riprap, 12-inch
	6.6	608	Total			

# **Summary for Subcatchment Area 50: Area 50**

Runoff = 9.80 cfs @ 12.20 hrs, Volume= 0.636 af, Depth= 2.05"

Routed to Reach S3.5 : Swale S3.5

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	Area (ac) CN		N Desc	cription				
	3.470 69			Pasture/grassland/range, Fair, HSG B				
	0.	256 g	96 Grav	<u>el surface</u>	, HSG A		_	
	3.	726 7	'1 Weig	ghted Aver	age			
	3.	726	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
(	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_	
	7.3	100	0.0500	0.23		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.9	83	0.0500	1.57		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	2.9	208	0.0289	1.19		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps	_	
	11.1	391	Total					

#### **Summary for Subcatchment Area 51: Area 51**

Runoff = 0.03 cfs @ 12.51 hrs, Volume= 0.011 af, Depth= 0.18"

Routed to Reach S2.1: Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Are	a (ac)	CN	Desc	cription						
0.698 39 Pasture/grassland/range, Good, HSG A										
0.698 100.00% Pervious Area										
T (min	5		lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.8	3 100	0.0	0600	0.25		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"		

# **Summary for Subcatchment Area 52: Area 52**

Runoff = 1.48 cfs @ 12.11 hrs, Volume= 0.075 af, Depth= 1.89"

Routed to Link F5B: Flume 5B

 Area (ac)	CN	Description
0.475	69	Pasture/grassland/range, Fair, HSG B
0.475		100.00% Pervious Area

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.5	90	0.2500	0.43		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
8.0	219	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
					n= 0.030 Earth, grassed & winding
0.2	116	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
4.5	425	Total			

# **Summary for Subcatchment Area 53: Area 53**

Runoff = 2.98 cfs @ 12.12 hrs, Volume= 0.160 af, Depth= 1.18" Routed to Reach S2.3 : Swale S2.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
1.079 69 Pasture/grassland/range,						Fair, HSG B
_	0.	539 3	39 Past	ture/grassl	and/range,	Good, HSG A
1.618 59 Weighted Average						
	1.	618	100.	00% Pervi	ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.1	77	0.2500	0.41	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	8.0	237	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding
	0.3	70	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	4.2	384	Total			

# Summary for Subcatchment Area 54: Area 54

Runoff = 1.45 cfs @ 12.11 hrs, Volume= 0.077 af, Depth= 1.12" Routed to Reach S2.2 : Swale S2.2

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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	Area (ac) CN Description								
0.520 69 Pasture/grassland/range, Fair, HSG B									
0.306 39 Pasture/grassland/range, Good, HSG A									
0.826 58 Weighted Average									
0.826 100.00% Pervious Area									
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_		
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	26	0.2500	3.50		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
_	3.9	126	Total				_		

### **Summary for Subcatchment Area 55: Area 55**

Runoff = 1.86 cfs @ 12.12 hrs, Volume= 0.102 a

0.102 af, Depth= 1.12"

Routed to Reach S4.1: Swale S4.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	0.	526 3	39 Past	ure/grassl	and/range,	Good, HSG A
	0.	405 6	69 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	158 9	96 Grav	el surface	, HSG A	
	1.	089 5		ghted Aver		
	1.	089	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.5	80	0.2000	0.38		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	20	0.0500	1.34		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.1	10	0.0050	1.44		Shallow Concentrated Flow,
	0.5	40	0.0005	4 75		Paved Kv= 20.3 fps
	0.5	48	0.0625	1.75		Shallow Concentrated Flow,
-						Short Grass Pasture Kv= 7.0 fps
	4 3	158	Total			

#### **Summary for Subcatchment Area 56: Area 56**

Runoff = 2.17 cfs @ 12.12 hrs, Volume= 0.118 af, Depth= 1.18"

Routed to Reach S4.2 : Swale S4.2

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	Area	(ac)	CN Des	cription				
	0.	580	39 Pas	ture/grassl	and/range,	Good, HSG A		
	0.	433	69 Pas	ture/grassl	and/range,	Fair, HSG B		
0.181 96 Gravel surface, HSG B								
1.194 59 Weighted Average								
	1.	194	100	.00% Pervi				
	Тс	Length			Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.6	121	0.2314	3.37		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	4.4	221	Total	_	_			

### **Summary for Subcatchment Area 57: Area 57**

Runoff = 2.69 cfs @ 12.13 hrs, Volume= 0.162 af, Depth= 0.88"

Routed to Reach S4.3: Swale S4.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area (ac) CN Description									
	Good, HSG A								
_	1.	086	Fair, HSG B						
	2.	220	100	.00% Pervi	ous Area				
	Тс	Length	•		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	8.0	140	0.1857	3.02		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	4.6	240	Total						

# **Summary for Subcatchment Area 58: Area 58**

Runoff = 4.13 cfs @ 12.13 hrs, Volume= 0.231 af, Depth= 1.12"

Routed to Reach S4.4: Swale S4.4

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	Area	(ac) C	N Des	cription					
				Pasture/grassland/range, Good, HSG A					
1.537 69 Pasture/grassland/range, Fair, HSG B 2.476 58 Weighted Average									
	2.	476	•	00% Pervi	•				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	3.8	100	0.2500	0.43		Sheet Flow,	_		
_	0.9	163	0.1718	2.90		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	4.7	263	Total						

## **Summary for Subcatchment Area 59: Area 59**

Runoff = 1.90 cfs @ 12.12 hrs, Volume= 0.115 af, Depth= 0.82"

Routed to Reach S4.5: Swale S4.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
0.898 39 Pasture/grassland/range, Good, HSG A								
0.785 69 Pasture/grassland/range, Fair, HSG B								
	1.683 53 Weighted Average							
	1.	683	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.9	100	0.2345	0.42		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.2	46	0.2345	3.39		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
_	4.1	146	Total			•	_	

# Summary for Subcatchment Area 6: Area 6

Runoff = 1.66 cfs @ 12.10 hrs, Volume= 0.079 af, Depth= 1.89"

Routed to Link F6A: Flume 6A

_	Area (ac)	CN	Description
	0.504	69	Pasture/grassland/range, Fair, HSG B
Ī	0.504		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.9		0.2500	0.41		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.5	186	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
•	3.4	258	Total			, 5

### **Summary for Subcatchment Area 60: Area 60**

Runoff = 0.24 cfs @ 12.55 hrs, Volume=

0.055 af, Depth= 0.33"

Routed to Reach S4.6: Swale S4.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription					
				Pasture/grassland/range, Good, HSG A					
	_			el surface	•				
	0.	130	96 Grav	∕el surface	, HSG A				
	2.	001 4	l3 Weid	hted Aver	age				
	2.	001		00% Pervi					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•			
_	15.7	100	0.0074	0.11		Sheet Flow,	_		
						Grass: Short n= 0.150 P2= 2.77"			
	4.7	169	0.0074	0.60		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.4	49	0.0800	1.98		Shallow Concentrated Flow,			
	3					Short Grass Pasture Kv= 7.0 fps			
_	20.8	318	Total			•	_		

# **Summary for Subcatchment Area 61: Area 61**

Runoff = 0.09 cfs @ 12.53 hrs, Volume=

0.033 af, Depth= 0.18"

Routed to Reach S2.3: Swale S2.3

_	Area (ac)	CN	Description
	2.177	39	Pasture/grassland/range, Good, HSG A
_	2.177		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	66	0.0303	0.17		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.6	34	0.2500	0.35		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	49	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.2	149	Total			·

### **Summary for Subcatchment Area 62: Area 62**

Runoff = 0.39 cfs @ 12.18 hrs, Volume= 0.032 af, Depth= 0.66"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area (ac) CN Description						
0.479 39 Pasture/grassland/range						Good, HSG A	
0.115 96 Gravel surface, HSG A							_
	0.594 50 Weighted Average						
	0.	594	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'	
	7.1	100	0.0544	0.24		Sheet Flow,	_
						Grass: Short n= 0.150 P2= 2.77"	
	0.5	47	0.0544	1.63		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	7.6	147	Total			·	_

# **Summary for Subcatchment Area 63: Area 63**

Runoff = 10.73 cfs @ 12.04 hrs, Volume= 0.588 af, Depth= 4.67"

Routed to Pond Sed Pond : Sedimentation Basin

	Area	(ac)	CN	Desc	cription		
1.509 98 W				Wate	er Surface,	, HSG A	
	1.509 100.00% Impervious Area						a
	Tc (min)	Lengt		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	0.0						Direct Entry,

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### **Summary for Subcatchment Area 64: Area 64**

Runoff = 0.22 cfs @ 12.53 hrs, Volume= 0.079 af, Depth= 0.18"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
_	5.	227 3	9 Past	ture/grassl	and/range,	Good, HSG A
	5.	227	100.	00% Pervi	ous Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.5	75	0.0933	0.28		Sheet Flow,
	4.0	0.5	0.0500	0.00		Grass: Short n= 0.150 P2= 2.77"
	1.3	25	0.2500	0.33		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.0	10	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.8	381	0.0265	7.85	109.92	Trap/Vee/Rect Channel Flow, Bot.W=0.00' D=2.00' Z= 4.0 & 3.0 '/' Top.W=14.00'
	0.8	162	0.2500	3.50		n= 0.030 Earth, grassed & winding  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps
_	0.5	48	0.0500	1.57		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
_	7.9	701	Total			

# **Summary for Subcatchment Area 65: Area 65**

Runoff = 0.61 cfs @ 12.73 hrs, Volume= 0.128 af, Depth= 0.51"

Routed to Reach RD1: Roadside Ditch 1

 Area (ac)	CN	Description				
2.616	39	Pasture/grassland/range, Good, HSG A				
0.039	69	Pasture/grassland/range, Fair, HSG B				
 0.380	96	Gravel surface, HSG A				
 3.035	47	Weighted Average				
3.035		100.00% Pervious Area				

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u> </u>
16.2	100	0.0068	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
22.7	786	0.0068	0.58		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
38.9	886	Total			

# **Summary for Subcatchment Area 66: Area 66**

Runoff = 0.81 cfs @ 12.10 hrs, Volume= 0.040 af, Depth= 1.18"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area (	(ac)	CN	Desc	Description							
0.213 39 Pasture/grassland/range, Good, HSG A												
0.104 69 Pasture/grassland/range, Fair, HSG B												
	0.0	092	96	Grav	el surface	, HSG Å						
	0.409 59 Weighted Average											
	0.409 100.00% Pervious Area											
	Тс	Lengt	h	Slope	Velocity	Capacity	Description					
(n	nin)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)						
	3.1	5	2 0	).1154	0.28		Sheet Flow,					
							Grass: Short	n= 0.150	P2= 2.77"			

## **Summary for Subcatchment Area 67: Area 67**

Runoff = 0.71 cfs @ 12.12 hrs, Volume= 0.045 af, Depth= 0.71"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac)	CN	Desc	Description						
0.524 39 Pasture/grassland/range, Good, HSG A											
0.139 69 Pasture/grassland/range, Fair, HSG B							, Fair, HSG B				
	0.092 96 Gravel surface, HSG A							_			
	0.755 51 Weighted Average										
	0.755 100.00% Pervious Area										
	_		_								
	Tc	Lengt	h	Slope	Velocity	Capacity	·				
	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)		_			
	3.5	8	6 (	0.2326	0.41		Sheet Flow,				

Grass: Short n= 0.150 P2= 2.77"

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## **Summary for Subcatchment Area 68: Area 68**

Runoff = 1.52 cfs @ 12.12 hrs, Volume= 0.099 af, Depth= 0.71"

Routed to Reach RD4: Roadside Ditch 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) (	CN De	Description						
	1.016 39 Pasture/grassland/range, Good, HSG A									
	0.	620	69 Pas	ture/grassl	and/range,	Fair, HSG B				
	0.	035	96 Gra	vel surface	e, HSG A					
	1.671 51 Weighted Average									
	1.	671	100	.00% Perv	ious Area					
	Тс	Length	Slope	Velocity	Capacity	Description				
(	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.8	100	0.2500	0.43		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.77"				
	0.2	26	0.1538	2.75		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	4.0	126	Total		·					

## **Summary for Subcatchment Area 69: Area 69**

Runoff = 0.99 cfs @ 12.52 hrs, Volume= 0.238 af, Depth= 0.29"

Routed to Pond N: North Infiltration Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	Description					
_	9.	360 3	9 Past	Pasture/grassland/range, Good, HSG A					
	0.	515 9		Gravel surface, HSG A					
	9.	875 4	2 Wei	hted Aver	age				
	9.	875		, 00% Pervi					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	10.5	100	0.0200	0.16		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	1.7	100	0.0200	0.99		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	4.4	1,137	0.0193	4.31	32.30	Trap/Vee/Rect Channel Flow, Roadside Ditch			
						Bot.W=0.00' D=1.00' Z= 5.0 & 10.0 '/' Top.W=15.00'			
_						n= 0.030			
	166	1 227	Total						

16.6 1,337 Total

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## **Summary for Subcatchment Area 7: Area 7**

Runoff = 2.81 cfs @ 12.13 hrs, Volume= 0.148 af, Depth= 1.89"

Routed to Link F6A: Flume 6A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Are	ea (	(ac) C	N Desc	cription		
	0.9	936 6	69 Pasture/grassla		and/range,	Fair, HSG B
•	0.9	936	100.	00% Pervi	ous Area	
T (min)	Гс n)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4	.1	100	0.2070	0.40		Sheet Flow,
0	.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1	.0	395	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5	.3	541	Total			

## **Summary for Subcatchment Area 70: Area 70**

Runoff = 1.94 cfs @ 12.11 hrs, Volume= 0.096 af, Depth= 1.67"

Routed to Reach S1.2 : Swale S1.2

_	Area	(ac) C	N Desc	cription					
	0.153 39 Pasture/grassland/range, Good, HSG A								
	0.441 69 Pasture/grassland/range, Fair, HSG B								
_	0.100 96 Gravel surface, HSG A								
	0.694 66 Weighted Average								
	0.694 100.00% Pervious Area								
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	27	0.2500	3.50		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
Ī	3.9	127	Total						

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## **Summary for Subcatchment Area 8: Area 8**

Runoff = 6.16 cfs @ 12.14 hrs, Volume= 0.333 af, Depth= 1.89"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription		
	2.	114 6	9 Past	ture/grassl	and/range,	Fair, HSG B
_	2.	114	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	5.5	100	0.1000	0.30		Sheet Flow,
	0.2	26	0.1000	2.21		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	121	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.3	124	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
Ī	6.6	371	Total			

## **Summary for Subcatchment Area 9: Area 9**

Runoff = 2.37 cfs @ 12.14 hrs, Volume= 0.128 af, Depth= 1.89"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription			
0.810 69 Pasture/grassland/range, Fair, HSG B							
_	0.	810	100.	00% Pervi	ious Area		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.2	71	0.1000	0.28		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	1.4	29	0.2500	0.34		Sheet Flow,	
	0.4	90	0.2500	2.50		Grass: Short n= 0.150 P2= 2.77"	
	0.4	89	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	0.3	103	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
	63	202	Total				

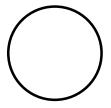
6.3 292 Total

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### Summary for Reach 53R: Culvert C1

Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 36.48 cfs

24.0" Round Pipe n= 0.012 Corrugated PP, smooth interior Length= 51.9' Slope= 0.0222 '/' Inlet Invert= 815.70', Outlet Invert= 814.55'



## **Summary for Reach RD1: Roadside Ditch 1**

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 0.51" for 25-yr, 24-hr event

Inflow = 0.61 cfs @ 12.73 hrs, Volume= 0.128 af

Outflow = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af, Atten= 2%, Lag= 9.5 min

Routed to Link C1: Culvert C1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.34 fps, Min. Travel Time= 5.5 min Avg. Velocity = 0.62 fps, Avg. Travel Time= 11.8 min

Peak Storage= 197 cf @ 12.80 hrs

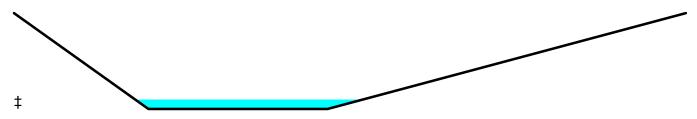
Average Depth at Peak Storage= 0.10', Surface Width= 5.08' Bank-Full Depth= 1.00' Flow Area= 9.5 sf, Capacity= 47.16 cfs

 $4.00' \times 1.00'$  deep channel, n= 0.030

Side Slope Z-value= 3.0 8.0 '/' Top Width= 15.00'

Length= 440.6' Slope= 0.0188 '/'

Inlet Invert= 824.00', Outlet Invert= 815.70'



# Summary for Reach RD2: Roadside Ditch 2

Inflow Area = 7.168 ac, 0.00% Impervious, Inflow Depth = 1.40" for 25-yr, 24-hr event

Inflow = 11.19 cfs @ 12.19 hrs, Volume= 0.838 af

Outflow = 10.57 cfs @ 12.26 hrs, Volume= 0.838 af, Atten= 6%, Lag= 4.5 min

Routed to Link C10: Culvert C10

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Routing by Stor-Ind+Trans method. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs

Max. Velocity= 2.70 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.78 fps, Avg. Travel Time= 9.2 min

Peak Storage= 1,703 cf @ 12.22 hrs

Average Depth at Peak Storage= 0.40', Surface Width= 13.91'

Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 72.77 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 433.0' Slope= 0.0162 '/'

Inlet Invert= 814.55', Outlet Invert= 807.54'



## Summary for Reach RD3: Roadside Ditch 3

1.552 ac, 0.00% Impervious, Inflow Depth = 2.64" for 25-yr, 24-hr event Inflow Area =

6.95 cfs @ 12.10 hrs, Volume= Inflow 0.341 af

5.77 cfs @ 12.20 hrs, Volume= 0.341 af, Atten= 17%, Lag= 5.9 min Outflow

Routed to Link C2: Culvert C2a

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.79 fps, Min. Travel Time= 3.6 min

Avg. Velocity = 1.26 fps, Avg. Travel Time= 10.8 min

Peak Storage= 1,282 cf @ 12.14 hrs

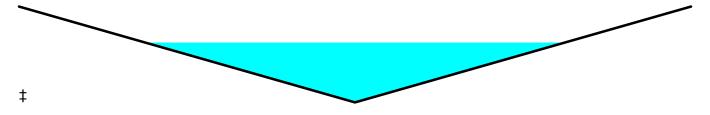
Average Depth at Peak Storage= 0.62', Surface Width= 5.00' Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 20.76 cfs

 $0.00' \times 1.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

Length= 821.0' Slope= 0.0288 '/'

Inlet Invert= 841.47', Outlet Invert= 817.83'



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## Summary for Reach RD4: Roadside Ditch 4

Inflow Area = 1.671 ac, 0.00% Impervious, Inflow Depth = 0.71" for 25-yr, 24-hr event

Inflow = 1.52 cfs @ 12.12 hrs, Volume= 0.099 af

Outflow = 0.92 cfs @ 12.35 hrs, Volume= 0.099 af, Atten= 39%, Lag= 13.6 min

Routed to Link C10: Culvert C10

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.05 fps, Min. Travel Time= 7.9 min

Avg. Velocity = 0.37 fps, Avg. Travel Time= 22.6 min

Peak Storage= 447 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.12', Surface Width= 8.49' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 54.26 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 495.6' Slope= 0.0090 '/'

Inlet Invert= 812.00', Outlet Invert= 807.54'



## Summary for Reach RD5: Roadside Ditch 5

Inflow Area = 0.616 ac, 0.00% Impervious, Inflow Depth = 2.13" for 25-yr, 24-hr event

Inflow = 2.25 cfs @ 12.10 hrs, Volume= 0.109 af

Outflow = 2.01 cfs @ 12.14 hrs, Volume= 0.109 af, Atten= 11%, Lag= 2.5 min

Routed to Reach S4.2 : Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.65 fps, Min. Travel Time= 1.3 min

Avg. Velocity = 1.38 fps, Avg. Travel Time= 3.5 min

Peak Storage= 164 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.38', Surface Width= 3.02'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 28.18 cfs

0.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

Length= 288.0' Slope= 0.0531 '/'

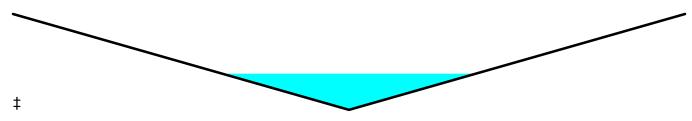
Inlet Invert= 841.47', Outlet Invert= 826.18'

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#### Summary for Reach S1.0: Swale S1.0

Inflow Area = 8.783 ac, 0.00% Impervious, Inflow Depth = 2.27" for 25-yr, 24-hr event

Inflow = 30.93 cfs @ 12.10 hrs, Volume= 1.665 af

Outflow = 26.03 cfs @ 12.21 hrs, Volume= 1.665 af, Atten= 16%, Lag= 6.2 min

Routed to Reach S1.2: Swale S1.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.44 fps, Min. Travel Time= 3.8 min

Avg. Velocity = 0.56 fps, Avg. Travel Time= 16.5 min

Peak Storage= 6,031 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.93', Surface Width= 15.46'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 118.85 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 551.3' Slope= 0.0039 '/'

Inlet Invert= 823.92', Outlet Invert= 821.75'



## Summary for Reach S1.1: Swale S1.1

Inflow Area = 0.306 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr, 24-hr event

Inflow = 0.24 cfs @ 12.14 hrs, Volume= 0.017 af

Outflow = 0.22 cfs @ 12.20 hrs, Volume= 0.017 af, Atten= 10%, Lag= 3.5 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs

Max. Velocity= 0.99 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 0.72 fps, Avg. Travel Time= 2.3 min

Peak Storage= 24 cf @ 12.16 hrs

Average Depth at Peak Storage= 0.03', Surface Width= 8.24'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 376.93 cfs

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8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 98.0' Slope= 0.0396 '/' Inlet Invert= 825.88', Outlet Invert= 822.00'



#### Summary for Reach S1.2: Swale S1.2

Inflow Area = 9.783 ac, 0.00% Impervious, Inflow Depth = 2.18" for 25-yr, 24-hr event

Inflow = 27.11 cfs @ 12.20 hrs, Volume= 1.778 af

Outflow = 26.46 cfs @ 12.22 hrs, Volume= 1.778 af, Atten= 2%, Lag= 1.0 min

Routed to Link C11: Culvert C11

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.60 fps, Min. Travel Time= 0.6 min

Avg. Velocity = 1.08 fps, Avg. Travel Time= 2.6 min

Peak Storage= 998 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.57', Surface Width= 12.57' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 294.55 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 170.0' Slope= 0.0242 '/'

Inlet Invert= 821.79', Outlet Invert= 817.68'



## Summary for Reach S1.3: Swale S1.3

Inflow Area = 11.213 ac, 0.00% Impervious, Inflow Depth = 2.10" for 25-yr, 24-hr event

Inflow = 27.98 cfs @ 12.21 hrs, Volume= 1.960 af

Outflow = 27.48 cfs @ 12.24 hrs, Volume= 1.960 af, Atten= 2%, Lag= 1.3 min

Routed to Link C3: Culvert C3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.94 fps, Min. Travel Time= 0.7 min

Avg. Velocity = 1.19 fps, Avg. Travel Time= 3.0 min

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Peak Storage= 1,194 cf @ 12.22 hrs

Average Depth at Peak Storage= 0.55', Surface Width= 12.40' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 324.28 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 212.6' Slope= 0.0293 '/'

Inlet Invert= 817.40', Outlet Invert= 811.17'



#### Summary for Reach S1.4: Swale S1.4

Inflow Area = 15.242 ac, 0.00% Impervious, Inflow Depth = 1.99" for 25-yr, 24-hr event

Inflow = 32.89 cfs @ 12.20 hrs, Volume= 2.521 af

Outflow = 32.85 cfs @ 12.21 hrs, Volume= 2.521 af, Atten= 0%, Lag= 0.6 min

Routed to Link C4: Culvert C4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.09 fps, Min. Travel Time= 0.3 min Avg. Velocity = 1.00 fps, Avg. Travel Time= 1.2 min

Peak Storage= 580 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.73', Surface Width= 13.88'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 227.36 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 72.2' Slope= 0.0144 '/'

Inlet Invert= 810.90', Outlet Invert= 809.86'



### **Summary for Reach S1.5: Swale S1.5**

Inflow Area = 15.981 ac, 0.00% Impervious, Inflow Depth = 1.94" for 25-yr, 24-hr event

Inflow = 33.64 cfs @ 12.21 hrs, Volume= 2.583 af

Outflow = 33.34 cfs @ 12.23 hrs, Volume= 2.583 af, Atten= 1%, Lag= 1.5 min

Routed to Reach S1.7: Swale S1.7

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.73 fps, Min. Travel Time= 0.9 min

Avg. Velocity = 0.66 fps, Avg. Travel Time= 3.7 min

Peak Storage= 1,830 cf @ 12.22 hrs

Average Depth at Peak Storage= 1.02', Surface Width= 16.18' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 126.50 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 148.0' Slope= 0.0045 '/' Inlet Invert= 809.60', Outlet Invert= 808.94'



#### Summary for Reach S1.6: Swale S1.6

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 2.14 cfs @ 12.15 hrs, Volume= 0.121 af

Outflow = 1.95 cfs @ 12.23 hrs, Volume= 0.121 af, Atten= 9%, Lag= 4.7 min

Routed to Reach S1.7 : Swale S1.7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.12 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.32 fps, Avg. Travel Time= 9.2 min

Peak Storage= 316 cf @ 12.19 hrs

Average Depth at Peak Storage= 0.20', Surface Width= 9.60'

Bank-Full Depth= 2.00', Flow Area= 32.0 st. Capacity= 134.06 ct

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 134.06 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 179.7' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.95'



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## Summary for Reach S1.7: Swale S1.7

Inflow Area = 18.333 ac, 0.00% Impervious, Inflow Depth = 1.93" for 25-yr, 24-hr event

Inflow = 38.89 cfs @ 12.22 hrs, Volume= 2.941 af

Outflow = 38.11 cfs @ 12.27 hrs, Volume= 2.941 af, Atten= 2%, Lag= 2.7 min

Routed to Link C5: Culvert C5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.05 fps, Min. Travel Time= 1.4 min

Avg. Velocity = 0.74 fps, Avg. Travel Time= 5.7 min

Peak Storage= 3,186 cf @ 12.25 hrs

Average Depth at Peak Storage= 1.04', Surface Width= 16.32'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 139.68 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 252.0' Slope= 0.0054 '/'

Inlet Invert= 808.94', Outlet Invert= 807.57'



## **Summary for Reach S1.8: Swale S1.8**

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 2.05" for 25-yr, 24-hr event

Inflow = 2.19 cfs @ 12.15 hrs, Volume= 0.122 af

Outflow = 1.99 cfs @ 12.21 hrs, Volume= 0.122 af, Atten= 9%, Lag= 3.8 min

Routed to Reach S1.7: Swale S1.7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.88 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.56 fps, Avg. Travel Time= 7.3 min

Peak Storage= 265 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.31', Surface Width= 4.38'

Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 90.14 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 245.8' Slope= 0.0099 '/'

Inlet Invert= 810.00', Outlet Invert= 807.57'

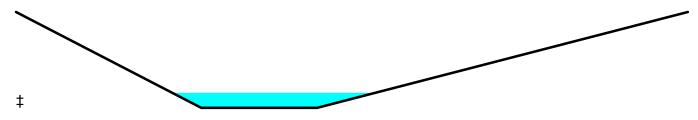
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#### Summary for Reach S1.9: Swale S1.9

Inflow Area = 21.125 ac, 0.00% Impervious, Inflow Depth = 1.84" for 25-yr, 24-hr event

Inflow = 41.96 cfs @ 12.27 hrs, Volume= 3.247 af

Outflow = 40.72 cfs @ 12.33 hrs, Volume= 3.247 af, Atten= 3%, Lag= 3.8 min

Routed to Reach S2.1: Swale S2.1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.48 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.84 fps, Avg. Travel Time= 8.4 min

Peak Storage= 5,006 cf @ 12.30 hrs

Average Depth at Peak Storage= 0.99', Surface Width= 15.93'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 163.67 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 422.0' Slope= 0.0075 '/'

Inlet Invert= 807.15', Outlet Invert= 804.00'



## Summary for Reach S2.1: Swale S2.1

Inflow Area = 23.239 ac, 0.00% Impervious, Inflow Depth = 1.69" for 25-yr, 24-hr event

Inflow = 40.74 cfs @ 12.33 hrs, Volume= 3.279 af

Outflow = 39.80 cfs @ 12.40 hrs, Volume= 3.279 af, Atten= 2%, Lag= 3.9 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs

Max. Velocity= 3.22 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.86 fps, Avg. Travel Time= 7.5 min

Peak Storage= 4,870 cf @ 12.36 hrs

Average Depth at Peak Storage= 1.34', Surface Width= 14.71'

Bank-Full Depth= 2.00' Flow Area= 24.0 sf, Capacity= 97.05 cfs

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4.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 20.00' Length= 389.0' Slope= 0.0054 '/' Inlet Invert= 806.10', Outlet Invert= 804.00'



### Summary for Reach S2.2: Swale S2.2

Inflow Area = 40.438 ac, 0.00% Impervious, Inflow Depth = 1.79" for 25-yr, 24-hr event

Inflow = 66.42 cfs @ 12.32 hrs, Volume= 6.023 af

Outflow = 65.44 cfs @ 12.38 hrs, Volume= 6.023 af, Atten= 1%, Lag= 3.7 min

Routed to Reach S2.3: Swale S2.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 3.34 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.80 fps, Avg. Travel Time= 8.5 min

Peak Storage= 8,100 cf @ 12.35 hrs

Average Depth at Peak Storage= 1.30', Surface Width= 20.38' Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 152.61 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 411.0' Slope= 0.0049 '/' Inlet Invert= 804.00', Outlet Invert= 802.00'



## Summary for Reach S2.3: Swale S2.3

Inflow Area = 44.233 ac, 0.00% Impervious, Inflow Depth = 1.69" for 25-yr, 24-hr event

Inflow = 66.33 cfs @ 12.38 hrs, Volume= 6.216 af

Outflow = 65.84 cfs @ 12.41 hrs, Volume= 6.216 af, Atten= 1%, Lag= 2.0 min

Routed to Link C7: Culvert C7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 4.74 fps, Min. Travel Time= 1.1 min

Avg. Velocity = 1.15 fps, Avg. Travel Time= 4.5 min

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Peak Storage= 4,294 cf @ 12.40 hrs

Average Depth at Peak Storage= 1.00', Surface Width= 17.99'

Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 249.72 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 26.00'

Length= 307.0' Slope= 0.0130 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



#### Summary for Reach S3.1: Swale S3.1

Inflow Area = 2.044 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 6.52 cfs @ 12.11 hrs, Volume= 0.322 af

Outflow = 5.46 cfs @ 12.21 hrs, Volume= 0.322 af, Atten= 16%, Lag= 5.9 min

Routed to Reach S3.2: Swale S3.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.62 fps, Min. Travel Time= 3.7 min

Avg. Velocity = 0.42 fps, Avg. Travel Time= 14.2 min

Peak Storage= 1,242 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.37', Surface Width= 10.94'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.76 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 357.0' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.07'



### Summary for Reach S3.2: Swale S3.2

Inflow Area = 7.661 ac, 0.00% Impervious, Inflow Depth = 1.90" for 25-yr, 24-hr event

Inflow = 20.49 cfs @ 12.14 hrs, Volume= 1.211 af

Outflow = 20.35 cfs @ 12.15 hrs, Volume= 1.211 af, Atten= 1%, Lag= 0.4 min

Routed to Reach S3.3: Swale S3.3

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.46 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.65 fps, Avg. Travel Time= 0.9 min

Peak Storage= 285 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.76', Surface Width= 14.07'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.95 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 34.0' Slope= 0.0050 '/'

Inlet Invert= 798.00', Outlet Invert= 797.83'



#### Summary for Reach S3.3: Swale S3.3

Inflow Area = 7.661 ac, 0.00% Impervious, Inflow Depth = 1.90" for 25-yr, 24-hr event

Inflow = 20.35 cfs @ 12.15 hrs, Volume= 1.211 af

Outflow = 19.21 cfs @ 12.17 hrs, Volume= 1.211 af, Atten= 6%, Lag= 1.7 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.39 fps, Min. Travel Time= 1.0 min

Avg. Velocity = 0.89 fps, Avg. Travel Time= 3.7 min

Peak Storage= 1,187 cf @ 12.16 hrs

Average Depth at Peak Storage= 0.58', Surface Width= 12.61'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 215.99 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 200.0' Slope= 0.0130 '/'

Inlet Invert= 808.00', Outlet Invert= 805.40'



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### Summary for Reach S3.4: Swale S3.4

Inflow Area = 1.194 ac, 0.00% Impervious, Inflow Depth = 2.21" for 25-yr, 24-hr event

Inflow = 3.99 cfs @ 12.15 hrs, Volume= 0.220 af

Outflow = 3.61 cfs @ 12.22 hrs, Volume= 0.220 af, Atten= 9%, Lag= 4.1 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.99 fps, Min. Travel Time= 2.4 min

Avg. Velocity = 0.59 fps, Avg. Travel Time= 8.0 min

Peak Storage= 520 cf @ 12.17 hrs

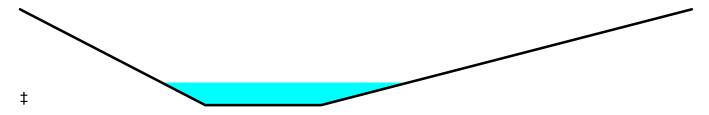
Average Depth at Peak Storage= 0.47', Surface Width= 5.32' Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 76.21 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 283.0' Slope= 0.0071 '/'

Inlet Invert= 810.00', Outlet Invert= 808.00'



## Summary for Reach S3.5: Swale S3.5

Inflow Area = 15.661 ac, 0.00% Impervious, Inflow Depth = 1.96" for 25-yr, 24-hr event

Inflow = 39.25 cfs @ 12.17 hrs, Volume= 2.554 af

Outflow = 37.05 cfs @ 12.24 hrs, Volume= 2.554 af, Atten= 6%, Lag= 4.5 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.27 fps, Min. Travel Time= 2.3 min

Avg. Velocity = 0.60 fps, Avg. Travel Time= 8.9 min

Peak Storage= 5,295 cf @ 12.20 hrs

Average Depth at Peak Storage= 1.27', Surface Width= 18.17'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 93.14 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 318.5' Slope= 0.0024 '/'

Inlet Invert= 804.76', Outlet Invert= 803.99'

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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#### Summary for Reach S4.1: Swale S4.1

Inflow Area = 1.364 ac, 0.00% Impervious, Inflow Depth = 1.28" for 25-yr, 24-hr event

Inflow = 2.74 cfs @ 12.10 hrs, Volume= 0.145 af

Outflow = 2.38 cfs @ 12.18 hrs, Volume= 0.145 af, Atten= 13%, Lag= 4.4 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.62 fps, Min. Travel Time= 2.5 min

Avg. Velocity = 0.43 fps, Avg. Travel Time= 9.3 min

Peak Storage= 363 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.14', Surface Width= 11.01' Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 70.22 cfs

10.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00'

Length= 240.0' Slope= 0.0153 '/'

Inlet Invert= 811.94', Outlet Invert= 808.26'



## Summary for Reach S4.2: Swale S4.2

Inflow Area = 3.174 ac, 0.00% Impervious, Inflow Depth = 1.41" for 25-yr, 24-hr event

Inflow = 6.29 cfs @ 12.15 hrs, Volume= 0.372 af

Outflow = 5.84 cfs @ 12.21 hrs, Volume= 0.372 af, Atten= 7%, Lag= 3.6 min

Routed to Reach S4.3: Swale S4.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.12 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.53 fps, Avg. Travel Time= 8.1 min

Peak Storage= 732 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.26', Surface Width= 11.81'

Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 63.88 cfs

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MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

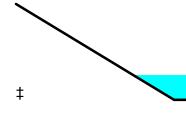
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10.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00' Length= 259.3' Slope= 0.0127 '/' Inlet Invert= 808.26', Outlet Invert= 804.97'



#### Summary for Reach S4.3: Swale S4.3

Inflow Area = 9.328 ac, 0.00% Impervious, Inflow Depth = 1.49" for 25-yr, 24-hr event

Inflow = 18.75 cfs @ 12.14 hrs, Volume= 1.155 af

Outflow = 16.68 cfs @ 12.24 hrs, Volume= 1.155 af, Atten= 11%, Lag= 5.7 min

Routed to Reach S4.4: Swale S4.4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.82 fps, Min. Travel Time= 3.3 min

Avg. Velocity = 0.46 fps, Avg. Travel Time= 13.2 min

Peak Storage= 3,352 cf @ 12.18 hrs

Average Depth at Peak Storage= 0.73', Surface Width= 15.14' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 108.12 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 362.9' Slope= 0.0027 '/'

Inlet Invert= 804.97', Outlet Invert= 804.00'



## Summary for Reach S4.4: Swale S4.4

Inflow Area = 14.003 ac, 0.00% Impervious, Inflow Depth = 1.48" for 25-yr, 24-hr event

Inflow = 22.06 cfs @ 12.20 hrs, Volume= 1.732 af

Outflow = 21.14 cfs @ 12.31 hrs, Volume= 1.732 af, Atten= 4%, Lag= 6.5 min

Routed to Reach S4.5: Swale S4.5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.26 fps, Min. Travel Time= 3.6 min

Avg. Velocity = 0.58 fps, Avg. Travel Time= 14.2 min

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Peak Storage= 4,669 cf @ 12.25 hrs

Average Depth at Peak Storage= 0.75', Surface Width= 15.23' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 132.85 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 495.6' Slope= 0.0040 '/'

Inlet Invert= 804.00', Outlet Invert= 802.00'



#### Summary for Reach S4.5: Swale S4.5

Inflow Area = 16.113 ac, 0.00% Impervious, Inflow Depth = 1.43" for 25-yr, 24-hr event

Inflow = 22.26 cfs @ 12.30 hrs, Volume= 1.915 af

Outflow = 21.79 cfs @ 12.37 hrs, Volume= 1.915 af, Atten= 2%, Lag= 3.9 min

Routed to Reach S4.6: Swale S4.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.07 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.85 fps, Avg. Travel Time= 8.0 min

Peak Storage= 2,934 cf @ 12.33 hrs

Average Depth at Peak Storage= 0.59', Surface Width= 14.14'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf. Capacity= 465.89 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 411.1' Slope= 0.0097 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



#### **Summary for Reach S4.6: Swale S4.6**

Inflow Area = 65.439 ac, 0.00% Impervious, Inflow Depth = 1.59" for 25-yr, 24-hr event

Inflow = 89.57 cfs @ 12.40 hrs, Volume= 8.673 af

Outflow = 88.39 cfs @ 12.45 hrs, Volume= 8.673 af, Atten= 1%, Lag= 3.3 min

Routed to Pond Sed Pond : Sedimentation Basin

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.00 fps, Min. Travel Time= 1.8 min

Avg. Velocity = 1.27 fps, Avg. Travel Time= 7.1 min

Peak Storage= 9,563 cf @ 12.42 hrs

Average Depth at Peak Storage= 1.24', Surface Width= 18.69'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf, Capacity= 499.25 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value 4.0 3.0 '/' Top Width= 31.00'

Length= 537.0' Slope= 0.0112 '/'

Inlet Invert= 798.00', Outlet Invert= 792.00'



#### Summary for Reach S5.1: Swale S5.1

Inflow Area = 0.993 ac, 0.00% Impervious, Inflow Depth = 2.46" for 25-yr, 24-hr event

Inflow = 4.05 cfs @ 12.11 hrs, Volume= 0.204 af

Outflow = 3.50 cfs @ 12.20 hrs, Volume= 0.204 af, Atten= 14%, Lag= 5.7 min

Routed to Link 4L : Culvert C2b

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.00 fps, Min. Travel Time= 3.6 min

Avg. Velocity = 0.55 fps, Avg. Travel Time= 13.0 min

Peak Storage= 769 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.20', Surface Width= 9.63'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 235.24 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 428.0' Slope= 0.0154 '/'

Inlet Invert= 825.20', Outlet Invert= 818.60'



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### **Summary for Reach S5.2: Swale S5.2**

Inflow Area = 0.424 ac, 0.00% Impervious, Inflow Depth = 2.29" for 25-yr, 24-hr event

Inflow = 1.73 cfs @ 12.10 hrs, Volume= 0.081 af

Outflow = 1.51 cfs @ 12.14 hrs, Volume= 0.081 af, Atten= 13%, Lag= 2.8 min

Routed to Link C2: Culvert C2a

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.89 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 0.70 fps, Avg. Travel Time= 4.4 min

Peak Storage= 156 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 8.81'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 344.73 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 183.6' Slope= 0.0331 '/'

Inlet Invert= 823.91', Outlet Invert= 817.83'



## **Summary for Pond C8: Culvert C8**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af

Outflow = 0.96 cfs @ 12.08 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.1 min

Primary = 0.96 cfs @ 12.08 hrs, Volume= 0.043 af

Routed to Reach S4.1: Swale S4.1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 811.21' 2 12.08 hrs Surf.Area= 0.000 ac Storage= 0.000 af Flood Elev= 819.00' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= 0.2 min calculated for 0.043 af (100% of inflow)

Center-of-Mass det. time= 0.2 min (830.2 - 830.0)

Volume	Invert	Avail.Storage	Storage Description
#1	810.70'	0.001 af	3.00'D x 7.00'H Vertical Cone/Cylinder
Device	Routing	Invert O	utlet Devices
#1	Primary	L= In	2.0" Round Culvert = 85.6' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 810.70' / 808.60' S= 0.0245 '/' Cc= 0.900 = 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

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Primary OutFlow Max=0.89 cfs @ 12.08 hrs HW=811.18' (Free Discharge) 1=Culvert (Inlet Controls 0.89 cfs @ 2.37 fps)

### **Summary for Pond N: North Infiltration Area**

Inflow Area = 18.714 ac. 0.00% Impervious, Inflow Depth = 0.75" for 25-yr, 24-hr event

11.60 cfs @ 12.28 hrs, Volume= 1.175 af Inflow =

1.40 cfs @ 13.94 hrs, Volume= 1.40 cfs @ 13.94 hrs, Volume= Outflow 1.175 af, Atten= 88%, Lag= 99.8 min =

1.175 af Primary =

Routed to nonexistent node 1L

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 803.89' @ 13.94 hrs Surf.Area= 16,766 sf Storage= 20,733 cf

Plug-Flow detention time= 178.4 min calculated for 1.174 af (100% of inflow)

Center-of-Mass det. time= 178.3 min ( 1,059.6 - 881.2 )

Volume	Inv	ert Avail.S	torage	Storage	Description	
#1 802		00' 256	569 cf	Custom Stage Data (Prismatic)Listed below (Recalc)		
				_		
Elevation		Surf.Area	Inc.	Store	Cum.Store	
(feet)		(sq-ft)	(cubic	:-feet)	(cubic-feet)	
802.0	00	5,140		0	0	
804.0	00	17,424		2,564	22,564	
806.0	00	32,191		9,615	72,179	
808.0	00	46,130	7	8,321	150,500	
810.0	00	59,939		6,069	256,569	
Device	Routing	Inve	t Outle	et Device	es	
#1	Primary	802.00	3.600	) in/hr E	xfiltration over	Surface area

Primary OutFlow Max=1.40 cfs @ 13.94 hrs HW=803.89' (Free Discharge) 1=Exfiltration (Exfiltration Controls 1.40 cfs)

# **Summary for Pond Sed Pond: Sedimentation Basin**

Inflow Area =	72.769 ac,	2.07% Impervious, Inflow L	Jepth = 1.55"	for 25-yr, 24-hr event
Inflow =	90.06 cfs @	12.45 hrs, Volume=	9.372 af	
Outflow =	15.75 cfs @	13.56 hrs, Volume=	9.372 af, Atte	n= 83%, Lag= 66.6 min
Discarded =	5.53 cfs @	13.56 hrs, Volume=	6.223 af	
Primary =	10.22 cfs @	13.56 hrs, Volume=	3.149 af	
Routed to Link	Wetland: We	etland		
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	
Routed to Link	Wetland: We	etland		
Tertiary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	
Routed to Link	Wetland: We	etland		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Peak Elev= 792.17' @ 13.56 hrs Surf.Area= 66,370 sf Storage= 175,109 cf Flood Elev= 794.00' Surf.Area= 75,797 sf Storage= 304,443 cf

Plug-Flow detention time= 186.0 min calculated for 9.366 af (100% of inflow)

Center-of-Mass det. time= 185.9 min ( 1,047.5 - 861.6 )

Volume	Invert	Avail.Sto	rage Storage	Description			
#1	789.00'	304,44	13 cf Custon	n Stage Data (P	rismatic)Listed below (Recalc)		
Elevation		f.Area	Inc.Store (cubic-feet)	Cum.Store			
(fee		(sq-ft)		(cubic-feet)			
789.0		27,325	0	0			
790.0		55,972	,	41,649 41,649			
791.0		51,532	58,752	100,401			
792.0		55,703	63,618	164,018			
793.0		69,675	67,689	231,707			
794.0	)0 /	75,797	72,736	304,443			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	787.70'	15.0" Round				
					nform to fill, Ke= 0.700		
					787.50' S= 0.0050 '/' Cc= 0.900		
			n= 0.011 Co	ncrete pipe, stra	ight & clean, Flow Area= 1.23 sf		
#2	Device 1	791.00'		Orifice/Grate (			
			Limited to we	eir flow at low hea	ads		
#3	Device 1	790.50'	<b>0.8" Vert. Orifice/Grate X 4.00</b> C= 0.600				
			Limited to we	ir flow at low hea	ads		
#4	Device 1	790.00'	<b>0.8" Vert. Orifice/Grate X 4.00</b> C= 0.600				
			Limited to we	ir flow at low hea	ads		
#5	Device 1	789.00'		ifice/Grate X 14			
			X 6 rows with 6.0" cc spacing C= 0.600				
				eir flow at low hea			
#6	Secondary	792.50'			Broad-Crested Rectangular Weir		
					0.80 1.00 1.20 1.40 1.60		
					70 2.69 2.68 2.69 2.67 2.64		
#7	Tertiary	793.00'			Broad-Crested Rectangular Weir		
					0.80 1.00 1.20 1.40 1.60		
	<b>5</b>	700.00:			70 2.69 2.68 2.69 2.67 2.64		
#8	Discarded	789.00'	3.600 in/hr E	xfiltration over	Surface area		

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**Discarded OutFlow** Max=5.53 cfs @ 13.56 hrs HW=792.17' (Free Discharge) **8=Exfiltration** (Exfiltration Controls 5.53 cfs)

Primary OutFlow Max=10.22 cfs @ 13.56 hrs HW=792.17' (Free Discharge)

**-1=Culvert** (Inlet Controls 10.22 cfs @ 8.33 fps)

**2=Orifice/Grate** (Passes < 25.54 cfs potential flow)

-3=Orifice/Grate (Passes < 0.09 cfs potential flow)

-4=Orifice/Grate (Passes < 0.10 cfs potential flow)

-5=Orifice/Grate (Passes < 0.74 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=789.00' (Free Discharge)
6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=789.00' (Free Discharge) 7=Broad-Crested Rectangular Weir( Controls 0.00 cfs)

## Summary for Link 4L: Culvert C2b

Inflow Area = 0.993 ac, 0.00% Impervious, Inflow Depth = 2.46" for 25-yr, 24-hr event

Inflow = 3.50 cfs @ 12.20 hrs, Volume= 0.204 af

Primary = 3.50 cfs @ 12.20 hrs, Volume= 0.204 af, Atten= 0%, Lag= 0.0 min

Routed to Link C2: Culvert C2a

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C1: Culvert C1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 0.51" for 25-yr, 24-hr event

Inflow = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af

Primary = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link C10: Culvert C10**

Inflow Area = 8.839 ac, 0.00% Impervious, Inflow Depth = 1.27" for 25-yr, 24-hr event

Inflow = 11.21 cfs @ 12.27 hrs, Volume= 0.937 af

Primary = 11.21 cfs @ 12.27 hrs, Volume= 0.937 af, Atten= 0%, Lag= 0.0 min

Routed to Pond N: North Infiltration Area

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link C11: Culvert C11**

Inflow Area = 9.783 ac, 0.00% Impervious, Inflow Depth = 2.18" for 25-yr, 24-hr event

Inflow = 26.46 cfs @ 12.22 hrs, Volume= 1.778 af

Primary = 26.46 cfs @ 12.22 hrs, Volume= 1.778 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### **Summary for Link C2: Culvert C2a**

Inflow Area = 2.969 ac, 0.00% Impervious, Inflow Depth = 2.53" for 25-yr, 24-hr event

Inflow = 10.41 cfs @ 12.20 hrs, Volume= 0.626 af

Primary = 10.41 cfs @ 12.20 hrs, Volume= 0.626 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link C3: Culvert C3

Inflow Area = 11.213 ac, 0.00% Impervious, Inflow Depth = 2.10" for 25-yr, 24-hr event

Inflow = 27.48 cfs @ 12.24 hrs, Volume= 1.960 af

Primary = 27.48 cfs @ 12.24 hrs, Volume= 1.960 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Summary for Link C4: Culvert C4

Inflow Area = 15.242 ac, 0.00% Impervious, Inflow Depth = 1.99" for 25-yr, 24-hr event

Inflow = 32.85 cfs @ 12.21 hrs, Volume= 2.521 af

Primary = 32.85 cfs @ 12.21 hrs, Volume= 2.521 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.5: Swale S1.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C5: Culvert C5

Inflow Area = 18.333 ac, 0.00% Impervious, Inflow Depth = 1.93" for 25-yr, 24-hr event

Inflow = 38.11 cfs @ 12.27 hrs, Volume= 2.941 af

Primary = 38.11 cfs @ 12.27 hrs, Volume= 2.941 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.9: Swale S1.9

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C6: Culvert C6

Inflow Area = 11.935 ac, 0.00% Impervious, Inflow Depth = 1.93" for 25-yr, 24-hr event

Inflow = 30.03 cfs @ 12.16 hrs, Volume= 1.918 af

Primary = 30.03 cfs @ 12.16 hrs, Volume= 1.918 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.5 : Swale S3.5

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## **Summary for Link C7: Culvert C7**

Inflow Area = 47.325 ac, 0.00% Impervious, Inflow Depth = 1.70" for 25-yr, 24-hr event

Inflow = 68.10 cfs @ 12.41 hrs, Volume= 6.703 af

Primary = 68.10 cfs @ 12.41 hrs, Volume= 6.703 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.6: Swale S4.6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link C9: Culvert C9**

Inflow Area = 0.306 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr, 24-hr event

Inflow = 0.22 cfs @ 12.20 hrs, Volume= 0.017 af

Primary = 0.22 cfs @ 12.20 hrs, Volume= 0.017 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.2 : Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link F10: Flume 10**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af

Primary = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.0 min

Routed to Pond C8: Culvert C8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F1A: Flume 1A

Inflow Area = 1.698 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 4.96 cfs @ 12.13 hrs, Volume= 0.268 af

Primary = 4.96 cfs @ 12.13 hrs, Volume= 0.268 af, Atten= 0%, Lag= 0.0 min

Routed to Link F1B: Flume 1B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F1B: Flume 1B

Inflow Area = 3.934 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 11.65 cfs @ 12.13 hrs, Volume= 0.620 af

Primary = 11.65 cfs @ 12.13 hrs, Volume= 0.620 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.3: Swale S4.3

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## **Summary for Link F2A: Flume 2A**

Inflow Area = 0.636 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 1.98 cfs @ 12.11 hrs, Volume= 0.100 af

Primary = 1.98 cfs @ 12.11 hrs, Volume= 0.100 af, Atten= 0%, Lag= 0.0 min

Routed to Link F2B: Flume 2B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F2B: Flume 2B

Inflow Area = 2.199 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 6.58 cfs @ 12.12 hrs, Volume= 0.347 af

Primary = 6.58 cfs @ 12.12 hrs, Volume= 0.347 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.4 : Swale S4.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Summary for Link F3: Flume 3

Inflow Area = 0.427 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 1.38 cfs @ 12.10 hrs, Volume= 0.067 af

Primary = 1.38 cfs @ 12.10 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.5: Swale S4.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F4A: Flume 4A

Inflow Area = 1.209 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 3.47 cfs @ 12.14 hrs, Volume= 0.191 af

Primary = 3.47 cfs @ 12.14 hrs, Volume= 0.191 af, Atten= 0%, Lag= 0.0 min

Routed to Link F4B: Flume 4B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F4B: Flume 4B

Inflow Area = 3.092 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 9.10 cfs @ 12.13 hrs, Volume= 0.488 af

Primary = 9.10 cfs @ 12.13 hrs, Volume= 0.488 af, Atten= 0%, Lag= 0.0 min

Routed to Link C7: Culvert C7

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## Summary for Link F5A: Flume 5A

Inflow Area = 0.237 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow 0.76 cfs @ 12.11 hrs, Volume= 0.037 af

0.76 cfs @ 12.11 hrs, Volume= Primary 0.037 af, Atten= 0%, Lag= 0.0 min

Routed to Link F5B: Flume 5B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F5B: Flume 5B

0.712 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event Inflow Area =

2.23 cfs @ 12.11 hrs, Volume= 0.112 af Inflow

2.23 cfs @ 12.11 hrs, Volume= 0.112 af, Atten= 0%, Lag= 0.0 min Primary

Routed to Reach S2.2: Swale S2.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F6A: Flume 6A

Inflow Area = 1.440 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow 4.38 cfs @ 12.11 hrs, Volume= 0.227 af

Primary 4.38 cfs @ 12.11 hrs, Volume= 0.227 af, Atten= 0%, Lag= 0.0 min

Routed to Link F6B: Flume 6B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F6B: Flume 6B

Inflow Area = 3.001 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

0.473 af 9.20 cfs @ 12.11 hrs, Volume= Inflow

9.20 cfs @ 12.11 hrs, Volume= 0.473 af, Atten= 0%, Lag= 0.0 min Primary

Routed to Link C6: Culvert C6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F7A: Flume 7A**

2.924 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event Inflow Area =

8.53 cfs @ 12.14 hrs, Volume= Inflow 0.461 af

Primary 8.53 cfs @ 12.14 hrs, Volume= 0.461 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7B: Flume 7B

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## **Summary for Link F7B: Flume 7B**

Inflow Area = 5.160 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 15.13 cfs @ 12.13 hrs, Volume= 0.814 af

Primary = 15.13 cfs @ 12.13 hrs, Volume= 0.814 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.2: Swale S3.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F8A: Flume 8A

Inflow Area = 0.791 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 2.31 cfs @ 12.14 hrs, Volume= 0.125 af

Primary = 2.31 cfs @ 12.14 hrs, Volume= 0.125 af, Atten= 0%, Lag= 0.0 min

Routed to Link F8B: Flume 8B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Summary for Link F8B: Flume 8B

Inflow Area = 2.667 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 7.92 cfs @ 12.12 hrs, Volume= 0.421 af

Primary = 7.92 cfs @ 12.12 hrs, Volume= 0.421 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F9A: Flume 9A

Inflow Area = 3.091 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 8.97 cfs @ 12.14 hrs, Volume= 0.487 af

Primary = 8.97 cfs @ 12.14 hrs, Volume= 0.487 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9B: Flume 9B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F9B: Flume 9B

Inflow Area = 6.189 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 18.15 cfs @ 12.13 hrs, Volume= 0.976 af

Primary = 18.15 cfs @ 12.13 hrs, Volume= 0.976 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.0: Swale S1.0

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# **Summary for Link Wetland: Wetland**

Inflow Area = 72.769 ac, 2.07% Impervious, Inflow Depth = 0.52" for 25-yr, 24-hr event

Inflow = 10.22 cfs @ 13.56 hrs, Volume= 3.149 af

Primary = 10.22 cfs @ 13.56 hrs, Volume= 3.149 af, Atten= 0%, Lag= 0.0 min

Routed to nonexistent node 1L

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentArea 1: Area 1	Runoff Area=1.141 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=500' Tc=6.5 min CN=69 Runoff=5.61 cfs 0.302 af
SubcatchmentArea 10: Area 10	Runoff Area=0.791 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=354' Tc=6.4 min CN=69 Runoff=3.91 cfs 0.210 af
SubcatchmentArea 11: Area 11	Runoff Area=0.885 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=288' Tc=6.4 min CN=69 Runoff=4.37 cfs 0.235 af
SubcatchmentArea 12: Area 12	Runoff Area=2.206 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=628' Tc=6.7 min CN=69 Runoff=10.79 cfs 0.585 af
SubcatchmentArea 13: Area 13	Runoff Area=1.610 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=712' Tc=5.3 min CN=69 Runoff=8.11 cfs 0.427 af
SubcatchmentArea 14: Area 14	Runoff Area=0.626 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=333' Tc=4.5 min CN=69 Runoff=3.29 cfs 0.166 af
SubcatchmentArea 15: Area 15	Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=331' Tc=4.4 min CN=69 Runoff=3.27 cfs 0.164 af
SubcatchmentArea 16: Area 16	Runoff Area=0.943 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=464' Tc=5.9 min CN=69 Runoff=4.72 cfs 0.250 af
SubcatchmentArea 17: Area 17	Runoff Area=0.571 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=315' Tc=4.4 min CN=69 Runoff=3.01 cfs 0.151 af
SubcatchmentArea 18: Area 18	Runoff Area=0.990 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=424' Tc=4.7 min CN=69 Runoff=5.14 cfs 0.262 af
SubcatchmentArea 19: Area 19	Runoff Area=1.179 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=385' Tc=4.7 min CN=69 Runoff=6.12 cfs 0.313 af
SubcatchmentArea 2: Area 2	Runoff Area=0.557 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=303' Tc=4.7 min CN=69 Runoff=2.89 cfs 0.148 af
SubcatchmentArea 20: Area 20	Runoff Area=1.057 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=419' Tc=4.7 min CN=69 Runoff=5.49 cfs 0.280 af
SubcatchmentArea 21: Area 21	Runoff Area=0.434 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=374' Tc=4.5 min CN=69 Runoff=2.28 cfs 0.115 af
SubcatchmentArea 22: Area 22	Runoff Area=1.442 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=489' Tc=4.8 min CN=69 Runoff=7.45 cfs 0.382 af
SubcatchmentArea 23: Area 23	Runoff Area=1.252 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=435' Tc=4.7 min CN=69 Runoff=6.50 cfs 0.332 af

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SubcatchmentArea 24: Area 24

Runoff Area=1.846 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=701' Tc=5.4 min CN=69 Runoff=9.33 cfs 0.489 af

SubcatchmentArea 25: Area 25 Runoff Area=1.552 ac 0.00% Impervious Runoff Depth=4.10" Flow Length=123' Slope=0.2500 '/' Tc=3.9 min CN=78 Runoff=10.67 cfs 0.531 af

SubcatchmentArea 26: Area 26

Runoff Area=0.616 ac 0.00% Impervious Runoff Depth=3.48"

Flow Length=109' Slope=0.2500 '/' Tc=3.8 min CN=72 Runoff=3.67 cfs 0.179 af

SubcatchmentArea 27: Area 27 Runoff Area=0.149 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=211' Tc=1.2 min CN=69 Runoff=0.86 cfs 0.039 af

SubcatchmentArea 28: Area 28 Runoff Area=0.126 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=59' Slope=0.2500 '/' Tc=2.5 min CN=69 Runoff=0.74 cfs 0.033 af

SubcatchmentArea 29: Area 29

Runoff Area=1.228 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=647' Tc=5.7 min CN=69 Runoff=6.17 cfs 0.325 af

SubcatchmentArea 3: Area 3 Runoff Area=0.348 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=298' Tc=4.8 min CN=69 Runoff=1.80 cfs 0.092 af

SubcatchmentArea 30: Area 30

Runoff Area=0.427 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=362' Tc=3.8 min CN=69 Runoff=2.33 cfs 0.113 af

SubcatchmentArea 31: Area 31

Runoff Area=0.223 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=296' Tc=3.7 min CN=69 Runoff=1.22 cfs 0.059 af

SubcatchmentArea 32: Area 32

Runoff Area=0.655 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=642' Tc=5.0 min CN=69 Runoff=3.35 cfs 0.174 af

SubcatchmentArea 33: Area 33

Runoff Area=0.237 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=348' Tc=4.0 min CN=69 Runoff=1.28 cfs 0.063 af

SubcatchmentArea 34: Area 34

Runoff Area=0.424 ac 0.00% Impervious Runoff Depth=3.69"

Flow Length=78' Slope=0.2500 '/' Tc=3.1 min CN=74 Runoff=2.76 cfs 0.130 af

**SubcatchmentArea 35: Area 35**Runoff Area=0.993 ac 0.00% Impervious Runoff Depth=3.89"
Flow Length=126' Tc=4.4 min CN=76 Runoff=6.35 cfs 0.322 af

SubcatchmentArea 36: Area 36

Runoff Area=2.594 ac 0.00% Impervious Runoff Depth=4.75"
Flow Length=183' Tc=2.2 min CN=84 Runoff=21.62 cfs 1.027 af

SubcatchmentArea 37: Area 37

Runoff Area=0.306 ac 0.00% Impervious Runoff Depth=1.44"
Flow Length=133' Tc=4.5 min CN=50 Runoff=0.66 cfs 0.037 af

SubcatchmentArea 38: Area 38 Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=2.69" Flow Length=156' Slope=0.2500 '/' Tc=4.1 min CN=64 Runoff=6.48 cfs 0.321 af

SubcatchmentArea 39: Area 39

Runoff Area=0.742 ac 0.00% Impervious Runoff Depth=2.60"
Flow Length=145' Tc=4.3 min CN=63 Runoff=3.20 cfs 0.161 af

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SubcatchmentArea 4: Area 4 Runoff Area=0.288 ac 0.00% Impervious Runoff Depth=3.18"

Flow Length=296' Tc=4.0 min CN=69 Runoff=1.55 cfs 0.076 af

SubcatchmentArea 40: Area 40

Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.96"
Flow Length=168' Tc=9.6 min CN=56 Runoff=1.57 cfs 0.101 af

SubcatchmentArea 41: Area 41

Runoff Area=0.739 ac 0.00% Impervious Runoff Depth=1.96"
Flow Length=141' Tc=7.2 min CN=56 Runoff=2.11 cfs 0.120 af

SubcatchmentArea 42a: Area 42a

Runoff Area=0.871 ac 0.00% Impervious Runoff Depth=2.79"

Flow Length=144' Slope=0.0500 '/' Tc=7.8 min CN=65 Runoff=3.58 cfs 0.202 af

SubcatchmentArea 42b: Area 42b Runoff Area=0.712 ac 0.00% Impervious Runoff Depth=3.38" Flow Length=102' Slope=0.0500 '/' Tc=7.3 min CN=71 Runoff=3.63 cfs 0.201 af

SubcatchmentArea 43: Area 43

Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=3.18"

Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=3.63 cfs 0.204 af

SubcatchmentArea 44: Area 44

Runoff Area=1.416 ac 0.00% Impervious Runoff Depth=0.63"

Flow Length=941' Slope=0.0260 '/' Tc=22.0 min CN=39 Runoff=0.43 cfs 0.074 af

SubcatchmentArea 45: Area 45

Runoff Area=2.792 ac 0.00% Impervious Runoff Depth=2.41"

Flow Length=419' Tc=14.7 min CN=61 Runoff=7.53 cfs 0.561 af

SubcatchmentArea 46: Area 46

Runoff Area=2.044 ac 0.00% Impervious Runoff Depth=3.18"
Flow Length=138' Tc=4.0 min CN=69 Runoff=11.02 cfs 0.542 af

SubcatchmentArea 47: Area 47

Runoff Area=0.457 ac 0.00% Impervious Runoff Depth=3.28"

Flow Length=122' Slope=0.2500 '/' Tc=3.9 min CN=70 Runoff=2.55 cfs 0.125 af

SubcatchmentArea 48: Area 48 Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=3.58" Flow Length=98' Slope=0.0500 '/' Tc=7.2 min CN=73 Runoff=6.45 cfs 0.357 af

SubcatchmentArea 49: Area 49

Runoff Area=0.079 ac 0.00% Impervious Runoff Depth=3.48"
Flow Length=143' Tc=5.1 min CN=72 Runoff=0.44 cfs 0.023 af

**SubcatchmentArea 5: Area 5**Runoff Area=0.986 ac 0.00% Impervious Runoff Depth=3.18"

Flow Length=608' Tc=6.6 min CN=69 Runoff=4.85 cfs 0.261 af

SubcatchmentArea 50: Area 50

Runoff Area=3.726 ac 0.00% Impervious Runoff Depth=3.38"

Flow Length=391' Tc=11.1 min CN=71 Runoff=16.26 cfs 1.050 af

SubcatchmentArea 51: Area 51 Runoff Area=0.698 ac 0.00% Impervious Runoff Depth=0.63"

Flow Length=100' Slope=0.0600 '/' Tc=6.8 min CN=39 Runoff=0.32 cfs 0.036 af

SubcatchmentArea 52: Area 52 Runoff Area=0.475 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=425' Tc=4.5 min CN=69 Runoff=2.50 cfs 0.126 af

SubcatchmentArea 53: Area 53

Runoff Area=1.618 ac 0.00% Impervious Runoff Depth=2.23"
Flow Length=384' Tc=4.2 min CN=59 Runoff=5.93 cfs 0.300 af

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SubcatchmentArea 54: Area 54

Runoff Area=0.826 ac 0.00% Impervious Runoff Depth=2.13"

Flow Length=126' Slope=0.2500 '/' Tc=3.9 min CN=58 Runoff=2.95 cfs 0.147 af

SubcatchmentArea 55: Area 55

Runoff Area=1.089 ac 0.00% Impervious Runoff Depth=2.13"
Flow Length=158' Tc=4.3 min CN=58 Runoff=3.79 cfs 0.194 af

SubcatchmentArea 56: Area 56 Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=2.23"

Flow Length=221' Tc=4.4 min CN=59 Runoff=4.33 cfs 0.221 af

SubcatchmentArea 57: Area 57 Runoff Area=2.220 ac 0.00% Impervious Runoff Depth=1.78"

Flow Length=240' Tc=4.6 min CN=54 Runoff=6.17 cfs 0.330 af

SubcatchmentArea 58: Area 58 Runoff Area=2.476 ac 0.00% Impervious Runoff Depth=2.13"

Flow Length=263' Tc=4.7 min CN=58 Runoff=8.43 cfs 0.441 af

SubcatchmentArea 59: Area 59 Runoff Area=1.683 ac 0.00% Impervious Runoff Depth=1.70"

Flow Length=146' Slope=0.2345 '/' Tc=4.1 min CN=53 Runoff=4.54 cfs 0.238 af

SubcatchmentArea 6: Area 6 Runoff Area=0.504 ac 0.00% Impervious Runoff Depth=3.18"

Flow Length=258' Tc=3.4 min CN=69 Runoff=2.81 cfs 0.134 af

SubcatchmentArea 60: Area 60 Runoff Area=2.001 ac 0.00% Impervious Runoff Depth=0.90"

Flow Length=318' Tc=20.8 min CN=43 Runoff=1.18 cfs 0.150 af

SubcatchmentArea 61: Area 61 Runoff Area=2.177 ac 0.00% Impervious Runoff Depth=0.63"

Flow Length=149' Tc=8.2 min CN=39 Runoff=0.97 cfs 0.114 af

SubcatchmentArea 62: Area 62 Runoff Area=0.594 ac 0.00% Impervious Runoff Depth=1.44"

Flow Length=147' Slope=0.0544 '/' Tc=7.6 min CN=50 Runoff=1.14 cfs 0.071 af

SubcatchmentArea 63: Area 63 Runoff Area = 1.509 ac 100.00% Impervious Runoff Depth = 6.35"

Tc=0.0 min CN=98 Runoff=14.43 cfs 0.799 af

SubcatchmentArea 64: Area 64 Runoff Area=5.227 ac 0.00% Impervious Runoff Depth=0.63"

Flow Length=701' Tc=7.9 min CN=39 Runoff=2.36 cfs 0.273 af

SubcatchmentArea 65: Area 65 Runoff Area=3.035 ac 0.00% Impervious Runoff Depth=1.20"

Flow Length=886' Slope=0.0068 '/' Tc=38.9 min CN=47 Runoff=1.99 cfs 0.304 af

SubcatchmentArea 66: Area 66 Runoff Area=0.409 ac 0.00% Impervious Runoff Depth=2.23"

Flow Length=52' Slope=0.1154 '/' Tc=3.1 min CN=59 Runoff=1.60 cfs 0.076 af

SubcatchmentArea 67: Area 67 Runoff Area=0.755 ac 0.00% Impervious Runoff Depth=1.53"

Flow Length=86' Slope=0.2326 '/' Tc=3.5 min CN=51 Runoff=1.86 cfs 0.096 af

SubcatchmentArea 68: Area 68 Runoff Area=1.671 ac 0.00% Impervious Runoff Depth=1.53"

Flow Length=126' Tc=4.0 min CN=51 Runoff=3.98 cfs 0.213 af

SubcatchmentArea 69: Area 69 Runoff Area=9.875 ac 0.00% Impervious Runoff Depth=0.83"

Flow Length=1,337' Tc=16.6 min CN=42 Runoff=5.65 cfs 0.684 af

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SubcatchmentArea 7: Area 7 Runoff Area=0.936 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=541' Tc=5.3 min CN=69 Runoff=4.72 cfs 0.248 af

SubcatchmentArea 70: Area 70 Runoff Area=0.694 ac 0.00% Impervious Runoff Depth=2.89" Flow Length=127' Slope=0.2500 '/' Tc=3.9 min CN=66 Runoff=3.41 cfs 0.167 af

SubcatchmentArea 8: Area 8 Runoff Area=2.114 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=371' Tc=6.6 min CN=69 Runoff=10.41 cfs 0.560 af

SubcatchmentArea 9: Area 9 Runoff Area=0.810 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=292' Tc=6.3 min CN=69 Runoff=4.01 cfs 0.215 af

**Reach 53R: Culvert C1**Avg. Flow Depth=0.00' Max Vel=0.00 fps
24.0" Round Pipe n=0.012 L=51.9' S=0.0222 '/' Capacity=36.48 cfs Outflow=0.00 cfs 0.000 af

**Reach RD1: Roadside Ditch 1** Avg. Flow Depth=0.19' Max Vel=2.00 fps Inflow=1.99 cfs 0.304 af n=0.030 L=440.6' S=0.0188 '/' Capacity=47.16 cfs Outflow=1.96 cfs 0.304 af

**Reach RD2: Roadside Ditch 2** Avg. Flow Depth=0.51' Max Vel=3.14 fps Inflow=18.53 cfs 1.460 af n=0.030 L=433.0' S=0.0162 '/' Capacity=72.77 cfs Outflow=17.64 cfs 1.460 af

**Reach RD3: Roadside Ditch 3**Avg. Flow Depth=0.74' Max Vel=4.23 fps Inflow=10.67 cfs 0.531 af n=0.030 L=821.0' S=0.0288 '/' Capacity=20.76 cfs Outflow=9.18 cfs 0.531 af

**Reach RD4: Roadside Ditch 4** Avg. Flow Depth=0.24' Max Vel=1.52 fps Inflow=3.98 cfs 0.213 af n=0.030 L=495.6' S=0.0090 '/' Capacity=54.26 cfs Outflow=2.99 cfs 0.213 af

**Reach RD5: Roadside Ditch 5** Avg. Flow Depth=0.46' Max Vel=4.14 fps Inflow=3.67 cfs 0.179 af n=0.030 L=288.0' S=0.0531 '/' Capacity=28.18 cfs Outflow=3.30 cfs 0.179 af

**Reach S1.0: Swale S1.0**Avg. Flow Depth=1.21' Max Vel=2.81 fps Inflow=49.33 cfs 2.668 af n=0.030 L=551.3' S=0.0039 '/' Capacity=118.85 cfs Outflow=43.23 cfs 2.668 af

**Reach S1.1: Swale S1.1**Avg. Flow Depth=0.06' Max Vel=1.42 fps Inflow=0.66 cfs 0.037 af n=0.030 L=98.0' S=0.0396 '/' Capacity=376.93 cfs Outflow=0.61 cfs 0.037 af

**Reach S1.2: Swale S1.2**Avg. Flow Depth=0.76' Max Vel=5.41 fps Inflow=45.42 cfs 2.871 af n=0.030 L=170.0' S=0.0242 '/' Capacity=294.55 cfs Outflow=44.63 cfs 2.871 af

**Reach S1.3: Swale S1.3**Avg. Flow Depth=0.74' Max Vel=5.86 fps Inflow=47.67 cfs 3.192 af n=0.030 L=212.6' S=0.0293 '/' Capacity=324.28 cfs Outflow=46.84 cfs 3.192 af

**Reach S1.4: Swale S1.4**Avg. Flow Depth=1.00' Max Vel=4.85 fps Inflow=58.99 cfs 4.161 af n=0.030 L=72.2' S=0.0144 '/' Capacity=227.36 cfs Outflow=58.04 cfs 4.161 af

**Reach S1.5: Swale S1.5**Avg. Flow Depth=1.38' Max Vel=3.22 fps Inflow=59.91 cfs 4.281 af n=0.030 L=148.0' S=0.0045 '/' Capacity=126.50 cfs Outflow=59.49 cfs 4.281 af

**Reach S1.6: Swale S1.6**Avg. Flow Depth=0.27' Max Vel=1.35 fps Inflow=3.63 cfs 0.204 af n=0.030 L=179.7' S=0.0050 '/' Capacity=134.06 cfs Outflow=3.33 cfs 0.204 af

Reach S4.6: Swale S4.6

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Avg. Flow Depth=1.41' Max Vel=3.58 fps Inflow=69.04 cfs 4.888 af Reach S1.7: Swale S1.7 n=0.030 L=252.0' S=0.0054 '/' Capacity=139.68 cfs Outflow=67.28 cfs 4.888 af Avg. Flow Depth=0.41' Max Vel=2.20 fps Inflow=3.63 cfs 0.201 af Reach S1.8: Swale S1.8 n=0.030 L=245.8' S=0.0099 '/' Capacity=90.14 cfs Outflow=3.32 cfs 0.201 af Reach S1.9: Swale S1.9 Avg. Flow Depth=1.35' Max Vel=4.11 fps Inflow=74.80 cfs 5.449 af n=0.030 L=422.0' S=0.0075 '/' Capacity=163.67 cfs Outflow=72.74 cfs 5.449 af Reach S2.1: Swale S2.1 Avg. Flow Depth=1.75' Max Vel=3.74 fps Inflow=73.27 cfs 5.559 af n=0.030 L=389.0' S=0.0054 '/' Capacity=97.05 cfs Outflow=71.33 cfs 5.559 af Reach S2.2: Swale S2.2 Avg. Flow Depth=1.78' Max Vel=3.97 fps Inflow=121.32 cfs 10.154 af n=0.030 L=411.0' S=0.0049 '/' Capacity=152.61 cfs Outflow=119.32 cfs 10.154 af Reach S2.3: Swale S2.3 Avg. Flow Depth=1.38' Max Vel=5.67 fps Inflow=121.74 cfs 10.568 af n=0.030 L=307.0' S=0.0130'/' Capacity=249.72 cfs Outflow=120.67 cfs 10.568 af Avg. Flow Depth=0.50' Max Vel=1.94 fps Inflow=11.02 cfs 0.542 af Reach S3.1: Swale S3.1 n=0.030 L=357.0' S=0.0050'/' Capacity=133.76 cfs Outflow=9.75 cfs 0.542 af Reach S3.2: Swale S3.2 Avg. Flow Depth=1.02' Max Vel=2.89 fps Inflow=35.67 cfs 2.034 af n=0.030 L=34.0' S=0.0050'/' Capacity=133.95 cfs Outflow=35.48 cfs 2.034 af Avg. Flow Depth=0.78' Max Vel=4.03 fps Inflow=35.48 cfs 2.034 af Reach S3.3: Swale S3.3 n=0.030 L=200.0' S=0.0130 '/' Capacity=215.99 cfs Outflow=33.84 cfs 2.034 af Avg. Flow Depth=0.61' Max Vel=2.29 fps Inflow=6.45 cfs 0.357 af Reach S3.4: Swale S3.4 n=0.030 L=283.0' S=0.0071 '/' Capacity=76.21 cfs Outflow=5.88 cfs 0.357 af Reach S3.5: Swale S3.5 Avg. Flow Depth=1.68' Max Vel=2.64 fps Inflow=67.72 cfs 4.259 af n=0.030 L=318.5' S=0.0024'/' Capacity=93.14 cfs Outflow=63.93 cfs 4.259 af Avg. Flow Depth=0.22' Max Vel=2.07 fps Inflow=5.28 cfs 0.267 af Reach S4.1: Swale S4.1 n=0.030 L=240.0' S=0.0153 '/' Capacity=70.22 cfs Outflow=4.68 cfs 0.267 af Avg. Flow Depth=0.38' Max Vel=2.70 fps Inflow=11.92 cfs 0.667 af Reach S4.2: Swale S4.2 n=0.030 L=259.3' S=0.0127'/' Capacity=63.88 cfs Outflow=11.06 cfs 0.667 af Reach S4.3: Swale S4.3 Avg. Flow Depth=1.07' Max Vel=2.24 fps Inflow=35.42 cfs 2.039 af n=0.030 L=362.9' S=0.0027'/' Capacity=108.12 cfs Outflow=32.72 cfs 2.039 af Reach S4.4: Swale S4.4 Avg. Flow Depth=1.10' Max Vel=2.81 fps Inflow=44.75 cfs 3.062 af n=0.030 L=495.6' S=0.0040 '/' Capacity=132.85 cfs Outflow=42.89 cfs 3.062 af Reach S4.5: Swale S4.5 Avg. Flow Depth=0.88' Max Vel=3.85 fps Inflow=45.33 cfs 3.413 af n=0.030 L=411.1' S=0.0097'/ Capacity=465.89 cfs Outflow=44.02 cfs 3.413 af

Avg. Flow Depth=1.73' Max Vel=6.01 fps Inflow=168.51 cfs 14.952 af

n=0.030 L=537.0' S=0.0112 '/' Capacity=499.25 cfs Outflow=165.78 cfs 14.952 af

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MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

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Reach S5.1: Swale S5.1 Avg. Flow Depth=0.27' Max Vel=2.36 fps Inflow=6.35 cfs 0.322 af

n=0.030 L=428.0' S=0.0154'/' Capacity=235.24 cfs Outflow=5.76 cfs 0.322 af

Reach S5.2: Swale S5.2 Avg. Flow Depth=0.13' Max Vel=2.27 fps Inflow=2.76 cfs 0.130 af

n=0.030 L=183.6' S=0.0331'/' Capacity=344.73 cfs Outflow=2.42 cfs 0.130 af

Pond C8: Culvert C8 Peak Elev=811.38' Storage=0.000 af Inflow=1.60 cfs 0.073 af

12.0" Round Culvert n=0.012 L=85.6' S=0.0245 '/' Outflow=1.60 cfs 0.073 af

Pond N: North Infiltration Area Peak Elev=805.24' Storage=49,884 cf Inflow=25.48 cfs 2.356 af

Outflow=2.22 cfs 2.356 af

**Pond Sed Pond: Sedimentation Basin** Peak Elev=793.23' Storage=247,875 cf Inflow=169.90 cfs 16.095 af 22 af Primary=11.55 cfs 5.267 af Secondary=33.59 cfs 2.497 af Tertiary=43.71 cfs 1.010 af Outflow=94.78 cfs 16.095 af

Link 4L: Culvert C2b Inflow=5.76 cfs 0.322 af

Primary=5.76 cfs 0.322 af

Link C1: Culvert C1 Inflow=1.96 cfs 0.304 af

Primary=1.96 cfs 0.304 af

Link C10: Culvert C10 Inflow=20.55 cfs 1.672 af

Primary=20.55 cfs 1.672 af

Link C11: Culvert C11 Inflow=44.63 cfs 2.871 af

Primary=44.63 cfs 2.871 af

Link C2: Culvert C2a Inflow=16.75 cfs 0.983 af

Primary=16.75 cfs 0.983 af

Link C3: Culvert C3 Inflow=46.84 cfs 3.192 af

Primary=46.84 cfs 3.192 af

Link C4: Culvert C4 Inflow=58.04 cfs 4.161 af

Primary=58.04 cfs 4.161 af

Link C5: Culvert C5 Inflow=67.28 cfs 4.888 af

Primary=67.28 cfs 4.888 af

Link C6: Culvert C6 Inflow=52.51 cfs 3.209 af

Primary=52.51 cfs 3.209 af

**Link C7: Culvert C7** Inflow=124.99 cfs 11.388 af

Primary=124.99 cfs 11.388 af

Link C9: Culvert C9 Inflow=0.61 cfs 0.037 af

Primary=0.61 cfs 0.037 af

Link F10: Flume 10 Inflow=1.60 cfs 0.073 af

Primary=1.60 cfs 0.073 af

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**230828_COL_Mod12**MSE 24-hr

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Trydroom Be 10.20 2d 3/11 00004 @ 2021 Trydroom B Contware Column E20	1 agc 01
Link F1A: Flume 1A	Inflow=8.37 cfs 0.450 af
	Primary=8.37 cfs 0.450 af
Link EAD. Elima AD	Inflow=10.50 of 1.042 of
Link F1B: Flume 1B	Inflow=19.59 cfs 1.043 af
	Primary=19.59 cfs 1.043 af
Link F2A: Flume 2A	Inflow=3.34 cfs 0.169 af
LIIIK FZA. FIUIIIe ZA	Primary=3.34 cfs 0.169 af
	Filliary=3.34 cis 0.109 ai
Link F2B: Flume 2B	Inflow=11.13 cfs 0.583 af
Lilik i 2D. i iuliię 2D	Primary=11.13 cfs 0.583 af
	1 11111ary - 11.13 cl3 0.303 ar
Link F3: Flume 3	Inflow=2.33 cfs 0.113 af
Link i 5. i luille 5	Primary=2.33 cfs 0.113 af
	1 1111ary = 2.00 013 0.110 ar
Link F4A: Flume 4A	Inflow=5.87 cfs 0.320 af
Link i TA. i idilie TA	Primary=5.87 cfs 0.320 af
	1 1111ary 0.01 010 0.020 ar
Link F4B: Flume 4B	Inflow=15.35 cfs 0.820 af
	Primary=15.35 cfs 0.820 af
	1 milary 10.00 010 0.020 ar
Link F5A: Flume 5A	Inflow=1.28 cfs 0.063 af
	Primary=1.28 cfs 0.063 af
Link F5B: Flume 5B	Inflow=3.77 cfs 0.189 af
	Primary=3.77 cfs 0.189 af
Link F6A: Flume 6A	Inflow=7.42 cfs 0.382 af
	Primary=7.42 cfs 0.382 af
	,
Link F6B: Flume 6B	Inflow=15.57 cfs 0.795 af
	Primary=15.57 cfs 0.795 af
	•
Link F7A: Flume 7A	Inflow=14.41 cfs 0.775 af
	Primary=14.41 cfs 0.775 af
Link F7B: Flume 7B	Inflow=25.54 cfs 1.368 af
	Primary=25.54 cfs 1.368 af
Link F8A: Flume 8A	Inflow=3.91 cfs 0.210 af
	Primary=3.91 cfs 0.210 af
Link F8B: Flume 8B	Inflow=13.41 cfs 0.707 af
	Primary=13.41 cfs 0.707 af
Link FOA. Floor - OA	I-f45 40 5 0040 5
Link F9A: Flume 9A	Inflow=15.16 cfs 0.819 af
	Primary=15.16 cfs 0.819 af
Link FOD. Fluma OD	Inflow=20.62 -f- 4.640 -f
Link F9B: Flume 9B	Inflow=30.63 cfs 1.640 af
	Primary=30.63 cfs 1.640 af

230828_COL_Mod12

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

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**Link Wetland: Wetland** 

Inflow=88.85 cfs 8.773 af Primary=88.85 cfs 8.773 af

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#### **Summary for Subcatchment Area 1: Area 1**

Runoff 5.61 cfs @ 12.14 hrs, Volume= 0.302 af, Depth= 3.18"

Routed to Link F1A: Flume 1A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
	1.	141 6	9 Past	ture/grassl	and/range,	Fair, HSG B
Ī	1.	141	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
Ī	3.7	60	0.1000	0.27		Sheet Flow,
	1.8	40	0.2500	0.36		Grass: Short n= 0.150 P2= 2.77" <b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.77"
	0.1	16	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.9	384	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
_	6.5	500	Total			

#### **Summary for Subcatchment Area 10: Area 10**

Runoff 3.91 cfs @ 12.14 hrs, Volume= 0.210 af, Depth= 3.18"

Routed to Link F8A: Flume 8A

	Area	(ac) C	N Des	cription					
_	0.791 69 Pasture/grassland/range, Fair, HSG B								
0.791 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	4.2	71	0.1000	0.28		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"			
	1.4	29	0.2500	0.34		Sheet Flow,			
	0.4	88	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow,			
	0.4	166	0.0200	6.74	80.87	Short Grass Pasture Kv= 7.0 fps Trap/Vee/Rect Channel Flow, Diversion Berm			
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
-	6.4	354	Total			11- 0.000 Latti, grassed & willding			

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### **Summary for Subcatchment Area 11: Area 11**

Runoff = 4.37 cfs @ 12.14 hrs, Volume= 0.235 af, Depth= 3.18"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription		
0.	885 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	885	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	71	0.1000	0.28		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.4	29	0.2500	0.34		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.6	119	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.2	69	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
					n= 0.030 Earth, grassed & winding
6.4	288	Total			

#### **Summary for Subcatchment Area 12: Area 12**

Runoff = 10.79 cfs @ 12.14 hrs, Volume= 0.585 af, Depth= 3.18"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	۸ م	(aa) C	N Dee						
_	Area	<u>(ac) C</u>	n Desc	cription					
	2.206 69 Pasture/grassland/range, Fair, HSG B								
2.206 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.7	40	0.1000	0.25		Sheet Flow,			
	2.5	60	0.2500	0.39		Grass: Short n= 0.150 P2= 2.77"  Sheet Flow,			
	0.3	56	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	1.2	472	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm			
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
_	6.7	628	Total			· 5			

6.7 628 Total

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### **Summary for Subcatchment Area 13: Area 13**

Runoff = 8.11 cfs @ 12.12 hrs, Volume= 0.427 af, Depth= 3.18"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription					
	1.610 69 Pasture/grassland/range, Fair, HSG B								
_	1.	610	100.	00% Pervi	ous Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	3.8	100	0.2500	0.43		Sheet Flow,			
	0.1	31	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	1.4	581	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
	53	712	Total			<u>-</u>			

#### 5.3 /12 lotal

## **Summary for Subcatchment Area 14: Area 14**

Runoff = 3.29 cfs @ 12.11 hrs, Volume= 0.166 af, Depth= 3.18"

Routed to Link F1B: Flume 1B

Area	(ac) C	N Desc	cription		
0.	.626 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	.626	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.5	183	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.5	333	Total			

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### **Summary for Subcatchment Area 15: Area 15**

Runoff = 3.27 cfs @ 12.11 hrs, Volume= 0.164 af, Depth= 3.18"

Routed to Link F2B: Flume 2B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	ı (ac) C	N Des	cription						
	0.620 69 Pasture/grassland/range, Fair, HSG B								
Tc (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
3.8	100	0.2500	0.43		Sheet Flow,				
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding				
4.4	331	Total							

## **Summary for Subcatchment Area 16: Area 16**

Runoff = 4.72 cfs @ 12.13 hrs, Volume= 0.250 af, Depth= 3.18"

Routed to Link F2B: Flume 2B

Area	(ac) C	N Desc	cription		
0.	943 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	943	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	100	0.1500	0.35		Sheet Flow,
0.5	95	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	269	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5.9	464	Total			

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#### **Summary for Subcatchment Area 17: Area 17**

Runoff = 3.01 cfs @ 12.11 hrs, Volume= 0.151 af, Depth= 3.18"

Routed to Link F6B: Flume 6B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	a (ac) C	N Des	cription		
(	).571 (	69 Past	ure/grassl	and/range,	Fair, HSG B
(	).571	100.	00% Pervi	ous Area	
To (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	44	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	171	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.4	315	Total			

### **Summary for Subcatchment Area 18: Area 18**

Runoff = 5.14 cfs @ 12.11 hrs, Volume= 0.262 af, Depth= 3.18"

Routed to Link F6B: Flume 6B

 Area	(ac) C	N Desc	cription		
0.	990 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	990	100.	00% Pervi	ous Area	
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	278	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
 4.7	424	Total			

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### **Summary for Subcatchment Area 19: Area 19**

Runoff = 6.12 cfs @ 12.11 hrs, Volume= 0.313 af, Depth= 3.18"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Desc	cription		
	1.	179 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	179	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.3	60	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	225	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
_	4.7	385	Total			11 0.000 Earth, grassed & Wilding

## **Summary for Subcatchment Area 2: Area 2**

Runoff = 2.89 cfs @ 12.11 hrs, Volume= 0.148 af, Depth= 3.18"

Routed to Link F1A: Flume 1A

Area	(ac) C	N Desc	cription		
0	.557 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0	.557	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	100	0.1950	0.39		Sheet Flow,
0.1	22	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	303	Total			

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### **Summary for Subcatchment Area 20: Area 20**

Runoff = 5.49 cfs @ 12.11 hrs, Volume= 0.280 af, Depth= 3.18"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription		
1.	.057 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	.057	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.3	57	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.6	262	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	419	Total			

## **Summary for Subcatchment Area 21: Area 21**

Runoff = 2.28 cfs @ 12.11 hrs, Volume= 0.115 af, Depth= 3.18"

Routed to Link F8B: Flume 8B

Area	(ac) C	N Desc	cription		
0.	.434 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	.434	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.1	15	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.6	259	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.5	374	Total			

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### **Summary for Subcatchment Area 22: Area 22**

Runoff = 7.45 cfs @ 12.12 hrs, Volume= 0.382 af, Depth= 3.18"

Routed to Link F8B: Flume 8B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription		
1.	.442 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	.442	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	49	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
8.0	340	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.8	489	Total			

### **Summary for Subcatchment Area 23: Area 23**

Runoff = 6.50 cfs @ 12.11 hrs, Volume= 0.332 af, Depth= 3.18"

Routed to Link F9B: Flume 9B

Area	(ac) C	N Desc	cription		
1	.252 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1	.252	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	285	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	435	Total			

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#### **Summary for Subcatchment Area 24: Area 24**

Runoff = 9.33 cfs @ 12.13 hrs, Volume= 0.489 af, Depth= 3.18"

Routed to Link F9B: Flume 9B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription		
1	.846 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1	.846	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	43	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.4	558	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5.4	701	Total			

### **Summary for Subcatchment Area 25: Area 25**

Runoff = 10.67 cfs @ 12.10 hrs, Volume= 0.531 af, Depth= 4.10"

Routed to Reach RD3: Roadside Ditch 3

	•	<i>(</i> ) 0					
_	Area	(ac) C	N Desc	cription			
	1.	024 6	9 Past	ure/grassla	and/range.	Fair, HSG B	
				el surface		,	
-					·		_
				ghted Aver			
	1.	552	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2000	
-					(013)		—
	3.8	100	0.2500	0.43		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.1	23	0.2500	3.50		Shallow Concentrated Flow,	
	0	_0	0.2000	0.00		Short Grass Pasture Kv= 7.0 fps	
_						311011 G1833 F831416 NV- 1.0 1P3	—
	3.9	123	Total				

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

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### **Summary for Subcatchment Area 26: Area 26**

Runoff = 3.67 cfs @ 12.10 hrs, Volume=

0.179 af, Depth= 3.48"

Routed to Reach RD5: Roadside Ditch 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription			_		
	0.	543	69 Past	ture/grassl	and/range,	Fair, HSG B			
_	0.	073	96 Grav	Gravel surface, HSG A					
	0.								
	0.616 72 Weighted Average 0.616 100.00% Pervious Area								
	Tc	Length	•		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_		
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.0	9	0.2500	3.50		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	3.8	109	Total						

#### **Summary for Subcatchment Area 27: Area 27**

Runoff = 0.86 cfs @ 12.07 hrs, Volume= 0.039 af, Depth= 3.18"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Des	cription			
0.	.149 6	9 Past	ure/grassl	and/range,	Fair, HSG B	
0.149 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
0.7	12	0.2500	0.28	•	Sheet Flow,	
0.5	199	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
1.2	211	Total				

#### **Summary for Subcatchment Area 28: Area 28**

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 0.033 af, Depth= 3.18"

Routed to Link F10: Flume 10

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Area	(ac) C	N Desc	cription					
0.	126 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
0.	126	100.	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.5	59	0.2500	0.39		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"	

#### **Summary for Subcatchment Area 29: Area 29**

Runoff = 6.17 cfs @ 12.13 hrs, Volume= 0.325 af, Depth= 3.18"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	1.	228 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	228	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.0	6	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	1.9	541	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding
	5.7	647	Total			

## **Summary for Subcatchment Area 3: Area 3**

Runoff = 1.80 cfs @ 12.12 hrs, Volume= 0.092 af, Depth= 3.18"

Routed to Link F2A: Flume 2A

Area (ac)	CN	Description
0.348	69	Pasture/grassland/range, Fair, HSG B
0.348		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.2	100	0.1950	0.39		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	36	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.4	162	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
						n= 0.030 Earth, grassed & winding
	4 8	298	Total			

#### **Summary for Subcatchment Area 30: Area 30**

Runoff = 2.33 cfs @ 12.10 hrs, Volume=

0.113 af, Depth= 3.18"

Routed to Link F3: Flume 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription			
0.427 69 Pasture/grassland/range, Fair, HSG B							
	0.427 100.00% Pervious Area				ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	2.9	70	0.2500	0.40		Sheet Flow,	
	0.8	227	0.0200	4.80	23.38	Grass: Short n= 0.150 P2= 2.77" <b>Trap/Vee/Rect Channel Flow, Existing Diversion Berm</b> Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding	
	0.1	65	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch	
	3.8	362	Total		·		

## **Summary for Subcatchment Area 31: Area 31**

Runoff = 1.22 cfs @ 12.10 hrs, Volume= 0.059 af, Depth= 3.18"

Routed to Link F4A: Flume 4A

_	Area (ac)	CN	Description
	0.223	69	Pasture/grassland/range, Fair, HSG B
	0.223		100.00% Pervious Area

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	77	0.2500	0.41		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.5	157	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow,
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
					n= 0.030 Earth, grassed & winding
0.1	62	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
3.7	296	Total			

### **Summary for Subcatchment Area 32: Area 32**

Runoff = 3.35 cfs @ 12.12 hrs, Volume= 0.174

0.174 af, Depth= 3.18"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	0.	655 6	Fair, HSG B			
	0.	655	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.8	100	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.1	11	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	8.0	314	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	0.3	217	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
	5.0	642	Total		•	

### **Summary for Subcatchment Area 33: Area 33**

Runoff = 1.28 cfs @ 12.10 hrs, Volume=

0.063 af, Depth= 3.18"

Routed to Link F5A: Flume 5A

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	Area	(ac) C	N Des	cription		
	0.237		9 Past	ture/grassl	and/range,	Fair, HSG B
	0.	237	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.3	83	0.2500	0.42	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.5	138	0.0200	4.80	23.38	, , , , , , , , , , , , , , , , , , ,
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
	0.0	407	0.0500	40.00	444.40	n= 0.030 Earth, grassed & winding
	0.2	127	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
_						n= 0.078 Riprap, 12-inch
	4.0	348	Total			

#### **Summary for Subcatchment Area 34: Area 34**

Runoff = 2.76 cfs @ 12.09 hrs, Volume= 0.130 af, Depth= 3.69"

Routed to Reach S5.2: Swale S5.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription					
	0.	346 6	9 Past	ture/grassl	and/range,	Fair, HSG B			
	0.	078	96 Grav	vel surface	, HSG A				
_	0.	424	74 Wei	ghted Avei	rage				
	0.	424	100.	.00% Pervi	ious Area				
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.1	78	0.2500	0.41		Sheet Flow,			
						Grass: Short	n= 0.150	P2= 2 77"	

## **Summary for Subcatchment Area 35: Area 35**

Runoff = 6.35 cfs @ 12.11 hrs, Volume= 0.322 af, Depth= 3.89" Routed to Reach S5.1 : Swale S5.1

Area (ac)	CN	Description
0.745	69	Pasture/grassland/range, Fair, HSG B
 0.248	96	Gravel surface, HSG A
 0.993	76	Weighted Average
0.993		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.3	70	0.1736	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	30	0.0050	0.58		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.1	12	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	4.4	126	Total			

### **Summary for Subcatchment Area 36: Area 36**

Runoff = 21.62 cfs @ 12.08 hrs, Volume= 1.027 af, Depth= 4.75"

Routed to Reach S1.0 : Swale S1.0

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription					
	1.120 69 Pasture/grassland/range, Fair, HSG B								
1.474 96 Gravel surface, HSG A									
2.594 84 Weighted Average									
	2.	594		, 00% Pervi					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	1.5	100	0.0140	1.11		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 2.77"			
	0.2	32	0.0140	2.40		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.5	40	0.0050	1.44		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.0	11	0.5000	4.95		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	2.2	183	Total						

### **Summary for Subcatchment Area 37: Area 37**

Runoff = 0.66 cfs @ 12.12 hrs, Volume= 0.037 af, Depth= 1.44"

Routed to Reach S1.1: Swale S1.1

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	Area	(ac) C	N Desc	cription				
0.249 39 Pasture/grassland/range, Good, HSG A								
0.057 96 Gravel surface, HSG A								
0.306 50 Weighted Average								
	0.	306	100.	00% Pervi	ous Area			
	_		0.1			D		
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.4	72	0.1736	0.35		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	8.0	28	0.0050	0.57		Sheet Flow,		
						Smooth surfaces n= 0.011 P2= 2.77"		
	0.2	19	0.0050	1.44		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	4.5	133	Total					

### **Summary for Subcatchment Area 38: Area 38**

Runoff = 6.48 cfs @ 12.11 hrs, Volume= 0.321 af, Depth= 2.69"

Routed to Reach S1.3: Swale S1.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

 Area	(ac) (	CN Des	scription					
0.395 39 Pasture/grassland/range, Good, HSG A								
0.	886	69 Pas	ture/grassl	and/range,	Fair, HSG B			
0.149 96 Gravel surface, HSG A								
1.430 64 Weighted Average								
1.	430	100	.00% Pervi	ious Area				
Tc	Length	•		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_		
3.8	100	0.2500	0.43		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
0.3	56	0.2500	3.50		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
4.1	156	Total				_		

#### **Summary for Subcatchment Area 39: Area 39**

Runoff = 3.20 cfs @ 12.11 hrs, Volume= 0.161 af, Depth= 2.60"

Routed to Reach S1.4: Swale S1.4

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Area	(ac) C	N Des	cription						
0.225 39 Pasture/grassland/range, Good, HSG A									
0.436 69 Pasture/grassland/range, Fair, HSG B									
0.	0.081 96 Gravel surface, HSG A								
0.	0.742 63 Weighted Average								
0.	742	•	00% Pervi	•					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
3.8	100	0.2500	0.43		Sheet Flow,				
					Grass: Short n= 0.150 P2= 2.77"				
0.3	23	0.0050	1.44		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
0.2	22	0.0833	2.02		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
4.3	145	Total			·				

#### **Summary for Subcatchment Area 4: Area 4**

Runoff = 1.55 cfs @ 12.10 hrs, Volume=

0.076 af, Depth= 3.18"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Desc	cription		
	0.	288 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	288	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.4	49	0.0820	0.24		Sheet Flow,
	0.6	247	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	<i>4</i> ∩	296	Total		·	

# **Summary for Subcatchment Area 40: Area 40**

Runoff = 1.57 cfs @ 12.18 hrs, Volume= 0.101 af, Depth= 1.96" Routed to Reach S1.4 : Swale S1.4

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Are	a (ac)	С	N Desc	cription		
	0.317	3	9 Past	ure/grassla	and/range,	Good, HSG A
	0.243	6			Fair, HSG B	
	0.060	Ç	<u>6 Grav</u>	<u>el surface</u>	, HSG A	
	0.620	5		ghted Aver		
	0.620		100.	00% Pervi	ous Area	
_			-			<b>-</b>
, T		ngth	Slope	Velocity	Capacity	Description
(min		eet)	(ft/ft)	(ft/sec)	(cfs)	
8.2	2	81	0.0245	0.16		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
1.0	)	19	0.2500	0.31		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
0.	1	29	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
0.2	2	20	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
0.	1	19	0.1053	2.27		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
9.0	6	168	Total			

### **Summary for Subcatchment Area 41: Area 41**

Runoff = 2.11 cfs @ 12.15 hrs, Volume=

0.120 af, Depth= 1.96"

Routed to Reach S1.5 : Swale S1.5

Area	(ac) C	N Des	cription					
0.	.389 3	39 Past	ure/grassl	Good, HSG A				
0.	.270 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
0.080 96 Gravel surface, HSG A								
0.739 56 Weighted Average								
0.	.739	100.	00% Pervi	ous Area				
Тс	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.5	49	0.0408	0.18		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
2.1	47	0.2500	0.37		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
0.2	4	0.0050	0.39		Sheet Flow,			
					Smooth surfaces n= 0.011 P2= 2.77"			
0.2	19	0.0050	1.44		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
0.2	22	0.1136	2.36		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
7.2	141	Total						

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### Summary for Subcatchment Area 42a: Area 42a

Runoff = 3.58 cfs @ 12.15 hrs, Volume= 0.202 af, Depth= 2.79"

Routed to Reach S1.7: Swale S1.7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Desc	cription				
0.249 39 Pasture/grassland/range, Good, HSG A								
	0.	489 6	39 Past	ure/grassl	and/range,	Fair, HSG B		
	0.133 96 Gravel surface, HSG A							
	0.871 65 Weighted Average							
	0.	871	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	7.3	100	0.0500	0.23		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.5	44	0.0500	1.57		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	7.8	144	Total					

## Summary for Subcatchment Area 42b: Area 42b

Runoff = 3.63 cfs @ 12.15 hrs, Volume= 0.201 af, Depth= 3.38"

Routed to Reach S1.8: Swale S1.8

	۸۳۵۵	(aa) C	N Door	orintion					
_	Area (ac) CN Description								
0.655 69 Pasture/grassland/range, Fair, HSG B									
0.057 96 Gravel surface, HSG A									
_	0.712 71 Weighted Average								
	0.	712		00% Pervi					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
_	7.3	100	0.0500	0.23		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.0	2	0.0500	1.57		Shallow Concentrated Flow,			
		_				Short Grass Pasture Kv= 7.0 fps			
-	7.3	102	Total			- ,			

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## **Summary for Subcatchment Area 43: Area 43**

Runoff = 3.63 cfs @ 12.15 hrs, Volume= 0.204 af, Depth= 3.18"

Routed to Reach S1.6: Swale S1.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C						
	0.769 69 Pasture/grassland/range, Fair, HSG B							
0.769 100.00% Pervious Area								
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
-	7.3	100	0.0500	0.23	, ,	Sheet Flow,		
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
_	77	139	Total					

#### Summary for Subcatchment Area 44: Area 44

Runoff = 0.43 cfs @ 12.47 hrs, Volume= 0.074 af, Depth= 0.63"

Routed to Reach S2.1: Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	1.	416 3	9 Past	ure/grassl	and/range,	Good, HSG A
	1.	416	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	9.5	100	0.0260	0.18		Sheet Flow,
	2.9	194	0.0260	1.13		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	9.6	647	0.0260	1.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	22.0	941	Total			

### **Summary for Subcatchment Area 45: Area 45**

Runoff = 7.53 cfs @ 12.24 hrs, Volume= 0.561 af, Depth= 2.41" Routed to Reach S1.9 : Swale S1.9

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	Area	(ac) C	N Des	cription		
	0.	797 3	39 Past	ure/grassl	and/range,	Good, HSG A
				•	•	Fair, HSG B
_	0.	057 9	<u>6 Gra</u>	<u>/el surface</u>	, HSG A	
2.792 61 Weighted Average						
	2.	792	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	7.3	100	0.0500	0.23	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.3	119	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	22	0.1905	3.06		Shallow Concentrated Flow,
	0.0	470	0.0050	0.40		Short Grass Pasture Kv= 7.0 fps
	6.0	178	0.0050	0.49		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	14.7	419	Total			

### **Summary for Subcatchment Area 46: Area 46**

Runoff = 11.02 cfs @ 12.10 hrs, Volume= 0.542 af, Depth= 3.18"

Routed to Reach S3.1: Swale S3.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription			
	2.	044 6	9 Past	ure/grassl	and/range,	Fair, HSG B	
2.044 100.00% Pervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
_	3.8	100	0.2500	0.43	,	Sheet Flow,	
	0.2	38	0.2632	3.59		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	4.0	138	Total				

### **Summary for Subcatchment Area 47: Area 47**

Runoff = 2.55 cfs @ 12.10 hrs, Volume= 0.125 af, Depth= 3.28"

Routed to Reach S3.2 : Swale S3.2

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	Area	(ac) C	N Des	Description							
	0.435 69 Pasture/grassland/range, Fair, HSG B 0.022 96 Gravel surface, HSG A										
_	0.	022	96 Grav	/ei surrace	, HSG A						
	0.457 70 Weighted Average										
	0.457 100.00% Pervious Area										
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	3.8	100	0.2500	0.43		Sheet Flow,	_				
	0.1	22		3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow,					
	0.1	22	0.2300	3.50		Short Grass Pasture Kv= 7.0 fps					
_	3.9	122	Total			<u> </u>	_				

### **Summary for Subcatchment Area 48: Area 48**

Runoff = 6.45 cfs @ 12.15 hrs, Volume= 0.357 af, Depth= 3.58"

Routed to Reach S3.4: Swale S3.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac)	CN	Desc	Description						
1.031 69 Pasture/grassland/range, Fair, HSG B											
_	0.163 96 Gravel surface, HSG A										
	1.194 73 Weighted Average										
	1.194 100.00% Pervious Area										
	Тс	Leng		Slope	Velocity	Capacity	Description				
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
	7.2	9	8	0.0500	0.23		Sheet Flow,				
							Grass: Short	n= 0.150	P2= 2.77"		

## **Summary for Subcatchment Area 49: Area 49**

Runoff = 0.44 cfs @ 12.12 hrs, Volume= 0.023 af, Depth= 3.48"

Routed to Link C6 : Culvert C6

Area (ac)	CN	Description
0.070	Pasture/grassland/range, Fair, HSG B	
 0.009	96	Gravel surface, HSG A
 0.079	72	Weighted Average
0.079		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.5	90	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.2	10	0.0500	0.14		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	10	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	15	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	18	0.1390	2.61		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	5.1	143	Total			

#### **Summary for Subcatchment Area 5: Area 5**

Runoff = 4.85 cfs @ 12.14 hrs, Volume= 0.261 af, Depth= 3.18"

Routed to Link F4A: Flume 4A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

 Area	(ac) C	N Des	cription		
0.	986 6	9 Past	ture/grassl	and/range,	Fair, HSG B
0.	986	100.	00% Pervi	ious Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.2	31	0.1000	0.24		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
2.8	69	0.2500	0.40		Sheet Flow,
0.0	9	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.5	419	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
0.1	80	0.2500	12.26	441.43	n= 0.030 Earth, grassed & winding Trap/Vee/Rect Channel Flow, Riprap Flume
0.1	00	0.2300	12.20	771.70	Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
6.6	608	Total			

### **Summary for Subcatchment Area 50: Area 50**

Runoff = 16.26 cfs @ 12.19 hrs, Volume= 1.050 af, Depth= 3.38"

Routed to Reach S3.5 : Swale S3.5

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_	Area	(ac) C	N Desc	cription		
	_					Fair, HSG B
_	0.	256 S	96 Grav	<u>/el surface</u>		
	3.	726 7	71 Weig	hted Aver	age	
3.726 100.00% Pervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.3	100	0.0500	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	83	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.9	208	0.0289	1.19		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	11.1	391	Total			·

#### **Summary for Subcatchment Area 51: Area 51**

Runoff = 0.32 cfs @ 12.19 hrs, Volume= 0.036 af, Depth= 0.63"

Routed to Reach S2.1: Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area (	(ac) C	CN Description							
	0.698 39 Pasture/grassland/range, Good, HSG A									
	0.698 100.00% Pervious Area									
(	Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	6.8	100	0.0600	0.25		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"		

## **Summary for Subcatchment Area 52: Area 52**

Runoff = 2.50 cfs @ 12.11 hrs, Volume= 0.126 af, Depth= 3.18"

Routed to Link F5B: Flume 5B

 Area (ac)	CN	Description
0.475	69	Pasture/grassland/range, Fair, HSG B
0.475		100.00% Pervious Area

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.5	90	0.2500	0.43		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
8.0	219	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
					n= 0.030 Earth, grassed & winding
0.2	116	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
4.5	425	Total			

### **Summary for Subcatchment Area 53: Area 53**

Runoff = 5.93 cfs @ 12.11 hrs, Volume= 0.300 af, Depth= 2.23"

Routed to Reach S2.3: Swale S2.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription				
	1.	.079 6	39 Past	ture/grassland/range, Fair, HSG B				
	0.	.539 3	39 Past	ture/grassl	and/range,	Good, HSG A		
1.618 59 Weighted Average								
	1.	.618	100.	00% Pervi	ous Area			
	_				_			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.1	77	0.2500	0.41		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	8.0	237	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm		
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'		
						n= 0.030 Earth, grassed & winding		
	0.3	70	0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
_	42	384	Total		•			

## Summary for Subcatchment Area 54: Area 54

Runoff = 2.95 cfs @ 12.11 hrs, Volume= 0.147 af, Depth= 2.13" Routed to Reach S2.2 : Swale S2.2

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	Area	(ac) C	N Desc	cription			
0.520 69 Pasture/grassland/range, Fair, HSG B						Fair, HSG B	
0.306 39 Pasture/grassland/range, Good, HSG A						Good, HSG A	
	0.	826 5	8 Weig	ghted Aver	age		
	0.	826	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.8	100	0.2500	0.43		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.1	26	0.2500	3.50		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	3.9	126	Total				

## **Summary for Subcatchment Area 55: Area 55**

Runoff = 3.79 cfs @ 12.11 hrs, Volume= 0.194 af, Depth= 2.13"

Routed to Reach S4.1: Swale S4.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	a (ac)	C1	N Desc	cription		
0.526 39			9 Past	ure/grassla	and/range,	Good, HSG A
(	0.405	69	9 Past	ure/grassla	and/range,	Fair, HSG B
	0.158	96	6 Grav	el surface	, HSG Å	
	1.089	58	8 Weig	ghted Aver	age	
	1.089		100.	00% Pervi	ous Area	
To	9		Slope	Velocity	Capacity	Description
(min	) (fee	et)	(ft/ft)	(ft/sec)	(cfs)	
3.5	5 6	30	0.2000	0.38		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
0.2	2 2	20	0.0500	1.34		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
0.1	1	10	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
0.5	5 4	48	0.0625	1.75		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
4.3	3 15	58	Total			

#### **Summary for Subcatchment Area 56: Area 56**

Runoff = 4.33 cfs @ 12.11 hrs, Volume= 0.221 af, Depth= 2.23"

Routed to Reach S4.2 : Swale S4.2

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	Area	(ac) (	N Des	cription					
-					.,	0 11100 4			
	_			Pasture/grassland/range, Good, HSG A					
	0.	433 (	69 Past	Pasture/grassland/range, Fair, HSG B					
	0.	181 9	96 Grav	el surface	, HSG B				
_	1.	194 :	59 Weid	hted Aver	age				
	1.	194	•	100.00% Pervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
Ī	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.6	121	0.2314	3.37		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
-	4.4	221	Total			<u> </u>			

### **Summary for Subcatchment Area 57: Area 57**

Runoff = 6.17 cfs @ 12.12 hrs, Volume=

0.330 af, Depth= 1.78"

Routed to Reach S4.3: Swale S4.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) (	CN Des	cription					
1.134 39		39 Pas	Pasture/grassland/range, Good, HSG A						
_	1.	086	69 Pas	Pasture/grassland/range, Fair, HSG B					
	2.	220	54 Wei	ghted Aver	age				
	2.	220	100	.00% Pervi	ous Area				
	Тс	Length	•		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	8.0	140	0.1857	3.02		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	4.6	240	Total						

## **Summary for Subcatchment Area 58: Area 58**

Runoff = 8.43 cfs @ 12.12 hrs, Volume= 0.441 af, Depth= 2.13"

Routed to Reach S4.4: Swale S4.4

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	Area	(ac) C	N Des	cription		
						Good, HSG A
						Fair, HSG B
	2.	476	58 Wei	ghted Aver	age	
	2.	476	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
(1	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	163	0.1718	2.90		Shallow Concentrated Flow,
	0.0	100	3	2.00		Short Grass Pasture Kv= 7.0 fps
	4.7	263	Total			Chort Grade i detaile 117 1.0 ipe

#### **Summary for Subcatchment Area 59: Area 59**

Runoff = 4.54 cfs @ 12.11 hrs, Volume= 0.238 af, Depth= 1.70"

Routed to Reach S4.5 : Swale S4.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription			
						Good, HSG A	
_	0.	785 6	69 Past	ure/grassl	and/range,	Fair, HSG B	
	1.	683 5	3 Weig	ghted Aver	age		
	1.	683	•	00% Pervi	•		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
_	3.9	100	0.2345	0.42		Sheet Flow,	_
_	0.2	46	0.2345	3.39		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	4.1	146	Total				

### Summary for Subcatchment Area 6: Area 6

Runoff = 2.81 cfs @ 12.10 hrs, Volume= 0.134 af, Depth= 3.18"

Routed to Link F6A: Flume 6A

_	Area (ac)	CN	Description
	0.504	69	Pasture/grassland/range, Fair, HSG B
Ī	0.504		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
•	2.9	72	0.2500	0.41		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.5	186	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
•	3.4	258	Total			n= 0.030 Earth, grassed & winding

#### **Summary for Subcatchment Area 60: Area 60**

Runoff = 1.18 cfs @ 12.39 hrs, Volume=

0.150 af, Depth= 0.90"

Routed to Reach S4.6: Swale S4.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription			
						Good, HSG A	
	_			el surface	•		
_	0.	130	96 Gra\	∕el surface	, HSG A		
	2.	001 4	43 Weig	ghted Aver	age		
	2.	001	•	00% Pervi	•		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	
	15.7	100	0.0074	0.11		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	4.7	169	0.0074	0.60		Shallow Concentrated Flow,	
				0.00		Short Grass Pasture Kv= 7.0 fps	
	0.4	49	0.0800	1.98		Shallow Concentrated Flow,	
	3	.0	2.2000	1.00		Short Grass Pasture Kv= 7.0 fps	
_	20.8	318	Total				

## **Summary for Subcatchment Area 61: Area 61**

Runoff = 0.97 cfs @ 12.21 hrs, Volume= 0.114 af, Depth= 0.63"

Routed to Reach S2.3 : Swale S2.3

	Area (ac)	CN	Description
	2.177	39	Pasture/grassland/range, Good, HSG A
_	2.177		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	66	0.0303	0.17		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.6	34	0.2500	0.35		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	49	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.2	149	Total			·

#### **Summary for Subcatchment Area 62: Area 62**

Runoff = 1.14 cfs @ 12.16 hrs, Volume= 0.

0.071 af, Depth= 1.44"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) (	CN Des	cription		
0.	479	39 Pas	ture/grassl	and/range,	Good, HSG A
0	.115	96 Gra	vel surface	, HSG A	
0.	594	50 Wei	ghted Aver	age	
0.	594	100.	.00% Pervi	ous Area	
Tc	Length			Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0544	0.24		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.5	47	0.0544	1.63		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.6	147	Total			

### **Summary for Subcatchment Area 63: Area 63**

Runoff = 14.43 cfs @ 12.04 hrs, Volume=

0.799 af, Depth= 6.35"

Routed to Pond Sed Pond: Sedimentation Basin

 Area	(ac)	CN	Desc	cription		
1.	509	98	Wate	er Surface,	, HSG A	
1.509 100.00% Impervious Area						a
Tc (min)	Lengt		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.0						Direct Entry,

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## **Summary for Subcatchment Area 64: Area 64**

Runoff = 2.36 cfs @ 12.21 hrs, Volume= 0.273 af, Depth= 0.63"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
_	5.	227 3	9 Past	ture/grassl	and/range,	Good, HSG A
	5.	227	100.	00% Pervi	ous Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.5	75	0.0933	0.28		Sheet Flow,
	4.0	0.5	0.0500	0.00		Grass: Short n= 0.150 P2= 2.77"
	1.3	25	0.2500	0.33		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.0	10	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.8	381	0.0265	7.85	109.92	Trap/Vee/Rect Channel Flow, Bot.W=0.00' D=2.00' Z= 4.0 & 3.0 '/' Top.W=14.00'
	0.8	162	0.2500	3.50		n= 0.030 Earth, grassed & winding  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps
_	0.5	48	0.0500	1.57		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
_	7.9	701	Total			

# **Summary for Subcatchment Area 65: Area 65**

Runoff = 1.99 cfs @ 12.64 hrs, Volume= 0.304 af, Depth= 1.20"

Routed to Reach RD1: Roadside Ditch 1

 Area (ac)	CN	Description
2.616	39	Pasture/grassland/range, Good, HSG A
0.039	69	Pasture/grassland/range, Fair, HSG B
 0.380	96	Gravel surface, HSG A
 3.035	47	Weighted Average
3.035		100.00% Pervious Area

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	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.2	100	0.0068	0.10		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	22.7	786	0.0068	0.58		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	38.9	886	Total			

# **Summary for Subcatchment Area 66: Area 66**

Runoff = 1.60 cfs @ 12.10 hrs, Volume=

0.076 af, Depth= 2.23"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area (	(ac)	CN	Desc	cription					
	0.2	213	39	Past	ure/grassla	and/range,	Good, HSG A			
0.104 69 Pasture/grassland/range, F						and/range,	Fair, HSG B			
0.092 96 Gravel surface, HSG A 0.409 59 Weighted Average										
	0.4	409		100.	00% Pervi	ous Area				
	Tc	Lengt	h	Slope	Velocity	Capacity	Description			
(n	nin)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)				
	3.1	5	2 0	).1154	0.28		Sheet Flow,			
							Grass: Short	n= 0.150	P2= 2.77"	

## **Summary for Subcatchment Area 67: Area 67**

Runoff = 1.86 cfs @ 12.11 hrs, Volume= 0.096 af, Depth= 1.53"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area		_								
	0.	524	39	Past	Pasture/grassland/range, Good, HSG A						
	0.	139	69	Past	, Fair, HSG B						
0.092 96 Gravel surface, HSG A 0.755 51 Weighted Average								_			
	0.	755		100.	00% Pervi	ous Area					
	_		_								
	Tc	Lengt	h	Slope	Velocity	Capacity	·				
	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)		_			
	3.5	8	6 (	0.2326	0.41		Sheet Flow,				

Grass: Short n= 0.150 P2= 2.77"

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## **Summary for Subcatchment Area 68: Area 68**

Runoff = 3.98 cfs @ 12.11 hrs, Volume= 0.213 af, Depth= 1.53"

Routed to Reach RD4: Roadside Ditch 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Desc	Description							
	1.	016 3	39 Past	asture/grassland/range, Good, HSG A							
	0.	620	39 Past	Pasture/grassland/range, Fair, HSG B							
	0.	035	96 Grav	el surface	, HSG A						
	1.										
	1.	671	•	ghted Aver 00% Pervi	•						
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	3.8	100	0.2500	0.43		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.77"					
	0.2	26	0.1538	2.75		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
_	4.0	126	Total			•	_				

## **Summary for Subcatchment Area 69: Area 69**

Runoff = 5.65 cfs @ 12.32 hrs, Volume= 0.684 af, Depth= 0.83"

Routed to Pond N: North Infiltration Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
_	9.	360 3	9 Past	ure/grassl	and/range.	Good, HSG A
	_			el surface	<b>O</b> /	,
-				hted Aver	•	
	_	875 ¬		00% Pervi		
	9.	010	100.	00 /0 1 CIVI	ous Alea	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Bookinplion
-	10.5	100	0.0200	0.16	(3.5)	Sheet Flow,
	10.0	100	0.0200	0.10		Grass: Short n= 0.150 P2= 2.77"
	1.7	100	0.0200	0.99		Shallow Concentrated Flow,
	1.7	100	0.0200	0.99		Short Grass Pasture Kv= 7.0 fps
	4.4	1,137	0.0193	4.31	22.20	• • • • • • • • • • • • • • • • • • •
	4.4	1,137	0.0193	4.31	32.30	Trap/Vee/Rect Channel Flow, Roadside Ditch
						Bot.W=0.00' D=1.00' Z= 5.0 & 10.0 '/' Top.W=15.00'
_						n= 0.030
	166	1 227	Total			

16.6 1,337 Total

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## **Summary for Subcatchment Area 7: Area 7**

Runoff = 4.72 cfs @ 12.12 hrs, Volume= 0.248 af, Depth= 3.18"

Routed to Link F6A: Flume 6A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Ar	ea (	(ac) C	N Desc	cription		
	0.	936 6	69 Pasture/grassland/range			Fair, HSG B
	0.	936	100.	00% Pervi	ous Area	
(mi	Гс n)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4	.1	100	0.2070	0.40		Sheet Flow,
0	.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1	.0	395	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5	.3	541	Total			

## **Summary for Subcatchment Area 70: Area 70**

Runoff = 3.41 cfs @ 12.10 hrs, Volume= 0.167 af, Depth= 2.89"

Routed to Reach S1.2 : Swale S1.2

_	Area	(ac) C	N Des	cription				
0.153 39 Pasture/grassland/range, Good, HSG A								
0.441 69 Pasture/grassland/range, Fair, HSG B								
	0.	100	96 Grav	el surface	, HSG A			
	0.	694	66 Weig	ghted Aver	age			
	0.	694	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•		
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	27	0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	3.9	127	Total			·		

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#### **Summary for Subcatchment Area 8: Area 8**

Runoff = 10.41 cfs @ 12.14 hrs, Volume= 0.560 af, Depth= 3.18"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	2.	114 6	9 Past	ture/grassl	and/range,	Fair, HSG B
_	2.	114	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	5.5	100	0.1000	0.30		Sheet Flow,
	0.2	26	0.1000	2.21		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	121	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.3	124	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	6.6	371	Total	·	·	

## **Summary for Subcatchment Area 9: Area 9**

Runoff = 4.01 cfs @ 12.14 hrs, Volume= 0.215 af, Depth= 3.18"

Routed to Link F7A: Flume 7A

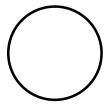
	Area	(ac) C	N Des	cription					
_	0.	810 6	9 Past	ure/grassl	and/range.	Fair, HSG B			
0.810 100.00% Pervious Area									
	0.	010	100.	00701 0101	00371100				
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Bosciption			
_					(010)	Chaot Flour			
	4.2	71	0.1000	0.28		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	1.4	29	0.2500	0.34		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.4	89	0.2500	3.50		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.3	103	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm			
	3.0		3.3200	<b>0</b> .	30.0.	Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'			
						n= 0.030 Earth, grassed & winding			
_						11- 0.000 Latti, grassed & Willuling			
	6.3	292	Total						

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#### Summary for Reach 53R: Culvert C1

Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 36.48 cfs

24.0" Round Pipe n= 0.012 Corrugated PP, smooth interior Length= 51.9' Slope= 0.0222 '/' Inlet Invert= 815.70', Outlet Invert= 814.55'



## Summary for Reach RD1: Roadside Ditch 1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 1.20" for 100-yr, 24-hr event

Inflow = 1.99 cfs @ 12.64 hrs, Volume= 0.304 af

Outflow = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af, Atten= 1%, Lag= 6.5 min

Routed to Link C1: Culvert C1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.00 fps, Min. Travel Time= 3.7 min Avg. Velocity = 0.80 fps, Avg. Travel Time= 9.2 min

Peak Storage= 434 cf @ 12.68 hrs

Average Depth at Peak Storage= 0.19', Surface Width= 6.14' Bank-Full Depth= 1.00' Flow Area= 9.5 sf, Capacity= 47.16 cfs

4.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 3.0 8.0 '/' Top Width= 15.00'

Length= 440.6' Slope= 0.0188 '/'

Inlet Invert= 824.00', Outlet Invert= 815.70'



# Summary for Reach RD2: Roadside Ditch 2

Inflow Area = 7.168 ac, 0.00% Impervious, Inflow Depth = 2.44" for 100-yr, 24-hr event

Inflow = 18.53 cfs @ 12.17 hrs, Volume= 1.460 af

Outflow = 17.64 cfs @ 12.24 hrs, Volume= 1.460 af, Atten= 5%, Lag= 4.0 min

Routed to Link C10: Culvert C10

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.14 fps, Min. Travel Time= 2.3 min

Avg. Velocity = 0.90 fps, Avg. Travel Time= 8.1 min

Peak Storage= 2,485 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.51', Surface Width= 16.30'

Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 72.77 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 433.0' Slope= 0.0162 '/'

Inlet Invert= 814.55', Outlet Invert= 807.54'



#### Summary for Reach RD3: Roadside Ditch 3

Inflow Area = 1.552 ac, 0.00% Impervious, Inflow Depth = 4.10" for 100-yr, 24-hr event

Inflow = 10.67 cfs @ 12.10 hrs, Volume= 0.531 af

Outflow = 9.18 cfs @ 12.19 hrs, Volume= 0.531 af, Atten= 14%, Lag= 5.4 min

Routed to Link C2: Culvert C2a

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.23 fps, Min. Travel Time= 3.2 min

Avg. Velocity = 1.37 fps, Avg. Travel Time= 10.0 min

Peak Storage= 1,795 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.74', Surface Width= 5.92'

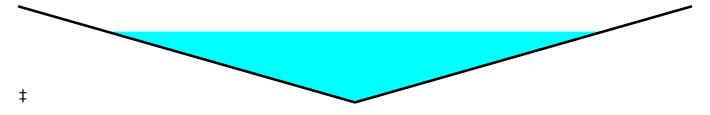
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 20.76 cfs

 $0.00' \times 1.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

Length= 821.0' Slope= 0.0288 '/'

Inlet Invert= 841.47', Outlet Invert= 817.83'



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## Summary for Reach RD4: Roadside Ditch 4

Inflow Area = 1.671 ac, 0.00% Impervious, Inflow Depth = 1.53" for 100-yr, 24-hr event

Inflow = 3.98 cfs @ 12.11 hrs, Volume= 0.213 af

Outflow = 2.99 cfs @ 12.26 hrs, Volume= 0.213 af, Atten= 25%, Lag= 9.0 min

Routed to Link C10: Culvert C10

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.52 fps, Min. Travel Time= 5.4 min

Avg. Velocity = 0.44 fps, Avg. Travel Time= 18.6 min

Peak Storage= 991 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.24', Surface Width= 10.77' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 54.26 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 495.6' Slope= 0.0090 '/'

Inlet Invert= 812.00', Outlet Invert= 807.54'



## Summary for Reach RD5: Roadside Ditch 5

Inflow Area = 0.616 ac, 0.00% Impervious, Inflow Depth = 3.48" for 100-yr, 24-hr event

Inflow = 3.67 cfs @ 12.10 hrs, Volume= 0.179 af

Outflow = 3.30 cfs @ 12.14 hrs, Volume= 0.179 af, Atten= 10%, Lag= 2.2 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.14 fps, Min. Travel Time= 1.2 min

Avg. Velocity = 1.52 fps, Avg. Travel Time= 3.2 min

Peak Storage= 238 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.46', Surface Width= 3.64'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 28.18 cfs

0.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

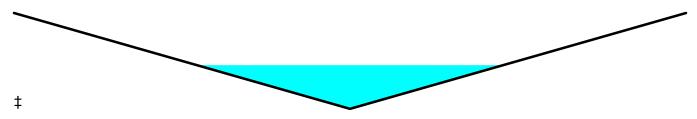
Length= 288.0' Slope= 0.0531 '/'

Inlet Invert= 841.47', Outlet Invert= 826.18'

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#### Summary for Reach S1.0: Swale S1.0

Inflow Area = 8.783 ac, 0.00% Impervious, Inflow Depth = 3.64" for 100-yr, 24-hr event

Inflow = 49.33 cfs @ 12.10 hrs, Volume= 2.668 af

Outflow = 43.23 cfs @ 12.19 hrs, Volume= 2.668 af, Atten= 12%, Lag= 5.6 min

Routed to Reach S1.2: Swale S1.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.81 fps, Min. Travel Time= 3.3 min

Avg. Velocity = 0.63 fps, Avg. Travel Time= 14.5 min

Peak Storage= 8,548 cf @ 12.14 hrs

Average Depth at Peak Storage= 1.21', Surface Width= 17.67'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 118.85 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 551.3' Slope= 0.0039 '/'

Inlet Invert= 823.92', Outlet Invert= 821.75'



### Summary for Reach S1.1: Swale S1.1

Inflow Area = 0.306 ac, 0.00% Impervious, Inflow Depth = 1.44" for 100-yr, 24-hr event

Inflow = 0.66 cfs @ 12.12 hrs, Volume= 0.037 af

Outflow = 0.61 cfs @ 12.16 hrs, Volume= 0.037 af, Atten= 7%, Lag= 2.0 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs

Max. Velocity= 1.42 fps, Min. Travel Time= 1.1 min

Avg. Velocity = 0.74 fps, Avg. Travel Time= 2.2 min

Peak Storage= 45 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.06', Surface Width= 8.45'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 376.93 cfs

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8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 98.0' Slope= 0.0396 '/' Inlet Invert= 825.88', Outlet Invert= 822.00'



#### **Summary for Reach S1.2: Swale S1.2**

Inflow Area = 9.783 ac, 0.00% Impervious, Inflow Depth = 3.52" for 100-yr, 24-hr event

Inflow = 45.42 cfs @ 12.19 hrs, Volume= 2.871 af

Outflow = 44.63 cfs @ 12.20 hrs, Volume= 2.871 af, Atten= 2%, Lag= 0.8 min

Routed to Link C11: Culvert C11

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 5.41 fps, Min. Travel Time= 0.5 min

Avg. Velocity = 1.22 fps, Avg. Travel Time= 2.3 min

Peak Storage= 1,428 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.76', Surface Width= 14.09' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 294.55 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 170.0' Slope= 0.0242 '/' Inlet Invert= 821.79', Outlet Invert= 817.68'



## Summary for Reach S1.3: Swale S1.3

Inflow Area = 11.213 ac, 0.00% Impervious, Inflow Depth = 3.42" for 100-yr, 24-hr event

Inflow = 47.67 cfs @ 12.20 hrs, Volume= 3.192 af

Outflow = 46.84 cfs @ 12.21 hrs, Volume= 3.192 af, Atten= 2%, Lag= 1.0 min

Routed to Link C3: Culvert C3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 5.86 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.34 fps, Avg. Travel Time= 2.6 min

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Peak Storage= 1,724 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.74', Surface Width= 13.92' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 324.28 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 212.6' Slope= 0.0293 '/' Inlet Invert= 817.40', Outlet Invert= 811.17'

‡

#### Summary for Reach S1.4: Swale S1.4

Inflow Area = 15.242 ac, 0.00% Impervious, Inflow Depth = 3.28" for 100-yr, 24-hr event

Inflow = 58.99 cfs @ 12.17 hrs, Volume= 4.161 af

Outflow = 58.04 cfs @ 12.18 hrs, Volume= 4.161 af, Atten= 2%, Lag= 0.6 min

Routed to Link C4: Culvert C4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.85 fps, Min. Travel Time= 0.2 min Avg. Velocity = 1.13 fps, Avg. Travel Time= 1.1 min

Peak Storage= 867 cf @ 12.18 hrs

Average Depth at Peak Storage= 1.00', Surface Width= 16.00'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 227.36 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 72.2' Slope= 0.0144 '/'

Inlet Invert= 810.90', Outlet Invert= 809.86'



#### **Summary for Reach S1.5: Swale S1.5**

Inflow Area = 15.981 ac, 0.00% Impervious, Inflow Depth = 3.21" for 100-yr, 24-hr event

Inflow = 59.91 cfs @ 12.18 hrs, Volume= 4.281 af

Outflow = 59.49 cfs @ 12.21 hrs, Volume= 4.281 af, Atten= 1%, Lag= 1.7 min

Routed to Reach S1.7: Swale S1.7

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.22 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 0.76 fps, Avg. Travel Time= 3.3 min

Peak Storage= 2,761 cf @ 12.20 hrs

Average Depth at Peak Storage= 1.38', Surface Width= 19.04'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 126.50 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 148.0' Slope= 0.0045 '/'

Inlet Invert= 809.60', Outlet Invert= 808.94'



#### Summary for Reach S1.6: Swale S1.6

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.63 cfs @ 12.15 hrs, Volume= 0.204 af

Outflow = 3.33 cfs @ 12.21 hrs, Volume= 0.204 af, Atten= 8%, Lag= 3.8 min

Routed to Reach S1.7: Swale S1.7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.35 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.36 fps, Avg. Travel Time= 8.2 min

Peak Storage= 448 cf @ 12.18 hrs

Average Depth at Peak Storage= 0.27', Surface Width= 10.19'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 134.06 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 179.7' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.95'



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#### **Summary for Reach S1.7: Swale S1.7**

Inflow Area = 18.333 ac, 0.00% Impervious, Inflow Depth = 3.20" for 100-yr, 24-hr event

Inflow = 69.04 cfs @ 12.21 hrs, Volume= 4.888 af

Outflow = 67.28 cfs @ 12.24 hrs, Volume= 4.888 af, Atten= 3%, Lag= 2.3 min

Routed to Link C5: Culvert C5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.58 fps, Min. Travel Time= 1.2 min

Avg. Velocity = 0.84 fps, Avg. Travel Time= 5.0 min

Peak Storage= 4,824 cf @ 12.22 hrs

Average Depth at Peak Storage= 1.41', Surface Width= 19.25' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 139.68 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 252.0' Slope= 0.0054 '/'

Inlet Invert= 808.94', Outlet Invert= 807.57'

‡

## Summary for Reach S1.8: Swale S1.8

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 3.38" for 100-yr, 24-hr event

Inflow = 3.63 cfs @ 12.15 hrs, Volume= 0.201 af

Outflow = 3.32 cfs @ 12.20 hrs, Volume= 0.201 af, Atten= 9%, Lag= 3.3 min

Routed to Reach S1.7: Swale S1.7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.20 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 0.63 fps, Avg. Travel Time= 6.5 min

Peak Storage= 380 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.41', Surface Width= 4.98'

Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 90.14 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 245.8' Slope= 0.0099 '/'

Inlet Invert= 810.00', Outlet Invert= 807.57'

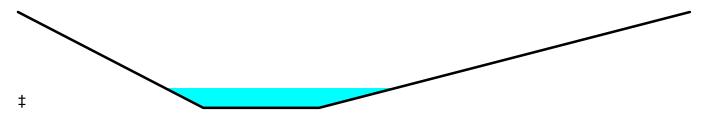
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#### Summary for Reach S1.9: Swale S1.9

Inflow Area = 21.125 ac, 0.00% Impervious, Inflow Depth = 3.10" for 100-yr, 24-hr event

Inflow = 74.80 cfs @ 12.24 hrs, Volume= 5.449 af

Outflow = 72.74 cfs @ 12.30 hrs, Volume= 5.449 af, Atten= 3%, Lag= 3.2 min

Routed to Reach S2.1: Swale S2.1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.11 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 0.96 fps, Avg. Travel Time= 7.4 min

Peak Storage= 7,601 cf @ 12.26 hrs

Average Depth at Peak Storage= 1.35', Surface Width= 18.77' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 163.67 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 422.0' Slope= 0.0075 '/'

Inlet Invert= 807.15', Outlet Invert= 804.00'



#### Summary for Reach S2.1: Swale S2.1

Inflow Area = 23.239 ac, 0.00% Impervious, Inflow Depth = 2.87" for 100-yr, 24-hr event

Inflow = 73.27 cfs @ 12.30 hrs, Volume= 5.559 af

Outflow = 71.33 cfs @ 12.35 hrs, Volume= 5.559 af, Atten= 3%, Lag= 3.2 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.74 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 0.98 fps, Avg. Travel Time= 6.6 min

Peak Storage= 7,518 cf @ 12.32 hrs

Average Depth at Peak Storage= 1.75', Surface Width= 18.04'

Bank-Full Depth= 2.00' Flow Area= 24.0 sf, Capacity= 97.05 cfs

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4.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 20.00' Length= 389.0' Slope= 0.0054 '/' Inlet Invert= 806.10', Outlet Invert= 804.00'



#### Summary for Reach S2.2: Swale S2.2

Inflow Area = 40.438 ac, 0.00% Impervious, Inflow Depth = 3.01" for 100-yr, 24-hr event

Inflow = 121.32 cfs @ 12.28 hrs, Volume= 10.154 af

Outflow = 119.32 cfs @ 12.34 hrs, Volume= 10.154 af, Atten= 2%, Lag= 3.2 min

Routed to Reach S2.3: Swale S2.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 3.97 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 0.92 fps, Avg. Travel Time= 7.5 min

Peak Storage= 12,489 cf @ 12.31 hrs

Average Depth at Peak Storage= 1.78', Surface Width= 24.21' Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 152.61 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 411.0' Slope= 0.0049 '/' Inlet Invert= 804.00', Outlet Invert= 802.00'



### Summary for Reach S2.3: Swale S2.3

Inflow Area = 44.233 ac, 0.00% Impervious, Inflow Depth = 2.87" for 100-yr, 24-hr event

Inflow = 121.74 cfs @ 12.34 hrs, Volume= 10.568 af

Outflow = 120.67 cfs @ 12.36 hrs, Volume= 10.568 af, Atten= 1%, Lag= 1.6 min

Routed to Link C7: Culvert C7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 5.67 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.32 fps, Avg. Travel Time= 3.9 min

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Peak Storage= 6,590 cf @ 12.35 hrs

Average Depth at Peak Storage= 1.38', Surface Width= 21.06' Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 249.72 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 307.0' Slope= 0.0130 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



#### Summary for Reach S3.1: Swale S3.1

Inflow Area = 2.044 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 11.02 cfs @ 12.10 hrs, Volume= 0.542 af

Outflow = 9.75 cfs @ 12.19 hrs, Volume= 0.542 af, Atten= 12%, Lag= 5.2 min

Routed to Reach S3.2: Swale S3.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.94 fps, Min. Travel Time= 3.1 min Avg. Velocity = 0.48 fps, Avg. Travel Time= 12.5 min

Peak Storage= 1,797 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.50', Surface Width= 12.02'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.76 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 357.0' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.07'



#### Summary for Reach S3.2: Swale S3.2

Inflow Area = 7.661 ac, 0.00% Impervious, Inflow Depth = 3.19" for 100-yr, 24-hr event

Inflow = 35.67 cfs @ 12.14 hrs, Volume= 2.034 af

Outflow = 35.48 cfs @ 12.14 hrs, Volume= 2.034 af, Atten= 1%, Lag= 0.3 min

Routed to Reach S3.3: Swale S3.3

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.89 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.74 fps, Avg. Travel Time= 0.8 min

Peak Storage= 420 cf @ 12.14 hrs

Average Depth at Peak Storage= 1.02', Surface Width= 16.18'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.95 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 34.0' Slope= 0.0050 '/'

Inlet Invert= 798.00', Outlet Invert= 797.83'



#### Summary for Reach S3.3: Swale S3.3

Inflow Area = 7.661 ac, 0.00% Impervious, Inflow Depth = 3.19" for 100-yr, 24-hr event

Inflow = 35.48 cfs @ 12.14 hrs, Volume= 2.034 af

Outflow = 33.84 cfs @ 12.17 hrs, Volume= 2.034 af, Atten= 5%, Lag= 1.3 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.03 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 1.02 fps, Avg. Travel Time= 3.3 min

Peak Storage= 1,746 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.78', Surface Width= 14.27'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 215.99 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 200.0' Slope= 0.0130 '/'

Inlet Invert= 808.00', Outlet Invert= 805.40'



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#### Summary for Reach S3.4: Swale S3.4

Inflow Area = 1.194 ac, 0.00% Impervious, Inflow Depth = 3.58" for 100-yr, 24-hr event

Inflow = 6.45 cfs @ 12.15 hrs, Volume= 0.357 af

Outflow = 5.88 cfs @ 12.20 hrs, Volume= 0.357 af, Atten= 9%, Lag= 3.6 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.29 fps, Min. Travel Time= 2.1 min

Avg. Velocity = 0.66 fps, Avg. Travel Time= 7.1 min

Peak Storage= 743 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.61', Surface Width= 6.15' Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 76.21 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 283.0' Slope= 0.0071 '/'

Inlet Invert= 810.00', Outlet Invert= 808.00'



# Summary for Reach S3.5: Swale S3.5

Inflow Area = 15.661 ac, 0.00% Impervious, Inflow Depth = 3.26" for 100-yr, 24-hr event

Inflow = 67.72 cfs @ 12.16 hrs, Volume= 4.259 af

Outflow = 63.93 cfs @ 12.22 hrs, Volume= 4.259 af, Atten= 6%, Lag= 3.7 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.64 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.68 fps, Avg. Travel Time= 7.8 min

Peak Storage= 7,862 cf @ 12.19 hrs

Average Depth at Peak Storage= 1.68', Surface Width= 21.43'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 93.14 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 318.5' Slope= 0.0024 '/'

Inlet Invert= 804.76', Outlet Invert= 803.99'

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#### Summary for Reach S4.1: Swale S4.1

Inflow Area = 1.364 ac, 0.00% Impervious, Inflow Depth = 2.35" for 100-yr, 24-hr event

Inflow = 5.28 cfs @ 12.10 hrs, Volume= 0.267 af

Outflow = 4.68 cfs @ 12.16 hrs, Volume= 0.267 af, Atten= 11%, Lag= 3.3 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.07 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 0.50 fps, Avg. Travel Time= 8.0 min

Peak Storage= 556 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.22', Surface Width= 11.51' Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 70.22 cfs

10.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00'

Length= 240.0' Slope= 0.0153 '/'

Inlet Invert= 811.94', Outlet Invert= 808.26'



## Summary for Reach S4.2: Swale S4.2

Inflow Area = 3.174 ac, 0.00% Impervious, Inflow Depth = 2.52" for 100-yr, 24-hr event

Inflow = 11.92 cfs @ 12.14 hrs, Volume= 0.667 af

Outflow = 11.06 cfs @ 12.18 hrs, Volume= 0.667 af, Atten= 7%, Lag= 2.8 min

Routed to Reach S4.3: Swale S4.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.70 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 0.62 fps, Avg. Travel Time= 7.0 min

Peak Storage= 1,118 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.38', Surface Width= 12.66'

Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 63.88 cfs

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10.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00' Length= 259.3' Slope= 0.0127 '/' Inlet Invert= 808.26', Outlet Invert= 804.97'



#### **Summary for Reach S4.3: Swale S4.3**

Inflow Area = 9.328 ac, 0.00% Impervious, Inflow Depth = 2.62" for 100-yr, 24-hr event

Inflow = 35.42 cfs @ 12.14 hrs, Volume= 2.039 af

Outflow = 32.72 cfs @ 12.21 hrs, Volume= 2.039 af, Atten= 8%, Lag= 4.6 min

Routed to Reach S4.4: Swale S4.4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.24 fps, Min. Travel Time= 2.7 min Avg. Velocity = 0.53 fps, Avg. Travel Time= 11.4 min

Peak Storage= 5,330 cf @ 12.17 hrs Average Depth at Peak Storage= 1.07', Surface Width= 17.49' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 108.12 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00' Length= 362.9' Slope= 0.0027 '/' Inlet Invert= 804.97', Outlet Invert= 804.00'



## Summary for Reach S4.4: Swale S4.4

Inflow Area = 14.003 ac, 0.00% Impervious, Inflow Depth = 2.62" for 100-yr, 24-hr event

Inflow = 44.75 cfs @ 12.17 hrs, Volume= 3.062 af

Outflow = 42.89 cfs @ 12.26 hrs, Volume= 3.062 af, Atten= 4%, Lag= 5.4 min

Routed to Reach S4.5: Swale S4.5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.81 fps, Min. Travel Time= 2.9 min Avg. Velocity = 0.67 fps, Avg. Travel Time= 12.3 min

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Peak Storage= 7,560 cf @ 12.21 hrs

Average Depth at Peak Storage= 1.10', Surface Width= 17.71'

Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 132.85 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 495.6' Slope= 0.0040 '/'

Inlet Invert= 804.00', Outlet Invert= 802.00'



#### Summary for Reach S4.5: Swale S4.5

Inflow Area = 16.113 ac, 0.00% Impervious, Inflow Depth = 2.54" for 100-yr, 24-hr event

Inflow = 45.33 cfs @ 12.26 hrs, Volume= 3.413 af

Outflow = 44.02 cfs @ 12.31 hrs, Volume= 3.413 af, Atten= 3%, Lag= 3.2 min

Routed to Reach S4.6: Swale S4.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.85 fps, Min. Travel Time= 1.8 min

Avg. Velocity = 0.98 fps, Avg. Travel Time= 7.0 min

Peak Storage= 4,750 cf @ 12.28 hrs

Average Depth at Peak Storage= 0.88', Surface Width= 16.18'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf. Capacity= 465.89 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 411.1' Slope= 0.0097 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



# Summary for Reach S4.6: Swale S4.6

Inflow Area = 65.439 ac, 0.00% Impervious, Inflow Depth = 2.74" for 100-yr, 24-hr event

Inflow = 168.51 cfs @ 12.34 hrs, Volume= 14.952 af

Outflow = 165.78 cfs @ 12.39 hrs, Volume= 14.952 af, Atten= 2%, Lag= 2.7 min

Routed to Pond Sed Pond : Sedimentation Basin

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.01 fps, Min. Travel Time= 1.5 min

Avg. Velocity = 1.44 fps, Avg. Travel Time= 6.2 min

Peak Storage= 14,973 cf @ 12.36 hrs

Average Depth at Peak Storage= 1.73', Surface Width= 22.14'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf, Capacity= 499.25 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 537.0' Slope= 0.0112 '/'

Inlet Invert= 798.00', Outlet Invert= 792.00'



#### Summary for Reach S5.1: Swale S5.1

Inflow Area = 0.993 ac, 0.00% Impervious, Inflow Depth = 3.89" for 100-yr, 24-hr event

Inflow = 6.35 cfs @ 12.11 hrs, Volume= 0.322 af

Outflow = 5.76 cfs @ 12.19 hrs, Volume= 0.322 af, Atten= 9%, Lag= 5.0 min

Routed to Link 4L: Culvert C2b

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.36 fps, Min. Travel Time= 3.0 min

Avg. Velocity = 0.60 fps, Avg. Travel Time= 11.9 min

Peak Storage= 1,042 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.27', Surface Width= 10.15'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 235.24 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 428.0' Slope= 0.0154 '/'

Inlet Invert= 825.20', Outlet Invert= 818.60'



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## Summary for Reach S5.2: Swale S5.2

Inflow Area = 0.424 ac, 0.00% Impervious, Inflow Depth = 3.69" for 100-yr, 24-hr event

Inflow = 2.76 cfs @ 12.09 hrs, Volume= 0.130 af

Outflow = 2.42 cfs @ 12.13 hrs, Volume= 0.130 af, Atten= 12%, Lag= 2.2 min

Routed to Link C2: Culvert C2a

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.27 fps, Min. Travel Time= 1.3 min

Avg. Velocity = 0.72 fps, Avg. Travel Time= 4.2 min

Peak Storage= 211 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.13', Surface Width= 9.08'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 344.73 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 183.6' Slope= 0.0331 '/'

Inlet Invert= 823.91', Outlet Invert= 817.83'



## **Summary for Pond C8: Culvert C8**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Outflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.1 min

Primary = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Routed to Reach S4.1: Swale S4.1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 811.38' @ 12.08 hrs Surf.Area= 0.000 ac Storage= 0.000 af Flood Elev= 819.00' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= 0.2 min calculated for 0.073 af (100% of inflow)

Center-of-Mass det. time= 0.2 min (817.4 - 817.3)

Volume	Invert	Avail.Storage	Storage Description
#1	810.70'	0.001 af	3.00'D x 7.00'H Vertical Cone/Cylinder
Device	Routing	Invert O	utlet Devices
#1	Primary	L: In	2.0" Round Culvert = 85.6' CPP, square edge headwall, Ke= 0.500 llet / Outlet Invert= 810.70' / 808.60' S= 0.0245 '/' Cc= 0.900 = 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

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Primary OutFlow Max=1.49 cfs @ 12.08 hrs HW=811.35' (Free Discharge) 1=Culvert (Inlet Controls 1.49 cfs @ 2.75 fps)

#### **Summary for Pond N: North Infiltration Area**

Inflow Area = 18.714 ac, 0.00% Impervious, Inflow Depth = 1.51" for 100-yr, 24-hr event

Inflow = 25.48 cfs @ 12.26 hrs, Volume= 2.356 af

Outflow = 2.22 cfs @ 14.45 hrs, Volume= 2.356 af, Atten= 91%, Lag= 131.5 min

Primary = 2.22 cfs @ 14.45 hrs, Volume= 2.356 af

Routed to nonexistent node 1L

#1

Primary

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 805.24' @ 14.45 hrs Surf.Area= 26.590 sf Storage= 49,884 cf

Plug-Flow detention time= 286.6 min calculated for 2.354 af (100% of inflow)

Center-of-Mass det. time= 286.6 min (1,151.8 - 865.2)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	802.00'	256,56	69 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)		rf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
802.00		5,140	0	0	
804.00		17,424	22,564	22,564	
806.00		32,191	49,615	72,179	
808.00		46,130	78,321	150,500	
810.00		59,939	106,069	256,569	
Device I	Routing	Invert	Outlet Device	s	

802.00' 3.600 in/hr Exfiltration over Surface area

Primary OutFlow Max=2.22 cfs @ 14.45 hrs HW=805.24' (Free Discharge)
—1=Exfiltration (Exfiltration Controls 2.22 cfs)

## **Summary for Pond Sed Pond: Sedimentation Basin**

Inflow Area =	72.769 ac,	2.07% Impervious,	Inflow Depth = 2.6	35" for 100-yr, 24-hr event					
Inflow =	169.90 cfs @	12.39 hrs, Volume	e= 16.095 af						
Outflow =	94.78 cfs @	12.65 hrs, Volume	e= 16.095 af,	Atten= 44%, Lag= 15.8 min					
Discarded =	5.92 cfs @	12.65 hrs, Volume	e= 7.322 af						
Primary =	11.55 cfs @	12.65 hrs, Volume	e= 5.267 af						
Routed to Lin	k Wetland : We	etland							
Secondary =	33.59 cfs @	12.65 hrs, Volume	e= 2.497 af						
Routed to Link Wetland : Wetland									
Tertiary =	43.71 cfs @	12.65 hrs, Volume	e= 1.010 af						
Routed to Link Wetland : Wetland									

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Peak Elev= 793.23' @ 12.65 hrs Surf.Area= 71,081 sf Storage= 247,875 cf Flood Elev= 794.00' Surf.Area= 75,797 sf Storage= 304,443 cf

Plug-Flow detention time= 154.9 min calculated for 16.084 af (100% of inflow)

Center-of-Mass det. time= 154.9 min ( 1,001.3 - 846.4 )

Volume	Invert	Avail.Sto	rage Storag	e Description
#1	789.00'	304,44	43 cf Custo	m Stage Data (Prismatic)Listed below (Recalc)
Clayatia	on Cu	rf.Area	Inc.Store	Cum.Store
Elevatio			(cubic-feet)	
(fee		(sq-ft)		(cubic-feet)
789.0		27,325	0	0
790.0		55,972	41,649	41,649
791.0		61,532	58,752	100,401
792.0		65,703	63,618	164,018
793.0		69,675	67,689	231,707
794.0	)0	75,797	72,736	304,443
Device	Routing	Invert	Outlet Device	ces
#1	Primary	787.70'	15.0" Roun	
" '	1 mmary	707.70		CP, mitered to conform to fill, Ke= 0.700
				t Invert= 787.70' / 787.50' S= 0.0050 '/' Cc= 0.900
				oncrete pipe, straight & clean, Flow Area= 1.23 sf
#2	Device 1	791.00'		. Orifice/Grate C= 0.600
112	DOVICE 1	701.00		eir flow at low heads
#3	Device 1	790.50'		orifice/Grate X 4.00 C= 0.600
""	Bovico i	700.00		reir flow at low heads
#4	Device 1	790.00'		orifice/Grate X 4.00 C= 0.600
,, ,	DOVICE 1	700.00		reir flow at low heads
#5	Device 1	789.00'		prifice/Grate X 14.00 columns
,,, 0	201.00	. 00.00		th 6.0" cc spacing C= 0.600
				eir flow at low heads
#6	Secondary	792.50'		x 10.0' breadth Broad-Crested Rectangular Weir
""	Cocomaary	702.00	•	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
				sh) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#7	Tertiary	793.00'		x 10.0' breadth Broad-Crested Rectangular Weir
π.	i Ortion y	7 33.30		0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
				sh) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#8	Discarded	789.00'	, ,	Exfiltration over Surface area
#0	Discarded	109.00	3.000 111/111	LAIIII audii dvei Bullace alea

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**Discarded OutFlow** Max=5.92 cfs @ 12.65 hrs HW=793.23' (Free Discharge) **8=Exfiltration** (Exfiltration Controls 5.92 cfs)

Primary OutFlow Max=11.55 cfs @ 12.65 hrs HW=793.23' (Free Discharge)

-1=Culvert (Inlet Controls 11.55 cfs @ 9.41 fps)

**2=Orifice/Grate** (Passes < 35.29 cfs potential flow)

-3=Orifice/Grate (Passes < 0.11 cfs potential flow)

-4=Orifice/Grate (Passes < 0.12 cfs potential flow)

-5=Orifice/Grate (Passes < 0.94 cfs potential flow)

Secondary OutFlow Max=33.55 cfs @ 12.65 hrs HW=793.23' (Free Discharge) 6=Broad-Crested Rectangular Weir (Weir Controls 33.55 cfs @ 2.30 fps)

Tertiary OutFlow Max=43.39 cfs @ 12.65 hrs HW=793.23' (Free Discharge) 7=Broad-Crested Rectangular Weir (Weir Controls 43.39 cfs @ 1.20 fps)

### Summary for Link 4L: Culvert C2b

Inflow Area = 0.993 ac, 0.00% Impervious, Inflow Depth = 3.89" for 100-yr, 24-hr event

Inflow = 5.76 cfs @ 12.19 hrs, Volume= 0.322 af

Primary = 5.76 cfs @ 12.19 hrs, Volume= 0.322 af, Atten= 0%, Lag= 0.0 min

Routed to Link C2: Culvert C2a

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link C1: Culvert C1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 1.20" for 100-yr, 24-hr event

Inflow = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af

Primary = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link C10: Culvert C10**

Inflow Area = 8.839 ac, 0.00% Impervious, Inflow Depth = 2.27" for 100-yr, 24-hr event

Inflow = 20.55 cfs @ 12.24 hrs, Volume= 1.672 af

Primary = 20.55 cfs @ 12.24 hrs, Volume= 1.672 af, Atten= 0%, Lag= 0.0 min

Routed to Pond N: North Infiltration Area

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link C11: Culvert C11**

Inflow Area = 9.783 ac, 0.00% Impervious, Inflow Depth = 3.52" for 100-yr, 24-hr event

Inflow = 44.63 cfs @ 12.20 hrs, Volume= 2.871 af

Primary = 44.63 cfs @ 12.20 hrs, Volume= 2.871 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

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Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link C2: Culvert C2a**

Inflow Area = 2.969 ac, 0.00% Impervious, Inflow Depth = 3.97" for 100-yr, 24-hr event

Inflow = 16.75 cfs @ 12.18 hrs, Volume= 0.983 af

Primary = 16.75 cfs @ 12.18 hrs, Volume= 0.983 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link C3: Culvert C3

Inflow Area = 11.213 ac, 0.00% Impervious, Inflow Depth = 3.42" for 100-yr, 24-hr event

Inflow = 46.84 cfs @ 12.21 hrs, Volume= 3.192 af

Primary = 46.84 cfs @ 12.21 hrs, Volume= 3.192 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### **Summary for Link C4: Culvert C4**

Inflow Area = 15.242 ac, 0.00% Impervious, Inflow Depth = 3.28" for 100-yr, 24-hr event

Inflow = 58.04 cfs @ 12.18 hrs, Volume= 4.161 af

Primary = 58.04 cfs @ 12.18 hrs, Volume= 4.161 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.5: Swale S1.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link C5: Culvert C5

Inflow Area = 18.333 ac, 0.00% Impervious, Inflow Depth = 3.20" for 100-yr, 24-hr event

Inflow = 67.28 cfs @ 12.24 hrs, Volume= 4.888 af

Primary = 67.28 cfs @ 12.24 hrs, Volume= 4.888 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.9: Swale S1.9

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link C6: Culvert C6

Inflow Area = 11.935 ac, 0.00% Impervious, Inflow Depth = 3.23" for 100-yr, 24-hr event

Inflow = 52.51 cfs @ 12.15 hrs, Volume= 3.209 af

Primary = 52.51 cfs @ 12.15 hrs, Volume= 3.209 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.5 : Swale S3.5

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#### **Summary for Link C7: Culvert C7**

Inflow Area = 47.325 ac, 0.00% Impervious, Inflow Depth = 2.89" for 100-yr, 24-hr event

Inflow = 124.99 cfs @ 12.36 hrs, Volume= 11.388 af

Primary = 124.99 cfs @ 12.36 hrs, Volume= 11.388 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.6: Swale S4.6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link C9: Culvert C9**

Inflow Area = 0.306 ac, 0.00% Impervious, Inflow Depth = 1.44" for 100-yr, 24-hr event

Inflow = 0.61 cfs @ 12.16 hrs, Volume= 0.037 af

Primary = 0.61 cfs @ 12.16 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.2 : Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link F10: Flume 10**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Primary = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.0 min

Routed to Pond C8: Culvert C8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F1A: Flume 1A

Inflow Area = 1.698 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 8.37 cfs @ 12.13 hrs, Volume= 0.450 af

Primary = 8.37 cfs @ 12.13 hrs, Volume= 0.450 af, Atten= 0%, Lag= 0.0 min

Routed to Link F1B: Flume 1B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F1B: Flume 1B

Inflow Area = 3.934 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 19.59 cfs @ 12.12 hrs, Volume= 1.043 af

Primary = 19.59 cfs @ 12.12 hrs, Volume= 1.043 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.3: Swale S4.3

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#### Summary for Link F2A: Flume 2A

Inflow Area = 0.636 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.34 cfs @ 12.11 hrs, Volume= 0.169 af

Primary = 3.34 cfs @ 12.11 hrs, Volume= 0.169 af, Atten= 0%, Lag= 0.0 min

Routed to Link F2B: Flume 2B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F2B: Flume 2B

Inflow Area = 2.199 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 11.13 cfs @ 12.12 hrs, Volume= 0.583 af

Primary = 11.13 cfs @ 12.12 hrs, Volume= 0.583 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.4 : Swale S4.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F3: Flume 3

Inflow Area = 0.427 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 2.33 cfs @ 12.10 hrs, Volume= 0.113 af

Primary = 2.33 cfs @ 12.10 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.5: Swale S4.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F4A: Flume 4A

Inflow Area = 1.209 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 5.87 cfs @ 12.13 hrs, Volume= 0.320 af

Primary = 5.87 cfs @ 12.13 hrs, Volume= 0.320 af, Atten= 0%, Lag= 0.0 min

Routed to Link F4B: Flume 4B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F4B: Flume 4B

Inflow Area = 3.092 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 15.35 cfs @ 12.13 hrs, Volume= 0.820 af

Primary = 15.35 cfs @ 12.13 hrs, Volume= 0.820 af, Atten= 0%, Lag= 0.0 min

Routed to Link C7: Culvert C7

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## Summary for Link F5A: Flume 5A

Inflow Area = 0.237 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.28 cfs @ 12.10 hrs, Volume= 0.063 af

Primary = 1.28 cfs @ 12.10 hrs, Volume= 0.063 af, Atten= 0%, Lag= 0.0 min

Routed to Link F5B: Flume 5B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F5B: Flume 5B

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.77 cfs @ 12.11 hrs, Volume= 0.189 af

Primary = 3.77 cfs @ 12.11 hrs, Volume= 0.189 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S2.2 : Swale S2.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link F6A: Flume 6A**

Inflow Area = 1.440 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 7.42 cfs @ 12.11 hrs, Volume= 0.382 af

Primary = 7.42 cfs @ 12.11 hrs, Volume= 0.382 af, Atten= 0%, Lag= 0.0 min

Routed to Link F6B: Flume 6B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F6B: Flume 6B

Inflow Area = 3.001 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 15.57 cfs @ 12.11 hrs, Volume= 0.795 af

Primary = 15.57 cfs @ 12.11 hrs, Volume= 0.795 af, Atten= 0%, Lag= 0.0 min

Routed to Link C6: Culvert C6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F7A: Flume 7A

Inflow Area = 2.924 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 14.41 cfs @ 12.14 hrs, Volume= 0.775 af

Primary = 14.41 cfs @ 12.14 hrs, Volume= 0.775 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7B: Flume 7B

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#### **Summary for Link F7B: Flume 7B**

Inflow Area = 5.160 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 25.54 cfs @ 12.13 hrs, Volume= 1.368 af

Primary = 25.54 cfs @ 12.13 hrs, Volume= 1.368 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.2: Swale S3.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### **Summary for Link F8A: Flume 8A**

Inflow Area = 0.791 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.91 cfs @ 12.14 hrs, Volume= 0.210 af

Primary = 3.91 cfs @ 12.14 hrs, Volume= 0.210 af, Atten= 0%, Lag= 0.0 min

Routed to Link F8B: Flume 8B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F8B: Flume 8B

Inflow Area = 2.667 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 13.41 cfs @ 12.12 hrs, Volume= 0.707 af

Primary = 13.41 cfs @ 12.12 hrs, Volume= 0.707 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F9A: Flume 9A

Inflow Area = 3.091 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 15.16 cfs @ 12.14 hrs, Volume= 0.819 af

Primary = 15.16 cfs @ 12.14 hrs, Volume= 0.819 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9B: Flume 9B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F9B: Flume 9B

Inflow Area = 6.189 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 30.63 cfs @ 12.13 hrs, Volume= 1.640 af

Primary = 30.63 cfs @ 12.13 hrs, Volume= 1.640 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.0: Swale S1.0

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MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

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# **Summary for Link Wetland: Wetland**

Inflow Area = 72.769 ac, 2.07% Impervious, Inflow Depth = 1.45" for 100-yr, 24-hr event

Inflow = 88.85 cfs @ 12.65 hrs, Volume= 8.773 af

Primary = 88.85 cfs @ 12.65 hrs, Volume= 8.773 af, Atten= 0%, Lag= 0.0 min

Routed to nonexistent node 1L



SCS ENGIN	Sheet No: 1 of 3		
	Calc. No.		
		Rev. No.	2
Job No. 25222260.00	Project: Columbia Energy Center MOD 12	By: SJL	Date: 8/28/23
Client: WPL	Subject: Swale Sizing	Chk'd: RJG	Date: 8/28/23

#### Purpose:

To size the proposed swales to accommodate the 25-year, 24-hour storm event and determine required erosion matting.

- 1. WisDOT Facilities Development Manual Chapter 13, Section 30-15 Grass Lined Channels.
- 2. Design of Roadside Channels with Flexible Linings, HEC-15, USDOT FHWA.
- 3. HydroCAD Report: COL_Mod12_HydroCAD Report
- 4. Wisconsin Department of Natural Resources Conservation Practice Standard 1053 Channel Erosion Mat.

#### Approach:

Use the HydroCAD Model results to obtain the peak flow during a 25-year, 24-hour storm event.

Use Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2 (from Reference #1) to size the swale for each design swale cross section. The WisDOT spreadsheet incorporates the design guidelines and equations described in "Design of Roadside Channels with Flexible Linings", HEC-15, USDOT FHWA (Reference #2).

Confirm the swale is stable and has enough capacity for the design flow rate.

Use Standard 1053 (see Reference #4) to select appropriate erosion control mat based on shear stress and application.

#### **Assumptions:**

- 1. Swales geometry shown on the drawing set.
- 2. Assume the following parameters per Section 15.2 Grass Lining Properties from Reference #1:

Vegetation Retardance Class = C for Swales

Vegetation Condition = Good

Vegetation Growth Form = Turf

3. Assume cohesive soil type with ASTM Soil Class SC and a Plasticity Index (PI) of 16.

#### Calculations:

From the HydroCAD Report, the 25-year, 24-hour peak discharge rates in the swales are

Swales:	25-year		25-year		25-year
Swale \$1.0 =	30.9 cfs	Swale \$2.1 =	40.7 cfs	Swale \$4.1 =	2.7 cfs
Swale \$1.1 =	0.24 cfs	Swale \$2.2 =	66.4 cfs	Swale \$4.2 =	6.3 cfs
Swale \$1.2 =	27.1 cfs	Swale \$2.3 =	66.3 cfs	Swale \$4.3 =	18.8 cfs
Swale \$1.3 =	28.0 cfs	Swale \$3.1 =	6.5 cfs	Swale \$4.4 =	22.1 cfs
Swale \$1.4 =	32.9 cfs	Swale \$3.2 =	20.5 cfs	Swale \$4.5 =	22.3 cfs
Swale \$1.5 =	33.6 cfs	Swale \$3.3 =	20.4 cfs	Swale \$4.6 =	90.0 cfs
Swale \$1.6 =	2.1 cfs	Swale \$3.4 =	4.0 cfs	Swale \$5.1 =	4.1 cfs
Swale \$1.7 =	38.9 cfs	Swale \$3.5 =	39.3 cfs	Swale \$5.2 =	1.7 cfs
Swale \$1.8 =	2.2 cfs				
Swale \$1.9 =	42.0 cfs				
Roadside Ditch 1 =	0.6 cfs	Roadside Ditch 2 =	11.2 cfs	Roadside Ditch 3 =	7.0 cfs
Roadside Ditch 4 =	1.5 cfs	Roadside Ditch 5 =	2.3 cfs		

Use the WisDOT Grass Swale Design Spreadsheet (Page 2) to determine the flow depth, velocity and shear stress in the swales.

#### Results:

The swales are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

The swales are stable at the design flow rates.

Use Class I, Type B erosion mat for all swales except Roadside Ditch 3 and 5 should be Class II, Type B if regraded.

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Channel/Ditch Geometry	Swale \$1.0	Swale S1.1	Swale S1.2	Swale S1.3	Swale S1.4	Swale S1.5	Swale S1.6	Swale S1.7	Swale S1.8	Swale S1.9	Swale S2.1	Swale S2.2	Swale S2.3	Swale S3.1	Swale S3.2	Swale S3.3	Swale S3.4 =	Swale S3.5 =	Swale S4.1 =	Swale \$4.2 =	Swale S4.3 =	Swale \$4.4 =	Swale S4.5 =	Swale S4.6 =	Swale S5.1 =	Swale S5.2 =	Roadside Ditch 1 =	Roadside Ditch 2 =	Roadside Ditch 3 =	Roadside Ditch 4 =	Roadside Ditch 5 =
Channel Slope, S _o (ft/ft)	0.0039	0.0396	0.0242	0.0293	0.0144	0.0045	0.005	0.0054	0.0099	0.0075	0.0054	0.0049	0.0130	0.0050	0.0050	0.0130	0.0071	0.0024	0.0153	0.0127	0.0027	0.0040	0.0097	0.0112	0.0154	0.0331	0.0188	0.0162	0.0288	0.0090	0.0531
Channel Bottom Width, B (ft)	8	8	8	8	8	8	8	8	2.5	8	4	10	10	8	8	8	2.5	8	10	10	10	10	10	10	8	8	4	6	0	6	0
Channel Side Slope, z 1	4	4	4	4	4	4	4	4	2	4	4	4	4	4	4	4	2	4	4	4	4	4	4	4	4	4	3	10	4	10	4
Channel Side Slope, z ₂	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	3	3	3	3	3	3	4	4	8	10	4	10	4
Flow Depth, d (ft) Solve iteratively	1.72	0.065	0.78	0.73	1.05	1.69	0.46	1.67	0.58	1.52	2.00	2.03	1.38	0.78	1.30	0.88	0.85	2.28	0.29	0.47	1.49	1.38	0.96	1.70	0.39	0.19	0.20	0.62	0.85	0.33	0.49
Safety Factor, SF	1.0	1.0	1.0	1.0	1.0	3.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Vegetation/Soil Parameters																															
Vegetation Retardance Class	C	c	С	С	С	С	С	С	С	С	С	С	c	c	С	С	С	c	С	С	С	C	С	С	С	С	С	С	С	С	С
Vegetation Condition	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good
Vegetation Growth Form	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf
Soil Type	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive
D ₂₅ (in) (Set at 0.00 for cohesive soils) ASTM Soil Class	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc	sc		sc	sc		sc	sc		sc	sc	SC.
Plasticity Index. Pl	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16
Results Summary	- 10	10	16	10	16	16	10	10	10	16	16	16	10	10	10	10	10	10	16	10	10	16	10	16	10	16	16	16	- 10	10	16
Design Q (ft ³ /s)	30.9	0.2	27.1	28.0	32.9	33.6	2.1	38.9	2.2	42.0	40.0	66.4	66.3	6.5	20.5	20.4	4.0	39.3	2.7	6.3	18.8	22.1	22.3	90.0	4.1	1.7	0.6	11.2	7.0	1.5	2.3
Swales geometry shown on the drawing se	31.2	0.2	27.0	27.9	32.6	34.0	2.1	38.7	2.2	41.7	39.5	65.9	66.2	6.6	20.3	20.2	4.0	39.4	2.8	6.3	18.6	22.2	22.6	90.0	4.0	1.7	0.6	11.0	7.0	1.5	2.3
Difference Retween Design & Calc. Flow (%)	0.9%	-2.0%	-0.4%	-0.3%	-0.9%	0.9%	-1.1%	-0.4%	1 1%	+0.5%	-1.3%	-0.8%	+0.1%	1.1%	aD 9%	-0.9%	+0.1%	0.3%	0.4%	0.4%	-1.0%	0.7%	1.6%	0.0%	.0.2%	0.9%	-1.5%	-1.8%	0.6%	-1.7%	0.4%
Stable (Yes or No)	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES
Channel Parameters																															
Vegetation Height, h (ft)	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Grass Roughness Coefficient, C.	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238
Cover Factor, C ₁	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Noncohesive Soil																															
Soil Grain Roughness, n	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016
Permissible Soil Shear Stress, τ _o (lb/ft²)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Cohesive Soil																															
Porosity, e Soil Coefficient 1, c.	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Soil Coefficient 1, c ₁ Soil Coefficient 2, c ₂	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700
Soil Coefficient 2, c ₂ Soil Coefficient 3, c ₁	47.700	47,700	47.700	47,700	47,700	47.700	47,700	47,700	47.700	47,700	14.30 47.700	14.30 47.700	47,700	47,700	47.700	47.700	47.700	47.700	47.700	47,700	47.700	47,700	47,700	47,700	47,700	47,700	47.700	47.700	47.700	47,700	47.700
Soil Coefficient 4. c4	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42
Soil Coefficient 5. c.	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61
Soil Coefficient 6. cs	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010
Permissible Soil Shear Stress c. (lb/ft²)	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080
Total Permissible Shear Stress, t, (lb/ft²)	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080
Cross Sectional Area, A (ft²)	25.594	0.537	8.674	7.972	12.810	24.944	4.526	24.516	2.459	21.402	24.000	36.784	21.418	8.674	17.160	10.138	4.293	39.034	3.194	5.473	22.670	20.465	12.826	27.115	3.673	1.617	0.989	7.564	2.856	3.069	0.960
Wetted Perimeter, P (ft)	22.18	8.54	14.43	14.02	16.66	21.94	11.79	21.77	6.19	20.53	20.49	26.74	21.38	14.43	18.72	15.26	7.91	26.80	12.11	13.42	20.86	20.05	16.99	22.39	11.17	9.53	6.19	18.46	6.97	12.63	4.04
Hydraulic Radius, R (ft)	1.154	0.063	0.601	0.569	0.769	1.137	0.384	1.126	0.397	1.042	1.171	1.376	1.002	0.601	0.917	0.664	0.543	1.456	0.264	0.408	1.087	1.021	0.755	1.211	0.329	0.170	0.160	0.410	0.410	0.243	0.238
Top Width, T (ft)	21.76	8.52	14.24	13.84	16.40	21.52	11.68	21.36	5.98	20.16	20.00	26.24	21.04	14.24	18.40	15.04	7.60	26.24	12.03	13.29	20.43	19.66	16.72	21.90	11.08	9.48	6.15	18.40	6.76	12.60	3.92
Hydraulic Depth, D (ft)	1.176	0.063	0.609	0.576	0.781	1.159	0.388	1.148	0.411	1.062	1.200	1.402	1.018	0.609	0.933	0.674	0.565	1.488	0.266	0.412	1.110	1.041	0.767	1.238	0.331	0.171	0.161	0.411	0.423	0.244	0.245
Froude Number (Q design)	0.198	0.308	0.703	0.813	0.507	0.223	0.132	0.260	0.247	0.334	0.264	0.267	0.540	0.171	0.216	0.427	0.218	0.146	0.295	0.317	0.137	0.187	0.355	0.526	0.337	0.461	0.267	0.399	0.664	0.174	0.838
Channel Shear Stress, τ _o (lb/ft²)	0.28	0.16	0.91	1.04	0.69	0.32	0.12	0.38	0.25	0.49	0.39	0.42	1.12	0.19	0.29	0.54	0.24	0.22	0.25	0.32	0.18	0.25	0.46	0.85	0.32	0.35	0.19	0.41	1.52	0.14	0.79
Actual Sheer Stress, τ _e (lb/ft²) Mannings n	0.42	0.107	0.053	0.050	0.059	0.080	0.119	0.075	0.089	0.068	0.074	0.072	0.055	0.099	0.084	0.065	0.090	0.093	0.088	0.080	0.100	0.34	0.069	0.054	0.080	0.38	0.23	0.072	0.057	0.19	0.056
Average Velocity, V (ft/s)	1.21	0.107	3.13	3.51	2.57	1.35	0.119	1.59	0.089	1.96	1.67	1.81	3.10	0.099	1.19	2.01	0.090	1.01	0.088	1.15	0.100	1.08	1.74	3.32	1.10	1.07	0.62	1.48	2.43	0.113	2.34
Calculated Flow, Q (ft ³ /s)	31.2	0.43	27.0	27.9	32.6	34.0	2.1	38.7	2.2	41.7	39.5	65.9	66.2	6.6	20.3	20.2	4.0	39.4	2.8	6.3	18.6	22.2	22.6	90.0	4.0	1.7	0.62	11.0	7.0	1.5	2.3
Difference Retween Design & Calc. Flow (%)	0.9%	-2.0%	-0.4%	-0.3%	-0.9%	0.9%	-1.1%	-0.4%	1.1%	-0.5%	-1.3%	-0.8%	-0.1%	1.1%	-0.9%	-0.9%	-0.1%	0.3%	0.4%	0.4%	-1.0%	0.7%	1.6%	0.0%	-0.2%	0.9%	-1.5%	-1.8%	0.6%	-1.7%	0.4%
Effective Shear on Soil Surface, r. (lb/ft²)	0.002	0.000	0.011	0.014	0.007	0.002	0.000	0.003	0.001	0.004	0.003	0.003	0.009	0.001	0.001	0.004	0.001	0.001	0.001	0.001	0.001	0.001	0.003	0.010	0.001	0.002	0.001	0.003	0.012	0.000	0.013
Total Permissible Shear on Veg., Town (lb/ft²)	22.08	35.83	8.79	7.82	10.89	20.03	44.32	17.60	24.79	14.47	17.14	16.22	9.47	30.67	22.08	13.22	25.35	27.07	24.24	20.03	31.30	24.24	14.90	9.13	20.03	18.56	30.67	16.22	10.17	39.96	9.81
Stable (Y or N)	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES	YES

Source: Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2

version 070228

# SC

SCS EN	GINEERS	Sheet No:	3 of 3
		Calc. No.	_
		Rev. No.	2
Job No. 25220183.00	Project: Job: Columbia Energy Center	By: SJL	Date: 8/28/23
Client: WPL	Subject: Subject: Swale Sizing	Chk'd: RJG	Date: 8/28/23

## **Channel Erosion Mat**

(1053)

Wisconsin Department of Natural Resources Conservation Practice Standard

To differentiate applications WisDOT organizes erosion mats into three classes of mats, which are further broken down into various Types.

- A. Class I: A short-term duration (minimum of 6 months), light duty, organic ECRM with plastic or biodegradable netting.
  - Type A Only suitable for slope applications, not channel applications.
  - 2. Type B Double netted product for use in channels where the calculated (design) shear stress is 1.5 lbs/ft2 or
- B. Class II: A long-term duration (three years or greater), organic ECRM.
  - Type A Jute fiber only for use in channels to reinforce sod.
  - 2. Type B For use in channels where the calculated (design) shear stress is 2.0 lbs/ft2 or less. Made with plastic or biodegradable mat.
  - 3. Type C A woven mat of 100% organic material for use in channels where the calculated (design) shear stress is 2.0 lbs/ft2 or less. Applicable for use in environmentally sensitive areas where plastic netting is inappropriate.
- C. Class III: A permanent 100% synthetic ECRM or TRM. Class I, Type B erosion mat or Class II, Type B or C erosion mat must be placed over a soil filled TRM.
  - 1. Type A An ECRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft2 or less.
  - 2. Type B A TRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft2 or less.
  - 3. Type C A TRM for use in channels where the calculated (design) shear stress of 3.5 lbs/ft2 or less.
  - 4. Type D A TRM for use in channels where the calculated (design) shear stress of 5.0 lbs/ft2 or less.



SCS ENGI	NEERS	Sheet No:	1/26
		Calc. No.	
		Rev. No.	3
Job No. 25222260.00	Job: Columbia Energy Center MOD 12	By: SJL	Date: 8/28/23
Client: WPL	Subject: Culvert Sizing	Chk'd: RJG	Date: 8/28/23

#### Purpose:

To size the post closure culverts to accommodate the 25-year, 24-hour storm event.

#### References:

- 1. HY-8 7.40 Computer Model
- 2. HydroCAD Report: COL_Mod12_HydroCAD Report
- 3. Figure 1 Final Grades (Module 12)

#### Approach:

- 1. Create culvert crossing in HY-8 and input data from Reference #2 and #3.
- 2. Adjust diameter size and number of culverts in model until design flow does not over top berm/road crossing.

#### **Assumptions:**

- 1. Assume the tailwater channel data is a based on discharge swale or rock chute geometry (Reference #2).
- 2. Culverts are circular, PE Pipe with smooth interior, and with square edge with headwall.
- 3. Culvert elevatons, lengths, and slopes based on Figure 1 (Reference #3).
- 4. Roadway data for crossing based on Figure 1 (Reference #3).
- 5. Discharge flows from HydroCAD report (Refence #2).

#### Calculations:

See attached HY-8 Model output reports for C1 through C11

#### Results:

The culverts are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

Culvert	Dia. (ft)	# of Barrels	Upstream Invert (ft)	Downstream Invert (ft)	Slope (%)	Length (ft)
C1	2	1	815.70	814.55	2.22	52
C2a	1.5	2	817.60	814.00	4.83	75
C2b	1	2	818.00	817.60	1.00	40
C3	2.5	2	811.1 <i>7</i>	810.90	0.54	50
C4	2.5	2	809.86	809.60	0.52	50
C5	2.5	2	807.57	807.15	0.84	50
C6	2	2	805.40	804.76	0.61	105
C7	3.5	2	796.64	796.34	0.50	60
C8	1	1	810.70	808.60	2.45	86
C9	1	1	822.00	821.79	0.57	37
C10	2	1	807.54	806.81	0.73	100
C11	2.5	2	817.68	817.40	0.50	56

# **Culvert Data: Culvert C1**

## Site Data - Culvert C1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 815.70 ft
Outlet Station: 51.88 ft
Outlet Elevation: 814.55 ft
Number of Barrels: 1

## **Culvert Data Summary - Culvert C1**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in

Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

Table 2 - Culvert Summary Table: Culvert C1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.60 cfs	0.60 cfs	816.05	0.35	0.287	1-S2n	0.18	0.27	0.18	0.29	4.28	1.23
0.74 cfs	0.74 cfs	816.09	0.39	0.310	1-S2n	0.20	0.29	0.20	0.31	4.64	1.29
0.87 cfs	0.87 cfs	816.13	0.43	0.331	1-JS1t	0.21	0.32	1.48	0.33	0.35	1.35
1.01 cfs	1.01 cfs	816.16	0.46	0.350	1-JS1t	0.23	0.35	1.50	0.35	0.40	1.40
1.14 cfs	1.14 cfs	816.19	0.49	0.368	1-S2n	0.24	0.37	0.24	0.36	5.29	1.44
1.28 cfs	1.28 cfs	816.22	0.52	0.385	1-JS1t	0.26	0.39	1.53	0.38	0.50	1.48
1.42 cfs	1.42 cfs	816.25	0.55	0.401	1-JS1t	0.27	0.41	1.54	0.39	0.54	1.52
1.55 cfs	1.55 cfs	816.28	0.58	0.416	1-JS1t	0.28	0.43	1.56	0.41	0.59	1.55
1.69 cfs	1.69 cfs	816.30	0.60	0.430	1-JS1t	0.29	0.45	1.57	0.42	0.64	1.59
1.82 cfs	1.82 cfs	816.33	0.63	0.444	1-JS1t	0.30	0.47	1.58	0.43	0.68	1.62
1.96 cfs	1.96 cfs	816.35	0.65	0.458	1-JS1t	0.31	0.49	1.60	0.45	0.73	1.65

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 815.70 ft, Outlet Elevation (invert): 814.55 ft

Culvert Length: 51.89 ft, Culvert Slope: 0.0222

Table 3 - Downstream Channel Rating Curve (Crossing: Culvert C1)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
0.60	815.99	0.29	1.23	0.34	0.57
0.74	816.01	0.31	1.29	0.36	0.58
0.87	816.03	0.33	1.35	0.39	0.59

4.04	04605	0.05	4.40	0.44	0.50
1.01	816.05	0.35	1.40	0.41	0.59
1.14	816.06	0.36	1.44	0.43	0.60
1.28	816.08	0.38	1.48	0.45	0.60
1.42	816.09	0.39	1.52	0.46	0.60
1.55	816.11	0.41	1.55	0.48	0.61
1.69	816.12	0.42	1.59	0.49	0.61
1.82	816.13	0.43	1.62	0.51	0.61
1.96	816.15	0.45	1.65	0.52	0.62

Tailwater Channel Option: Triangular Channel

Side Slope (H:V): 6.00 (_:1) Channel Slope: 0.0188 Channel Manning's n: 0.0450 Channel Invert Elevation: 815.70 ft

# **Roadway Data for Crossing: Culvert C1**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 819.06 ft Roadway Surface: Gravel Roadway Top Width: 30.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0.60 cfs Design Flow: 0.60 cfs Maximum Flow: 1.96 cfs

Table 4 - Summary of Culvert Flows at Crossing: Culvert C1

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert C1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
816.05	0.60	0.60	0.00	1
816.09	0.74	0.74	0.00	1
816.13	0.87	0.87	0.00	1
816.16	1.01	1.01	0.00	1
816.19	1.14	1.14	0.00	1
816.22	1.28	1.28	0.00	1
816.25	1.42	1.42	0.00	1
816.28	1.55	1.55	0.00	1
816.30	1.69	1.69	0.00	1
816.33	1.82	1.82	0.00	1
816.35	1.96	1.96	0.00	1
819.06	22.61	22.61	0.00	Overtopping

### Site Data - C2a

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 817.60 ft
Outlet Station: 74.54 ft
Outlet Elevation: 814.00 ft
Number of Barrels: 2

# **Culvert Data Summary - C2a**

Barrel Shape: Circular Barrel Diameter: 1.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in

Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall (Ke=0.5)

Inlet Depression: None

**Table 3 - Culvert Summary Table: C2a** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
10.41 cfs	10.41 cfs	818.90	1.30	0.0*	1-S2n	0.46	0.88	0.46	0.52	11.19	1.99
11.12 cfs	11.12 cfs	818.96	1.36	0.0*	1-S2n	0.48	0.91	0.51	0.54	10.53	2.03
11.84 cfs	11.84 cfs	819.02	1.42	0.0*	1-S2n	0.50	0.94	0.52	0.56	10.95	2.07
12.55 cfs	12.55 cfs	819.08	1.48	0.0*	1-S2n	0.51	0.97	0.52	0.58	11.51	2.11
13.27 cfs	13.27 cfs	819.14	1.54	0.0*	5-S2n	0.53	1.00	0.53	0.60	11.98	2.14
13.98 cfs	13.98 cfs	819.20	1.60	0.0*	5-S2n	0.54	1.02	0.57	0.61	11.33	2.18
14.70 cfs	14.70 cfs	819.27	1.67	0.0*	5-S2n	0.56	1.05	0.58	0.63	11.54	2.21
15.41 cfs	15.41 cfs	819.33	1.73	0.0*	5-S2n	0.57	1.08	0.59	0.65	11.84	2.25
16.13 cfs	16.13 cfs	819.40	1.80	0.0*	5-S2n	0.59	1.10	0.60	0.66	12.17	2.28
16.84 cfs	16.77 cfs	819.47	1.87	0.0*	5-S2n	0.60	1.12	0.60	0.68	12.77	2.31
17.56 cfs	17.05 cfs	819.50	1.90	0.0*	5-S2n	0.60	1.13	0.60	0.70	12.83	2.34

^{*} Full Flow Headwater elevation is below inlet invert.

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 817.60 ft, Outlet Elevation (invert): 814.00 ft

Culvert Length: 74.63 ft, Culvert Slope: 0.0483

**Table 5 - Downstream Channel Rating Curve (Crossing: Culvert C2a)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
10.41	814.52	0.52	1.99	0.16	0.53

11.12	814.54	0.54	2.03	0.17	0.54
11.84	814.56	0.56	2.07	0.17	0.54
12.55	814.58	0.58	2.11	0.18	0.54
13.27	814.60	0.60	2.14	0.19	0.54
13.98	814.61	0.61	2.18	0.19	0.55
14.70	814.63	0.63	2.21	0.20	0.55
15.41	814.65	0.65	2.25	0.20	0.55
16.13	814.66	0.66	2.28	0.21	0.55
16.84	814.68	0.68	2.31	0.21	0.55
17.56	814.70	0.70	2.34	0.22	0.55

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0300 Channel Invert Elevation: 814.00 ft

# **Roadway Data for Crossing: Culvert C2a**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 27.00 ft Crest Elevation: 819.46 ft Roadway Surface: Gravel Roadway Top Width: 20.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 10.41 cfs Design Flow: 10.41 cfs Maximum Flow: 17.56 cfs

Table 6 - Summary of Culvert Flows at Crossing: Culvert C2a

Headwater Elevation (ft)	Total Discharge (cfs)	C2a Discharge (cfs)	Roadway Discharge (cfs)	Iterations
818.90	10.41	10.41	0.00	1
818.96	11.12	11.12	0.00	1
819.02	11.84	11.84	0.00	1
819.08	12.55	12.55	0.00	1
819.14	13.27	13.27	0.00	1
819.20	13.98	13.98	0.00	1
819.27	14.70	14.70	0.00	1
819.33	15.41	15.41	0.00	1
819.40	16.13	16.13	0.00	1
819.47	16.84	16.77	0.05	13
819.50	17.56	17.05	0.50	7
819.46	16.69	16.69	0.00	Overtopping

#### Site Data - C2b

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 818.00 ft
Outlet Station: 40.00 ft
Outlet Elevation: 817.60 ft
Number of Barrels: 2

### **Culvert Data Summary - C2b**

Barrel Shape: Circular Barrel Diameter: 1.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall (Ke=0.5)

Inlet Depression: None

**Table 12 - Culvert Summary Table: C2b** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
3.50 cfs	3.50 cfs	819.00	0.84	1.002	1-S1f	0.47	0.56	1.00	0.20	2.23	1.98
3.73 cfs	3.73 cfs	819.04	0.88	1.035	1-S1f	0.49	0.58	1.00	0.21	2.37	2.03
3.95 cfs	3.95 cfs	819.07	0.91	1.070	1-S1f	0.51	0.60	1.00	0.22	2.52	2.07
4.18 cfs	4.18 cfs	819.11	0.95	1.106	1-S1f	0.52	0.62	1.00	0.22	2.66	2.11
4.40 cfs	4.40 cfs	819.14	0.98	1.143	1-S1f	0.54	0.63	1.00	0.23	2.80	2.15
4.63 cfs	4.63 cfs	819.18	1.01	1.183	1-S1f	0.56	0.65	1.00	0.24	2.95	2.19
4.86 cfs	4.86 cfs	819.22	1.05	1.223	4-FFf	0.57	0.67	1.00	0.24	3.09	2.23
5.08 cfs	5.08 cfs	819.27	1.09	1.266	4-FFf	0.59	0.68	1.00	0.25	3.24	2.26
5.31 cfs	5.31 cfs	819.31	1.13	1.310	4-FFf	0.61	0.70	1.00	0.26	3.38	2.30
5.53 cfs	5.53 cfs	819.36	1.16	1.356	4-FFf	0.63	0.71	1.00	0.26	3.52	2.33
5.76 cfs	5.76 cfs	819.40	1.20	1.403	4-FFf	0.64	0.73	1.00	0.27	3.67	2.36

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 818.00 ft, Outlet Elevation (invert): 817.60 ft

Culvert Length: 40.00 ft, Culvert Slope: 0.0100

Table 23 - Downstream Channel Rating Curve (Crossing: Culvert C2b)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
3.50	818.80	0.20	1.98	0.19	0.81
3.73	818.81	0.21	2.03	0.20	0.82
3.95	818.82	0.22	2.07	0.21	0.82

4.18	818.82	0.22	2.11	0.21	0.83
4.40	818.83	0.23	2.15	0.22	0.83
4.63	818.84	0.24	2.19	0.23	0.83
4.86	818.84	0.24	2.23	0.23	0.84
5.08	818.85	0.25	2.26	0.24	0.84
5.31	818.86	0.26	2.30	0.25	0.84
5.53	818.86	0.26	2.33	0.25	0.85
5.76	818.87	0.27	2.36	0.26	0.85

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0154 Channel Manning's n: 0.0300 Channel Invert Elevation: 818.60 ft

# **Roadway Data for Crossing: Culvert C2b**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 12.00 ft Crest Elevation: 819.46 ft Roadway Surface: Gravel Roadway Top Width: 19.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 3.50 cfs Design Flow: 3.50 cfs Maximum Flow: 5.76 cfs

Table 24 - Summary of Culvert Flows at Crossing: Culvert C2b

Headwater Elevation (ft)	Total Discharge (cfs)	C2b Discharge (cfs)	Roadway Discharge (cfs)	Iterations
819.00	3.50	3.50	0.00	1
819.04	3.73	3.73	0.00	1
819.07	3.95	3.95	0.00	1
819.11	4.18	4.18	0.00	1
819.14	4.40	4.40	0.00	1
819.18	4.63	4.63	0.00	1
819.22	4.86	4.86	0.00	1
819.27	5.08	5.08	0.00	1
819.31	5.31	5.31	0.00	1
819.36	5.53	5.53	0.00	1
819.40	5.76	5.76	0.00	1
819.46	6.02	6.02	0.00	Overtopping

#### Site Data - C3

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 811.17 ft
Outlet Station: 50.00 ft
Outlet Elevation: 810.90 ft
Number of Barrels: 2

# **Culvert Data Summary - C3**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 4 - Culvert Summary Table: C3** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
27.48 cfs	27.48 cfs	813.00	1.83	1.207	1-S2n	1.13	1.25	1.14	0.89	6.31	2.68
29.42 cfs	29.42 cfs	813.08	1.91	1.286	1-S2n	1.17	1.29	1.18	0.92	6.43	2.73
31.35 cfs	31.35 cfs	813.16	1.99	1.366	1-S2n	1.22	1.34	1.23	0.95	6.54	2.79
33.29 cfs	33.29 cfs	813.23	2.06	1.446	1-S2n	1.26	1.38	1.27	0.98	6.64	2.84
35.22 cfs	35.22 cfs	813.31	2.14	1.528	1-S2n	1.31	1.42	1.31	1.01	6.73	2.88
37.16 cfs	37.16 cfs	813.38	2.21	1.611	1-S2n	1.35	1.46	1.36	1.04	6.83	2.93
39.10 cfs	39.10 cfs	813.45	2.28	1.695	1-S2n	1.39	1.50	1.40	1.07	6.91	2.97
41.03 cfs	41.03 cfs	813.53	2.36	1.781	1-S2n	1.43	1.54	1.44	1.10	7.00	3.01
42.97 cfs	42.97 cfs	813.60	2.43	1.868	1-S2n	1.48	1.58	1.48	1.13	7.07	3.05
44.90 cfs	44.90 cfs	813.68	2.51	1.956	5-S2n	1.52	1.61	1.53	1.15	7.15	3.09
46.84 cfs	46.84 cfs	813.76	2.59	2.046	5-S2n	1.56	1.65	1.57	1.18	7.22	3.13

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert

Inlet Elevation (invert): 811.17 ft,

Outlet Elevation (invert): 810.90 ft

Culvert Length: 50.00 ft,

Culvert Slope: 0.0054

Table 7 - Downstream Channel Rating Curve (Crossing: Culvert C3)

Flow (cfs)	Water	Velocity	Depth (ft)	Shear (psf)	Froude
	Surface	(ft/s)			Number

	Elev (ft)				
27.48	812.06	0.89	2.68	0.28	0.57
29.42	812.10	0.92	2.73	0.29	0.58
31.35	812.13	0.95	2.79	0.30	0.58
33.29	812.16	0.98	2.84	0.31	0.58
35.22	812.19	1.01	2.88	0.32	0.58
37.16	812.22	1.04	2.93	0.33	0.59
39.10	812.25	1.07	2.97	0.33	0.59
41.03	812.27	1.10	3.01	0.34	0.59
42.97	812.30	1.13	3.05	0.35	0.59
44.90	812.33	1.15	3.09	0.36	0.59
46.84	812.35	1.18	3.13	0.37	0.60

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft

Side Slope (H:V): 4.00 (_:1)

Channel Slope: 0.0050

Channel Manning's n: 0.0300

Channel Invert Elevation: 811.17 ft

# **Roadway Data for Crossing: Culvert C3**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.00 ft

Crest Elevation: 813.80 ft

Roadway Surface: Gravel

Roadway Top Width: 30.00 ft

# **Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 27.48 cfs

Design Flow: 27.48 cfs

Maximum Flow: 46.84 cfs

### Table 8 - Summary of Culvert Flows at Crossing: Culvert C3

Headwater	Total	C3 Discharge	Roadway	Iterations
Elevation (ft)	Discharge	(cfs)	Discharge	

	(cfs)		(cfs)	
813.00	27.48	27.48	0.00	1
813.08	29.42	29.42	0.00	1
813.16	31.35	31.35	0.00	1
813.23	33.29	33.29	0.00	1
813.31	35.22	35.22	0.00	1
813.38	37.16	37.16	0.00	1
813.45	39.10	39.10	0.00	1
813.53	41.03	41.03	0.00	1
813.60	42.97	42.97	0.00	1
813.68	44.90	44.90	0.00	1
813.76	46.84	46.84	0.00	1
813.80	47.92	47.92	0.00	Overtopping

#### Site Data - C4

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 809.86 ft
Outlet Station: 50.00 ft
Outlet Elevation: 809.60 ft
Number of Barrels: 2

# **Culvert Data Summary - C4**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall (Ke=0.5)

Inlet Depression: None

**Table 7 - Culvert Summary Table: C4** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
32.85 cfs	32.85 cfs	811.91	2.05	1.438	1-S2n	1.27	1.37	1.27	0.89	6.53	3.18
35.37 cfs	35.37 cfs	812.00	2.14	1.544	1-S2n	1.32	1.42	1.33	0.93	6.66	3.25
37.89 cfs	37.89 cfs	812.10	2.24	1.653	1-S2n	1.38	1.48	1.39	0.96	6.77	3.32
40.41 cfs	40.41 cfs	812.20	2.34	1.763	1-S2n	1.44	1.53	1.44	1.00	6.88	3.38
42.93 cfs	42.93 cfs	812.29	2.43	1.876	1-S2n	1.50	1.57	1.50	1.03	6.98	3.44
45.45 cfs	45.45 cfs	812.39	2.53	1.991	5-S2n	1.55	1.62	1.56	1.06	7.07	3.50
47.96 cfs	47.96 cfs	812.49	2.63	2.108	5-S2n	1.61	1.67	1.61	1.09	7.15	3.55
50.48 cfs	50.48 cfs	812.60	2.74	2.228	5-S2n	1.67	1.71	1.67	1.12	7.24	3.61
53.00 cfs	53.00 cfs	812.70	2.84	2.350	5-S2n	1.73	1.75	1.73	1.15	7.31	3.66
55.52 cfs	55.52 cfs	812.81	2.95	2.475	5-S2n	1.79	1.80	1.79	1.18	7.36	3.71
58.04 cfs	58.04 cfs	813.00	3.07	3.139	7-M2c	1.86	1.84	1.84	1.21	7.51	3.75

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 809.86 ft, Outlet Elevation (invert): 809.60 ft

Culvert Length: 50.00 ft, Culvert Slope: 0.0052

Table 13 - Downstream Channel Rating Curve (Crossing: Culvert C4)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
32.85	810.76	0.89	3.18	0.39	0.68
35.37	810.80	0.93	3.25	0.41	0.68
37.89	810.83	0.96	3.32	0.42	0.69

40.41	810.86	1.00	3.38	0.44	0.69
42.93	810.90	1.03	3.44	0.45	0.69
45.45	810.93	1.06	3.50	0.46	0.69
47.96	810.96	1.09	3.55	0.48	0.70
50.48	810.99	1.12	3.61	0.49	0.70
53.00	811.02	1.15	3.66	0.50	0.70
55.52	811.05	1.18	3.71	0.51	0.70
58.04	811.07	1.21	3.75	0.53	0.71

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0070 Channel Manning's n: 0.0300 Channel Invert Elevation: 809.87 ft

**Roadway Data for Crossing: Culvert C4** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.00 ft Crest Elevation: 813.14 ft Roadway Surface: Gravel Roadway Top Width: 30.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 32.85 cfs Design Flow: 32.85 cfs Maximum Flow: 58.04 cfs

Table 14 - Summary of Culvert Flows at Crossing: Culvert C4

Headwater Elevation (ft)	Total Discharge (cfs)	C4 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
811.91	32.85	32.85	0.00	1
812.00	35.37	35.37	0.00	1
812.10	37.89	37.89	0.00	1
812.20	40.41	40.41	0.00	1
812.29	42.93	42.93	0.00	1
812.39	45.45	45.45	0.00	1
812.49	47.96	47.96	0.00	1
812.60	50.48	50.48	0.00	1
812.70	53.00	53.00	0.00	1
812.81	55.52	55.52	0.00	1
813.00	58.04	58.04	0.00	1
813.14	62.27	62.27	0.00	Overtopping

#### Site Data - C5

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 807.57 ft
Outlet Station: 50.00 ft
Outlet Elevation: 807.15 ft
Number of Barrels: 2

# **Culvert Data Summary - C5**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in
Barrel Manning's n: 0.0120
Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall (Ke=0.5)

Inlet Depression: None

**Table 8 - Culvert Summary Table: C5** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
38.11 cfs	38.11 cfs	809.81	2.24	1.502	1-S2n	1.20	1.48	1.25	1.06	7.80	2.95
41.03 cfs	41.03 cfs	809.92	2.35	1.631	1-S2n	1.25	1.54	1.30	1.10	7.94	3.01
43.94 cfs	43.94 cfs	810.04	2.47	1.762	1-S2n	1.31	1.59	1.36	1.14	8.08	3.07
46.86 cfs	46.86 cfs	810.15	2.58	1.897	5-S2n	1.36	1.65	1.41	1.18	8.22	3.13
49.78 cfs	49.78 cfs	810.27	2.70	2.034	5-S2n	1.41	1.70	1.46	1.22	8.34	3.18
52.70 cfs	52.70 cfs	810.40	2.83	2.175	5-S2n	1.46	1.75	1.51	1.25	8.47	3.24
55.61 cfs	55.61 cfs	810.52	2.95	2.320	5-S2n	1.51	1.80	1.57	1.29	8.59	3.29
58.53 cfs	58.53 cfs	810.66	3.09	2.468	5-S2n	1.57	1.84	1.62	1.32	8.71	3.33
61.45 cfs	61.45 cfs	810.80	3.23	2.925	5-S2n	1.62	1.89	1.67	1.35	8.82	3.38
64.36 cfs	63.52 cfs	810.90	3.33	3.019	5-S2n	1.66	1.92	1.71	1.39	8.89	3.42
67.28 cfs	64.67 cfs	810.96	3.39	3.072	5-S2n	1.68	1.94	1.73	1.42	8.94	3.47

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 807.57 ft, Outlet Elevation (invert): 807.15 ft

Culvert Length: 50.00 ft, Culvert Slope: 0.0084

Table 15 - Downstream Channel Rating Curve (Crossing: Culvert C5)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
38.11	808.63	1.06	2.95	0.33	0.59
41.03	808.67	1.10	3.01	0.34	0.59
43.94	808.71	1.14	3.07	0.36	0.59

46.86	808.75	1.18	3.13	0.37	0.60
49.78	808.79	1.22	3.18	0.38	0.60
52.70	808.82	1.25	3.24	0.39	0.60
55.61	808.86	1.29	3.29	0.40	0.60
58.53	808.89	1.32	3.33	0.41	0.60
61.45	808.92	1.35	3.38	0.42	0.61
64.36	808.96	1.39	3.42	0.43	0.61
67.28	808.99	1.42	3.47	0.44	0.61

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0300 Channel Invert Elevation: 807.57 ft

# **Roadway Data for Crossing: Culvert C5**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 28.00 ft Crest Elevation: 810.85 ft Roadway Surface: Gravel Roadway Top Width: 20.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 38.11 cfs Design Flow: 38.11 cfs Maximum Flow: 67.28 cfs

Table 16 - Summary of Culvert Flows at Crossing: Culvert C5

Headwater Elevation (ft)	Total Discharge (cfs)	C5 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
809.81	38.11	38.11	0.00	1
809.92	41.03	41.03	0.00	1
810.04	43.94	43.94	0.00	1
810.15	46.86	46.86	0.00	1
810.27	49.78	49.78	0.00	1
810.40	52.70	52.70	0.00	1
810.52	55.61	55.61	0.00	1
810.66	58.53	58.53	0.00	1
810.80	61.45	61.45	0.00	1
810.90	64.36	63.52	0.81	10
810.96	67.28	64.67	2.58	7
810.85	62.52	62.52	0.00	Overtopping

#### Site Data - C6

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 805.40 ft
Outlet Station: 104.56 ft
Outlet Elevation: 804.76 ft
Number of Barrels: 2

# **Culvert Data Summary - C6**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

**Table 1 - Culvert Summary Table: C6** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
30.03 cfs	30.03 cfs	807.77	2.37	1.750	5-S2n	1.33	1.40	1.33	0.80	6.77	3.22
32.28 cfs	32.28 cfs	807.95	2.55	1.956	5-S2n	1.40	1.45	1.41	0.84	6.84	3.30
34.53 cfs	34.53 cfs	808.15	2.75	2.170	5-S2n	1.48	1.50	1.48	0.87	6.92	3.38
36.77 cfs	36.77 cfs	808.36	2.96	2.848	7-M2c	1.57	1.54	1.54	0.90	7.07	3.45
39.02 cfs	38.66 cfs	808.54	3.14	2.934	7-M2c	1.65	1.58	1.58	0.93	7.26	3.52
41.27 cfs	39.50 cfs	808.63	3.23	2.974	7-M2c	1.70	1.60	1.60	0.97	7.35	3.58
43.52 cfs	40.15 cfs	808.69	3.29	3.007	7-M2c	1.74	1.61	1.61	1.00	7.41	3.65
45.77 cfs	40.72 cfs	808.75	3.35	3.037	7-M2c	1.78	1.62	1.62	1.02	7.47	3.71
48.01 cfs	41.23 cfs	808.81	3.41	3.066	7-M2c	2.00	1.63	1.63	1.05	7.53	3.77
50.26 cfs	41.69 cfs	808.86	3.46	3.094	7-M2c	2.00	1.64	1.64	1.08	7.58	3.82
52.51 cfs	42.13 cfs	808.91	3.51	3.122	7-M2c	2.00	1.64	1.64	1.11	7.62	3.88

### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 805.40 ft, Outlet Elevation (invert): 804.76 ft

Culvert Length: 104.56 ft, Culvert Slope: 0.0061

**Table 1 - Downstream Channel Rating Curve (Crossing: Culvert C6)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
30.03	805.36	0.80	3.22	0.35	0.68
32.28	805.40	0.84	3.30	0.37	0.68
34.53	805.43	0.87	3.38	0.38	0.68
36.77	805.46	0.90	3.45	0.39	0.69

39.02	805.49	0.93	3.52	0.41	0.69
41.27	805.53	0.97	3.58	0.42	0.69
43.52	805.56	1.00	3.65	0.43	0.70
45.77	805.58	1.02	3.71	0.45	0.70
48.01	805.61	1.05	3.77	0.46	0.70
50.26	805.64	1.08	3.82	0.47	0.70
52.51	805.67	1.11	3.88	0.48	0.71

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 10.00 ft Side Slope (H:V): 2.00 (_:1) Channel Slope: 0.0070 Channel Manning's n: 0.0300 Channel Invert Elevation: 804.56 ft

# **Roadway Data for Crossing: Culvert C6**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 15.00 ft Crest Elevation: 808.50 ft Roadway Surface: Gravel Roadway Top Width: 100.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 30.03 cfs Design Flow: 30.03cfs Maximum Flow: 52.51 cfs

Table 2 - Summary of Culvert Flows at Crossing: Culvert C6

Headwater Elevation (ft)	Total Discharge (cfs)	C6 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
807.77	30.03	30.03	0.00	1
807.95	32.28	32.28	0.00	1
808.15	34.53	34.53	0.00	1
808.36	36.77	36.77	0.00	1
808.54	39.02	38.66	0.34	12
808.63	41.27	39.50	1.75	8
808.69	43.52	40.15	3.35	7
808.75	45.77	40.72	5.03	6
808.81	48.01	41.23	6.78	6
808.86	50.26	41.69	8.56	5
808.91	52.51	42.13	10.37	5
808.50	38.24	38.24	0.00	Overtopping

#### Site Data - C7

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 796.64 ft Outlet Station: 60.20 ft Outlet Elevation: 796.34 ft Number of Barrels: 2

# **Culvert Data Summary - C7**

Barrel Shape: Circular Barrel Diameter: 3.50 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in Barrel Manning's n: 0.0240

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 5 - Culvert Summary Table: C7** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
68.10 cfs	68.10 cfs	799.54	2.62	2.904	2-M2c	2.56	1.81	1.81	1.09	6.79	4.35
73.79 cfs	73.79 cfs	799.69	2.75	3.048	2-M2c	2.74	1.89	1.89	1.14	6.98	4.46
79.48 cfs	79.48 cfs	799.83	2.89	3.191	2-M2c	2.98	1.96	1.96	1.18	7.17	4.56
85.17 cfs	85.17 cfs	799.97	3.02	3.334	2-M2c	3.50	2.03	2.03	1.23	7.35	4.65
90.86 cfs	90.86 cfs	800.12	3.15	3.477	2-M2c	3.50	2.10	2.10	1.27	7.53	4.74
96.54 cfs	96.54 cfs	800.26	3.29	3.620	7-M2c	3.50	2.17	2.17	1.31	7.71	4.82
102.23 cfs	102.23 cfs	800.41	3.43	3.765	7-M2c	3.50	2.24	2.24	1.35	7.88	4.90
107.92 cfs	107.92 cfs	800.55	3.57	3.913	7-M2c	3.50	2.30	2.30	1.39	8.06	4.98
113.61 cfs	113.61 cfs	800.71	3.71	4.065	7-M2c	3.50	2.36	2.36	1.43	8.23	5.06
119.30 cfs	119.30 cfs	800.86	3.85	4.224	7-M2c	3.50	2.42	2.42	1.47	8.41	5.13
124.99 cfs	124.99 cfs	801.03	4.00	4.393	7-M2c	3.50	2.48	2.48	1.50	8.58	5.20

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 796.64 ft, Outlet Elevation (invert): 796.34 ft

Culvert Length: 60.20 ft, Culvert Slope: 0.0050

Table 9 - Downstream Channel Rating Curve (Crossing: Culvert C7)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
68.10	796.69	1.09	4.35	0.68	0.84

73.79	796.74	1.14	4.46	0.71	0.84	
79.48	796.78	1.18	4.56	0.74	0.85	
85.17	796.83	1.23	4.65	0.77	0.85	
90.86	796.87	1.27	4.74	0.79	0.86	
96.54	796.91	1.31	4.82	0.82	0.86	
102.23	796.95	1.35	4.90	0.84	0.86	
107.92	796.99	1.39	4.98	0.87	0.87	
113.61	797.03	1.43	5.06	0.89	0.87	
119.30	797.07	1.47	5.13	0.92	0.87	
124.99	797.10	1.50	5.20	0.94	0.88	

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 10.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0100 Channel Manning's n: 0.0300 Channel Invert Elevation: 795.60 ft

# **Roadway Data for Crossing: Culvert C7**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 802.50 ft Roadway Surface: Gravel Roadway Top Width: 60.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 68.10 cfs Design Flow: 68.10 cfs Maximum Flow: 124.99 cfs

Table 10 - Summary of Culvert Flows at Crossing: Culvert C7

Headwater Elevation (ft)	Total Discharge (cfs)	C7 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
799.54	68.10	68.10	0.00	1
799.69	73.79	73.79	0.00	1
799.83	79.48	79.48	0.00	1
799.97	85.17	85.17	0.00	1
800.12	90.86	90.86	0.00	1
800.26	96.54	96.54	0.00	1
800.41	102.23	102.23	0.00	1
800.55	107.92	107.92	0.00	1
800.71	113.61	113.61	0.00	1
800.86	119.30	119.30	0.00	1
801.03	124.99	124.99	0.00	1
802.50	162.83	162.83	0.00	Overtopping

#### Site Data - C8

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 810.70 ft
Outlet Station: 85.63 ft
Outlet Elevation: 808.60 ft
Number of Barrels: 1

### **Culvert Data Summary - C8**

Barrel Shape: Circular Barrel Diameter: 1.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in
Barrel Manning's n: 0.0120
Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 6 - Culvert Summary Table: C8** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.96 cfs	0.96 cfs	811.27	0.57	0.315	1-JS1f	0.27	0.41	1.00	0.23	1.22	1.44
1.02 cfs	1.02 cfs	811.30	0.60	0.335	1-JS1f	0.28	0.43	1.00	0.24	1.30	1.47
1.09 cfs	1.09 cfs	811.32	0.62	0.356	1-JS1f	0.29	0.44	1.00	0.24	1.39	1.49
1.15 cfs	1.15 cfs	811.34	0.64	0.377	1-JS1f	0.30	0.45	1.00	0.25	1.47	1.52
1.22 cfs	1.22 cfs	811.37	0.67	0.399	1-JS1f	0.30	0.47	1.00	0.26	1.55	1.55
1.28 cfs	1.28 cfs	811.39	0.69	0.421	1-JS1f	0.31	0.48	1.00	0.27	1.63	1.57
1.34 cfs	1.34 cfs	811.41	0.71	0.444	1-JS1f	0.32	0.49	1.00	0.27	1.71	1.59
1.41 cfs	1.41 cfs	811.43	0.73	0.467	1-JS1f	0.33	0.50	1.00	0.28	1.79	1.61
1.47 cfs	1.47 cfs	811.45	0.75	0.492	1-JS1f	0.34	0.51	1.00	0.29	1.87	1.63
1.54 cfs	1.54 cfs	811.47	0.77	0.516	1-JS1f	0.34	0.53	1.00	0.29	1.96	1.65
1.60 cfs	1.60 cfs	811.49	0.79	0.542	1-JS1f	0.35	0.54	1.00	0.30	2.04	1.67

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 810.70 ft, Outlet Elevation (invert): 808.60 ft

Culvert Length: 85.63 ft, Culvert Slope: 0.0245

Table 11 - Downstream Channel Rating Curve (Crossing: Culvert C8)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
0.96	810.93	0.23	1.44	0.29	0.61
1.02	810.94	0.24	1.47	0.30	0.61
1.09	810.94	0.24	1.49	0.31	0.61

1.15	810.95	0.25	1.52	0.31	0.62
1.22	810.96	0.26	1.55	0.32	0.62
1.28	810.97	0.27	1.57	0.33	0.62
1.34	810.97	0.27	1.59	0.34	0.62
1.41	810.98	0.28	1.61	0.35	0.63
1.47	810.99	0.29	1.63	0.36	0.63
1.54	810.99	0.29	1.65	0.37	0.63
1.60	811.00	0.30	1.67	0.37	0.63

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 2.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0200 Channel Manning's n: 0.0450 Channel Invert Elevation: 810.70 ft

# **Roadway Data for Crossing: Culvert C8**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 824.00 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0.96 cfs Design Flow: 0.96 cfs Maximum Flow: 1.60 cfs

Table 12 - Summary of Culvert Flows at Crossing: Culvert C8

Headwater Elevation (ft)	Total Discharge (cfs)	C8 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
811.27	0.96	0.96	0.00	1
811.30	1.02	1.02	0.00	1
811.32	1.09	1.09	0.00	1
811.34	1.15	1.15	0.00	1
811.37	1.22	1.22	0.00	1
811.39	1.28	1.28	0.00	1
811.41	1.34	1.34	0.00	1
811.43	1.41	1.41	0.00	1
811.45	1.47	1.47	0.00	1
811.47	1.54	1.54	0.00	1
811.49	1.60	1.60	0.00	1
824.00	11.48	11.48	0.00	Overtopping

#### Site Data - C9

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 822.00 ft Outlet Station: 36.70 ft Outlet Elevation: 821.79 ft Number of Barrels: 1

# **Culvert Data Summary - C9**

Barrel Shape: Circular Barrel Diameter: 1.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 9 - Culvert Summary Table: C9** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.22 cfs	0.22 cfs	822.26	0.26	0.072	1-JS1t	0.19	0.19	0.28	0.07	1.23	0.38
0.26 cfs	0.26 cfs	822.28	0.28	0.080	1-JS1t	0.20	0.21	0.29	0.08	1.39	0.41
0.30 cfs	0.30 cfs	822.31	0.31	0.088	1-JS1t	0.22	0.22	0.29	0.08	1.55	0.43
0.34 cfs	0.34 cfs	822.33	0.33	0.096	1-JS1t	0.23	0.24	0.30	0.09	1.71	0.45
0.38 cfs	0.38 cfs	822.35	0.35	0.104	1-S2n	0.24	0.25	0.24	0.10	2.56	0.47
0.42 cfs	0.42 cfs	822.36	0.36	0.112	1-JS1t	0.25	0.27	0.31	0.10	1.99	0.49
0.45 cfs	0.45 cfs	822.38	0.38	0.119	1-JS1t	0.27	0.28	0.32	0.11	2.13	0.51
0.49 cfs	0.49 cfs	822.40	0.40	0.127	1-JS1t	0.28	0.29	0.32	0.11	2.26	0.52
0.53 cfs	0.53 cfs	822.41	0.41	0.135	1-S2n	0.29	0.30	0.29	0.12	2.83	0.54
0.57 cfs	0.57 cfs	822.43	0.43	0.142	1-S2n	0.30	0.31	0.30	0.12	2.89	0.55
0.61 cfs	0.61 cfs	822.45	0.45	0.150	1-S2n	0.31	0.32	0.31	0.13	2.94	0.57

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 822.00 ft, Outlet Elevation (invert): 821.79 ft

Culvert Length: 36.70 ft, Culvert Slope: 0.0057

**Table 17 - Downstream Channel Rating Curve (Crossing: Culvert C9)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
0.22	822.07	0.07	0.38	0.02	0.26
0.26	822.08	0.08	0.41	0.02	0.27
0.30	822.08	0.08	0.43	0.03	0.27

0.34	822.09	0.09	0.45	0.03	0.27
	022.09	0.09		0.03	
0.38	822.10	0.10	0.47	0.03	0.28
0.42	822.10	0.10	0.49	0.03	0.28
0.45	822.11	0.11	0.51	0.03	0.28
0.49	822.11	0.11	0.52	0.03	0.28
0.53	822.12	0.12	0.54	0.04	0.28
0.57	822.12	0.12	0.55	0.04	0.29
0.61	822.13	0.13	0.57	0.04	0.29

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0450 Channel Invert Elevation: 822.00 ft

# **Roadway Data for Crossing: Culvert C9**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 824.00 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0.22 cfs Design Flow: 0.22 cfs Maximum Flow: 0.61 cfs

Table 18 - Summary of Culvert Flows at Crossing: Culvert C9

Headwater Elevation (ft)	Total Discharge (cfs)	C9 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
822.26	0.22	0.22	0.00	1
822.28	0.26	0.26	0.00	1
822.31	0.30	0.30	0.00	1
822.33	0.34	0.34	0.00	1
822.35	0.38	0.38	0.00	1
822.36	0.42	0.42	0.00	1
822.38	0.45	0.45	0.00	1
822.40	0.49	0.49	0.00	1
822.41	0.53	0.53	0.00	1
822.43	0.57	0.57	0.00	1
822.45	0.61	0.61	0.00	1
824.00	4.50	4.50	0.00	Overtopping

#### Site Data - C10

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 807.54 ft
Outlet Station: 99.86 ft
Outlet Elevation: 806.81 ft
Number of Barrels: 1

### **Culvert Data Summary - C10**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in Barrel Manning's n: 0.0240 Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 10 - Culvert Summary Table: C10** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
11.21 cfs	11.21 cfs	809.68	1.98	2.141	7-M2c	2.00	1.20	1.20	0.27	5.69	2.88
12.14 cfs	12.14 cfs	809.83	2.11	2.288	7-M2c	2.00	1.25	1.25	0.28	5.87	2.96
13.08 cfs	13.08 cfs	810.00	2.24	2.465	7-M2c	2.00	1.30	1.30	0.29	6.04	3.03
14.01 cfs	14.01 cfs	810.29	2.38	2.747	7-M2c	2.00	1.35	1.35	0.30	6.22	3.10
14.95 cfs	14.95 cfs	810.59	2.52	3.054	7-M2c	2.00	1.39	1.39	0.31	6.40	3.17
15.88 cfs	15.88 cfs	810.91	2.67	3.374	7-M2c	2.00	1.44	1.44	0.32	6.57	3.23
16.81 cfs	16.81 cfs	811.25	2.83	3.710	7-M2c	2.00	1.48	1.48	0.33	6.75	3.29
17.75 cfs	17.75 cfs	811.57	3.00	4.030	7-M2c	2.00	1.52	1.52	0.34	6.94	3.35
18.68 cfs	18.68 cfs	811.93	3.18	4.386	7-M2c	2.00	1.56	1.56	0.35	7.13	3.40
19.62 cfs	19.62 cfs	812.30	3.36	4.762	7-M2c	2.00	1.59	1.59	0.36	7.32	3.46
20.55 cfs	20.55 cfs	812.69	3.56	5.146	7-M2c	2.00	1.63	1.63	0.37	7.51	3.51

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 807.54 ft, Outlet Elevation (invert): 806.81 ft

Culvert Length: 99.86 ft, Culvert Slope: 0.0073

**Table 19 - Downstream Channel Rating Curve (Crossing: Culvert C10)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
11.21	807.81	0.27	2.88	0.93	1.07
12.14	807.82	0.28	2.96	0.97	1.08
13.08	807.83	0.29	3.03	1.01	1.09

14.01	807.84	0.30	3.10	1.05	1.09
14.95	807.85	0.31	3.17	1.09	1.10
15.88	807.86	0.32	3.23	1.13	1.10
16.81	807.87	0.33	3.29	1.16	1.11
17.75	807.88	0.34	3.35	1.20	1.11
18.68	807.89	0.35	3.40	1.24	1.12
19.62	807.90	0.36	3.46	1.27	1.12
20.55	807.91	0.37	3.51	1.30	1.13

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 12.00 ft Side Slope (H:V): 10.00 (_:1) Channel Slope: 0.0560 Channel Manning's n: 0.0450 Channel Invert Elevation: 807.54 ft

# **Roadway Data for Crossing: Culvert C10**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 812.87 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 11.21 cfs Design Flow: 11.21 cfs Maximum Flow: 20.55 cfs

Table 20 - Summary of Culvert Flows at Crossing: Culvert C10

Headwater Elevation (ft)	Total Discharge (cfs)	C10 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
809.68	11.21	11.21	0.00	1
809.83	12.14	12.14	0.00	1
810.00	13.08	13.08	0.00	1
810.29	14.01	14.01	0.00	1
810.59	14.95	14.95	0.00	1
810.91	15.88	15.88	0.00	1
811.25	16.81	16.81	0.00	1
811.57	17.75	17.75	0.00	1
811.93	18.68	18.68	0.00	1
812.30	19.62	19.62	0.00	1
812.69	20.55	20.55	0.00	1
812.87	21.01	21.01	0.00	Overtopping

#### Site Data - C11

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 817.68 ft
Outlet Station: 56.00 ft
Outlet Elevation: 817.40 ft
Number of Barrels: 2

### **Culvert Data Summary - C11**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 11 - Culvert Summary Table: C11** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
26.46 cfs	26.46 cfs	819.47	1.79	1.298	1-S2n	1.13	1.22	1.13	1.08	6.11	1.99
28.28 cfs	28.28 cfs	819.55	1.87	1.367	1-S2n	1.17	1.27	1.18	1.12	6.21	2.03
30.09 cfs	30.09 cfs	819.62	1.94	1.438	1-S2n	1.22	1.31	1.22	1.15	6.31	2.06
31.91 cfs	31.91 cfs	819.69	2.01	1.509	1-S2n	1.26	1.35	1.26	1.19	6.41	2.10
33.73 cfs	33.73 cfs	819.76	2.08	1.581	1-S2n	1.30	1.39	1.31	1.23	6.50	2.13
35.55 cfs	35.55 cfs	819.83	2.15	1.654	1-S2n	1.34	1.43	1.35	1.26	6.58	2.17
37.36 cfs	37.36 cfs	819.90	2.22	1.728	1-S2n	1.39	1.46	1.39	1.29	6.66	2.20
39.18 cfs	39.18 cfs	819.97	2.29	1.803	1-S2n	1.43	1.50	1.43	1.32	6.74	2.23
41.00 cfs	41.00 cfs	820.04	2.36	1.880	1-S2n	1.47	1.54	1.47	1.36	6.81	2.25
42.81 cfs	42.81 cfs	820.11	2.43	1.958	1-S2n	1.51	1.57	1.51	1.39	6.88	2.28
44.63 cfs	44.63 cfs	820.18	2.50	2.037	1-S2n	1.55	1.61	1.56	1.42	6.94	2.31

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 817.68 ft, Outlet Elevation (invert): 817.40 ft

Culvert Length: 56.00 ft, Culvert Slope: 0.0050

**Table 21 - Downstream Channel Rating Curve (Crossing: Culvert C11)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
26.46	818.76	1.08	1.99	0.34	0.39
28.28	818.80	1.12	2.03	0.35	0.39
30.09	818.83	1.15	2.06	0.36	0.40

31.91	818.87	1.19	2.10	0.37	0.40	
33.73	818.91	1.23	2.13	0.38	0.40	
35.55	818.94	1.26	2.17	0.39	0.40	
37.36	818.97	1.29	2.20	0.40	0.40	
39.18	819.00	1.32	2.23	0.41	0.40	
41.00	819.04	1.36	2.25	0.42	0.40	
42.81	819.07	1.39	2.28	0.43	0.41	
44.63	819.10	1.42	2.31	0.44	0.41	

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0450 Channel Invert Elevation: 817.68 ft

# **Roadway Data for Crossing: Culvert C11**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 821.40 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 26.46 cfs Design Flow: 26.46 cfs Maximum Flow: 44.63 cfs

Table 22 - Summary of Culvert Flows at Crossing: Culvert C11

Headwater Elevation (ft)	Total Discharge (cfs)	C11 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
819.47	26.46	26.46	0.00	1
819.55	28.28	28.28	0.00	1
819.62	30.09	30.09	0.00	1
819.69	31.91	31.91	0.00	1
819.76	33.73	33.73	0.00	1
819.83	35.55	35.55	0.00	1
819.90	37.36	37.36	0.00	1
819.97	39.18	39.18	0.00	1
820.04	41.00	41.00	0.00	1
820.11	42.81	42.81	0.00	1
820.18	44.63	44.63	0.00	1
821.40	70.72	70.72	0.00	Overtopping



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Client: WPL	Subject: Diversion Berm Sizing	Chk'd: RJG	Date: 8/24/23

#### Purpose:

To size the post closure diversion berms on the final cover to accommodate the 25-year, 24-hour storm event.

#### References:

- 1. WisDOT Facilities Development Manual Chapter 13, Section 30-15 Grass Lined Channels.
- 2. Design of Roadside Channels with Flexible Linings, HEC-15, USDOT FHWA.
- 3. HydroCAD Report: COL_Mod12_HydroCAD Report
- 4. Wisconsin Department of Natural Resources Conservation Practice Standard 1053 Channel Erosion Mat.

#### Approach:

Use the Post Closure HydroCAD Model results to obtain the peak flow during a 25-year, 24-hour storm event along the diversion berms.

Use Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2 (from Reference #1) to size the swale for each design swale cross section. The WisDOT spreadsheet incorporates the design guidelines and equations described in "Design of Roadside Channels with Flexible Linings", HEC-15, USDOT FHWA (Reference #2).

Confirm the swale is stable and has enough capacity for the design flow rate.

#### **Assumptions:**

- 1. Assume the channel geometry is a v-notch swale with one sideslope at 4:1 and one sideslope at 2:1 and a depth of 2.0 ft.
- 2. Assume 2.0% slope along the flowpath of the diversion swale.
- 3. Assume the following parameters per Section 15.2 Grass Lining Properties from Reference #1:

Vegetation Retardance Class = C for Swales

Vegetation Condition = Good

Vegetation Growth Form = Turf

4. Assume cohesive soil type with ASTM Soil Class SC and a Plasticity Index (PI) of 16.

#### Calculations:

From the HydroCAD Report, the peak flow rate along the diversion berms are as follows:

<u>Areas</u>			<u>Areas</u>			<u>Areas</u>	
1	3.32	cfs	10	2.31	cfs	18	3.04 cfs
2	1 <b>.7</b> 1	cfs	11	2.59	cfs	19	3.35 cfs
3	1.06	cfs	12	6.39	cfs	20	3.24 cfs
4	0.92	cfs	13	4.83	cfs	21	1.35 cfs
6	1.66	cfs	14	1.94	cfs	22	4.40 cfs
7	2.81	cfs	15	1.93	cfs	23	3.84 cfs
8	6.16	cfs	16	2.80	cfs	24	5.53 cfs
9	2.37	cfs	1 <i>7</i>	1.78	cfs		

Use highest flow to confirm diversion berm functions.

Use the Grass Swale Design Spreadsheet (Page 2) to determine the flow depth, velocity and shear stress in the swales.

#### Results:

The diversion berms are adequately designed to accommodate the flows from the 25-year, 24-hour storm event. The diversion berms are stable at the design flow rates. The design flow depth of 2.0 feet maintains at least 0.5 ft of freeboard during the 25-year, 24-hour storm event. Based on shear stress, use erosion mat Class I, Type B along the flow path of the diversion berms.

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Client: WPL	Subject: Diversion Berm Sizing	Chk'd: RJG

Channel/Ditch Geometry	Area 12
-	
Channel Slope, S _o (ft/ft)	0.02
Channel Bottom Width, B (ft)	0
Channel Side Slope, z ₁	4
Channel Side Slope, z ₂	2
Flow Depth, d (ft) Solve iteratively	1.02
Safety Factor, SF	1.0
Vegetation/Soil Parameters	
Vegetation Retardance Class	С
Vegetation Condition	good
Vegetation Growth Form	turf
Soil Type	cohesive
D ₇₅ (in) (Set at 0.00 for cohesive soils)	
ASTM Soil Class	SC
Plasticity Index, Pl	16
Results Summary	
Design Q (ft ³ /s)	6.4
Calculated Q (ft ³ /s)	6.5
Difference Between Design & Calc. Flow (%)	1.9%
Stable (Yes or No)	YES
Channel Parameters	
Vegetation Height, h (ft)	0.67
Grass Roughness Coefficient, C _n	0.238
Cover Factor, C _f	0.90
Noncohesive Soil	0.00
Soil Grain Roughness, n _s	0.016
Permissible Soil Shear Stress, τ _p (lb/ft²)	N/A
Cohesive Soil	
Porosity, e	0.35
Soil Coefficient 1, c ₁	1.0700
Soil Coefficient 2, c ₂	14.30
Soil Coefficient 3, c ₃	47.700
Soil Coefficient 4, c ₄	1.42
Soil Coefficient 5, c ₅	-0.61
Soil Coefficient 6, c ₆	0.00010
Permissible Soil Shear Stress, τ _p (lb/ft²)	0.080
Total Permissible Shear Stress, τ _p (lb/ft²)	0.080
Cross Sectional Area. A (ft ² )	3.121
Wetted Perimeter, P (ft)	6.49
Hydraulic Radius, R (ft)	0.481
Top Width, T (ft)	6.12
Hydraulic Depth, D (ft)	0.510
Froude Number (Q design)	0.515
Channel Shear Stress, τ _o (lb/ft²)	0.60
Actual Sheer Stress, τ _d (lb/ft²)	1.27
Mannings n	0.062
Average Velocity, V (ft/s)	2.05
Calculated Flow, Q (ft ³ /s)	6.5
Difference Between Design & Calc. Flow (%)	1.9%
Effective Shear on Soil Surface, τ _e (lb/ft²)	0.008
Total Permissible Shear on Veg., τ _{p.veg} (lb/ft²)	12.03
Stable (Y or N)	YES

Source: Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2

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### **Channel Erosion Mat**

(1053)

Wisconsin Department of Natural Resources Conservation Practice Standard

To differentiate applications WisDOT organizes erosion mats into three classes of mats, which are further broken down into various Types.

- A. Class I: A short-term duration (minimum of 6 months), light duty, organic ECRM with plastic or biodegradable netting.
  - 1. Type A Only suitable for slope applications, not channel applications.
  - 2. Type B Double netted product for use in channels where the calculated (design) shear stress is 1.5 lbs/ft2 or less.
- B. Class II: A long-term duration (three years or greater), organic ECRM.
  - 1. Type A Jute fiber only for use in channels to reinforce sod.
  - 2. Type B For use in channels where the calculated (design) shear stress is 2.0 lbs/ft² or less. Made with plastic or biodegradable mat.
  - 3. Type C A woven mat of 100% organic material for use in channels where the calculated (design) shear stress is 2.0 lbs/ft2 or less. Applicable for use in environmentally sensitive areas where plastic netting is inappropriate.
- C. Class III: A permanent 100% synthetic ECRM or TRM. Class I, Type B erosion mat or Class II, Type B or C erosion mat must be placed over a soil filled TRM.
  - 1. Type A An ECRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft2 or less.
  - 2. Type B A TRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft² or less.
  - 3. Type C A TRM for use in channels where the calculated (design) shear stress of 3.5 lbs/ft2 or less.
  - 4. Type D A TRM for use in channels where the calculated (design) shear stress of 5.0 lbs/ft2 or less.

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Client: WPL	Subject: Diversion Berm Spacing Calculation	Chk'd: RJG	Date: 8/18/23	

#### Purpose:

Determine the spacing between diversion berms on the landfill final cover, with the goal of maintaining ≤ 3 ton/acre of soil loss along the final cover.

#### References

1. "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.

(Figure 1 on Sheet 2 and Tables 10 and 13 on Sheet 4).

- 2. Erosion and Sediment Control Handbook," Goldman, Jackson, & Bursztynsky, 1986. (Table 5.5 on Sheet 5).
- 3. Rainfed retention probabilities computed for different cropping tillage systems. Agricultural Water Management, A.W. Mills & G.W. Thomas, 1985. Table 5.10 on Sheet 3)
- 4. Columbia Energy Center POO Update Drawings

#### Approach:

Use the Universal Soil Loss Equation (USLE) to determine diversion berm spacing.

**USLE Equation:** A = R * K * LS * C * P

where: A = Average annual soil loss, tons/acre

R = Rainfall and runoff erosivity index K = Soil erodibility factor, tons/acre

LS = Slope length and steepness factor

C = Cover management factor

P = Practice factor

or 
$$LS = A$$

#### Assumptions:

A =tons/acre 3

R =145 see Figure 1 on Sheet 2 (Reference #1)

see Table 5.10 on Sheet 3 for Loamy Very Fine Sand (Reference #3) 0.38

0.0064 see Table 10 on Sheet 4, assuming 90% cover (Reference #1)

P =1.0 assume no support practice used

#### Calculation:

LS = 
$$\frac{A}{R \times K \times C \times P}$$
 =  $\frac{3}{145 \times 0.38 \times 0.0064 \times 1.0}$  = 8.51

From the LS Values Table (Sheet 5), based on the 4:1 final cover slope, the LS value of 8.51 fallso between the slope distance of 200 and 250 feet. Use linear interpolation between the LS values for 200 and 250 feet to determine the slope length value for the 4:1 slope.

LS= Slope Length @ 200 ft 8.33 LS= 9.31 Slope Length @ 250 ft

Slope length for the calculate LS factor = 209 ft

#### Results:

The maximum distance between diversion berms along the final cover to maintain less than 3 tons/acre soil loss is 209 ft.

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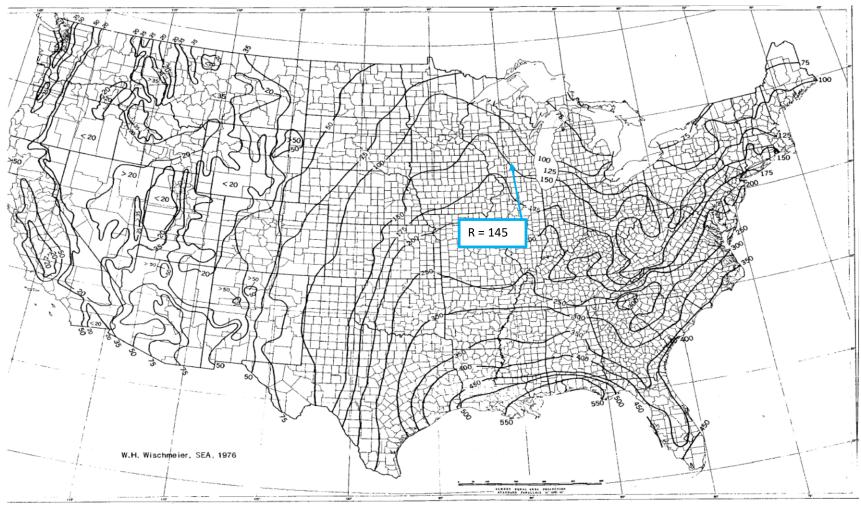


FIGURE 1.—Average annual values of the rainfall erasion index

Source: "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.

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Table 5.10. Soil Erodibility Factor  $K_{fact}$  (after Stewart et al. 1975)^(a)

	P _{om} (%)		
Textural Class	<0.5	2	4
Sand	0.05	0.03	0.02
Fine sand	0.16	0.14	0.10
Very finesand	0.42	0.36	0.28
Loamy sand	0.12	0.10	0.08
Loamy finesand	0.24	0.20	0.16
Loamy veryfine sand	0.44	0.38	0.30
Sandy loam	0.27	0.24	0.19
Fine sandyloam	0.35	0.30	0.24
Very fine sandy loam	0.47	0.41	0.33
Loam	0.38	0.34	0.29
Silt loam	0.48	0.42	0.33
Silt	0.60	0.52	0.42
Sandy clayloam	0.27	0.25	0.21
Clay loam	0.28	0.25	0.21
Silty clayloam	0.37	0.32	0.26
Sandy clay	0.14	0.13	0.12
Silty clay	0.25	0.23	0.19
Clay		0.13- 0.2	

(a) The values shown are estimated averages of broad ranges of specific soil values. When a texture is near the border line of two texture classes, use the average of the two K_{fact} values. In addition, the values shown are commensurate with the English units used in the cited reference (and as used in the source-term module input files). To obtain analagous values in the metric units used in this report, the above values should be multiplied by 1.292.

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Interpolated value C = 0.0064

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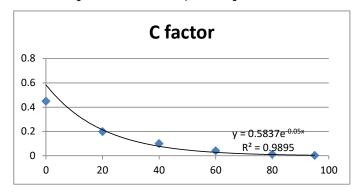
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TABLE 10.—Factor C for permanent pasture, range, and idle land¹

Vegetative cano	ру	Co	ver th	at cor	ntacts	the soi	l surfa	ce	
Type and		Percent ground cov							
height ²	cover3	Type ^t	0	20	40	60	80	95+	
No appreciable		G	0.45	0.20	0.10	0.042	0.013	0.00	
canopy		W	.45	.24	.15	.091	.043	.01	
Tall weeds or	25	G	.36	.17	.09	.038	.013	.00	
short brush with average		W	.36	.20	.13	.083	.041	.01	
drop fall height	50	G	.26	.13	.07	.035	.012	.00	
of 20 in		W	.26	.16	.11	.076	.039	.01	
	75	G	.17	.10	.06	.032	.011	.00	
		W	.17	.12	.09	.068	.038	.01	
Appreciable brush	25	G	.40	.18	.09	.040	.013	.00	
or bushes, with average drop fa	II	w	.40	.22	.14	.087	.042	.01	
height of 61/2 ft	50	G	.34	.16	.08	.038	.012	.00	
		w	.34	.19	.13	.082	.041	.01	
	75	G	.28	.14	.08	.036	.012	.00:	
		W	.28	.17	.12	.078	.040	.01	
Trees, but no	25	G	.42	.19	.10	.041	.013	.00:	
appreciable low brush. Average		W	.42	.23	.14	.089	.042	.01	
drop fall height	50	G	.39	.18	.09	.040	.013	.003	
of 13 ft		W	.39	.21	.14	.087	.042	.01	
	75	G	.36	.17	.09	.039	.012	.00:	
		w	.36	.20	.13	.084	.041	.01	

¹ The listed C values assume that the vegetation and mulch are randomly distributed over the entire area.

Source: "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.





² Canopy height is measured as the average fall height of water drops falling from the canopy to the ground. Canopy effect is inversely proportional to drop fall height and is negligible if fall height exceeds 33 ft.

³Portion of total-area surface that would be hidden from view by canopy in a vertical projection (a bird's-eye view).

⁴G: cover at surface is grass, grasslike plants, decaying compacted duff, or litter at least 2 in deep.

W: cover at surface is mostly broadleaf herbaceous plants (as weeds with little lateral-root network near the surface) or undecayed residues or both.

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TABLE 5.5 LS Values* (10)

	Slope			LS va	lues for	followi	ng slope	lengths	<i>l</i> , ft (n	1)					LS	values	for fo	llowing	g slope	lengths	l, ft (m	)		
Slope	gradient	10	20	30	40	50	60	70	80	90	100	150	200	250	300	350	400	450	500	600	700	800	900	1000
ratio	3, %	(3.0)	(6.1)	(9.1)	(12.2)	(15.2)	(18.3)	(21.3)	(24.4)	(27.4)	(30.5)	(46)	(61)	(76)	N .	(107)	(122)	(137)		(183)	(213)	(244)	(274)	(305)
	0.5	0.06	0.07	0.07	0.08	0.08	0.09	0.09	0.09	0.09	0.10	0.10	0.11	0.11	0.12	0.12	0.13	0.13	0.13	0.14	0.14	0.14	0.15	0.18
100:1	1 2	0.08	0.09			0.11	0.11	0.12	0.12	0.12	0.12		0.14	0.15	0.16	0.16	0.16		0.17	0.18	0.18	0.19	0.19	0.20
	3	0.10	0.12		0.15 $0.22$	0.16 0.23	0.17 0.25	0.18	0.19	0.19	0.20	0.23	0.25	0.26	0.28	0.29	0.30		0.33	0.34	0.36	0.37	0.39	0.40
	4	0.16	0.21	0.25	0.28	0.23	0.23	0.26	0.27	0.28 $0.38$	0.29 0.40	0.32 0.47	$0.35 \\ 0.53$	0.38 0.58	$0.40 \\ 0.62$	0.42 0.66	0.43 $0.70$		0.46 0.76	0.49 0.82	0.51 0.87	0.54 0.92	0.55 0.96	0.57
20:1	5	0.17	0.24	0.29	0.34	0.38	0.41	0.45	0.48	0.51	0.53	0.66	0.76	0.85	0.93	1.00	1.07	1.13	1.20	1.31	1.42	1.61		
	6	0.21	0.30	0.37	0.43	0.48	0.52	0.56	0.60	0.64	0.67	0.82	0.76	1.06	1.16	1.26	1.34	1.43	1.50	1.65	1.78	1.51 1.90	1.60 2.02	$\frac{1.69}{2.13}$
	7	0.26	0.37	0.45	0.52	0.58	0.64	0.69	0.74	0.78	0.82	1.01	1.17	1.30	1.43	1.54	1.65	1.75	1.84	2.02	2.18	2.33	2.47	2.61
12½:1	8	0.31	0.44	0.54	0.63	0.70	0.77	0.83	0.89	0.94	0.99	1.21	1.40	1.57	1.72	1.85	1.98	2.10	2.22	2.43	2.62	2.80	2.97	3.13
	9	0.37	0.52	0.64	0.74	0.83	0.91	0.98	1.05	1.11	1.17	1.44		1.85	2.03	2.19	2.35		2.62	2.87	3.10	3.32	3.52	3.71
10:1	10	0.43	0.61	0.75	0.87	0.97	1.06	1.15	1.22	1.30	1.37	1.68	1.94	2.16	2.37	2.56	274	9.00	9.06	0 05	2.52	3.87	4.11	4.33
0.1	11	0.50	0.71	0.86	1.00	1.12	1.22	1.32	1.41	1.50	1.58	1.93	2.23	2.50	2.74	2.95	3	Interr	olate	with	18	4.47	4.74	4.99
8:1	12.5	0.61	0.86	1.05	1.22	1.36	1.49	1.61	1.72	1.82	1.92	2.35	2.72	3.04	3.33	3.59	9	incerp	Joiate	VVICII	)8	5.43	5.76	6.08
6:1	15 16.7	0.81	1.14 1.36	1.40 1.67	1.62 1.92	1.81 2.15	1.98 2.36	2.14 $2.54$	2.29 2.72	2.43 2.88	2.56 3.04	3.13 3.72	3.62 4.30	4.05 4.81	4.43 5.27	4.79 5.69	5. 6.				77	7.24 8.60	7.68	8.09
											0.04	0.12					0.00		0.00	****	J.J4	8.60	9.12	9.62
5:1 4%:1	20 22	1.29 1.51	1.82 2.13	2.23 2.61	$\frac{2.58}{3.02}$	2.88 3.37	3.16 3.69	3.41 3.99	$\frac{3.65}{4.27}$	3.87 4.53	4.08 4.77	5.00	5.77	6.45	7.06			8.65	9.12	9.99	10.79	11.54	12.24	12.90
~ 4:1	25	1.86	2.63	3.23	3.73	4.16	4.56	4.93	5.27	5.59	5.89	5.84 7.21	6.75 8.33	7.54				10.12 12.49		11.68 14.43	12.62 $15.58$	13.49 16.66	14.31 17.67	15.08 18.63
	30 '	2.51	3.56	4.36	5.03	5.62	6.16	6.65	7.11	7.54	7.95							16.87		19.48	21.04	22.49	23.86	25.15
3:1	33.3	2.98	4.22	5.17	5.96	6.67	7.30	7.89	8.43	8.95	9.43							20.00		23.10	24.95	26.67	28.29	29.82
	35	3.23	4.57	5.60	6.46	7.23	7.92	8.55	9.14	9.70	10.22	12.52	14.46	16.16	17.70	19.12	20.44	21.68	22.86	25.04	27.04	28.91	30.67	32.32
2%:1	40	4.00	5.66	6.93	8.00	8.95	9.80	10.59	11.32	12.00	12.65	15.50	17.89	20.01	21.91	23.67	25.30	26.84	28.29	30.99	33.48	35.79	37.96	40.01
0.1	45	4.81	6.80	8.33	9.61	10.75	11.77	12.72	13.60	14.42	15.20							32.24		37.23	40.22	42.99	45.60	48.07
2:1	50 55	5.64 6.48	7.97 <b>9.16</b>	9.76 $11.22$	11.27 $12.96$	12.60 $14.48$	13.81 15.87	14.91 17.14	15.94 18.32	16.91 19.43	17.82 20.48							37.81 43.45		43.66 50.18	47.16	50.41	53.47	56.36
1%:1	57	6.82										25.09	20.91	32.39	39.40	30.32	40.97	43.45	45.80	90.18	54.20	57.94	61.45	64.78
14.1	60	7.32	10.35	11.80 12.68	13.63 14.64	15.24 16.37	16.69 17.93	18.03	19.28	20.45	21.55							45.72		52.79	57.02	60.96	64.66	68.15
1%:1	66.7		11.93	14.61	16.88	18.87	20.67	19.37 $22.32$	20.71 $23.87$	$21.96 \\ 25.31$	23.15 26.68							49.11		56.71	61.25	65.48	69.45	73.21
	70		12.70		17.96	20.08	21.99		25.39	26.93	28.39							56.60 60.23		65.36 69.54	70.60 75.12	75.47	80.05	84.38
	75	9.78	13.83	16.94	19.56	21.87	23.95		27.66		30.92							65.60		75.75	81.82	80.30 87.46	85.17 92.77	89.78 97.79
1%:1	80		14.93		21.11	23.60	25.85	27.93	29.85	31.66	33.38	40.88	47.20	52.77	57.81	62.44	66.75	70.80	74.63	81.76	88.31	94.41	100.13	105.55
	85		15.98		22.61	25.27	27.69	29.90	31.97	33.91	35.74							75.82		87.55		101.09		
	90		17.00		24.04	26.88		31.80	34.00	36.06	38.01							80.63				107.51		
	95			22.01	25.41	28.41	31.12			38.12	40.18							85.23				113.64		
1:1	100	13.36	18.89	23.14	26.72	29.87	32.72	35.34	37.78	40.08	42.24											119.48		

*Calculated from

LS = 
$$\left(\frac{65.41 \times s^2}{s^2 + 10,000} + \frac{4.56 \times s}{\sqrt{s^2 + 10,000}} + 0.065\right) \left(\frac{l}{72.5}\right)^n$$

FROM "EROSION É SEAMENT COUTROL HANDBOOK", Goldman, Jackson, + Bursztynsky, 1986

LS = topographic factor

l = slope length, ft (m × 0.3048)

s = slope steepness,
m = exponent dependent upon slope steep
(0.2 for slopes < 1%, 0.3 for slopes 1:
0.4 for slopes 3.5 to 4.5%, and
0.5 for slopes > 5%)



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Client: WPL	Subject: Downslope Pipe and Inlet Sizing	Chk'd: RJG	Date: 8/28/23

To size the downslope pipe and inlet to accommodate the 25-year, 24-hour storm event.

1. HydroCAD Report: COL_Mod12_HydroCAD Report

### Approach:

Use the orifice equation to size the downslope pipe inlet. Size the inlet for the largest diversion berm flow rate and apply that inlet size to all downslope pipe inlets. Confirm the head (h) acting on the orifice will not overtop the diversion berm depth of 2.0 ft.

Use Manning's equation to size the downslope pipe based on the largest diversion berm flow rate. Confirm the pipe has capacity for the design flow under open channel flow conditions.

## **Assumptions:**

- 1. Orifice coefficient = 0.63
- 2. Assume the orifice head (h) acts on the centerline of the inlet pipe.
- 0.012 (For smooth walled HDPE pipe: http://www.engineeringtoolbox.com/mannings-roughness-d_799.html) 3. Manning's n =
- 4. Size flumes under the vegetated cover condition.

From the HydroCAD Report (Reference 1), the peak discharge to each downslope flume resulting from a 25-year, 24-hour storm is as follows*:

Flume 1	F	lume 2		Flume 3 Existing)	Flume 4 (Existing)	Flume 5 (Existi	ng)
Area 1	3.32	Area 3	1.06				
Area 2	1.71	Area 4	0.92				
Area 13	4.83	Area 15	1.93				
Area 14	1.94	Area 16	2.80	<u> </u>	<u> </u>		
Total =	11.80		6.71	0	0		0
Eluma 6	_	luma 7		Eluma Q	Eluma O	Eluma 10	

Flume 6		Flume 7		Flume 8		Flume 9		Flume 10	
Area 6	1.66	Area 8	6.16	Area 10	2.31	Area 11	2.59	Area 27	0.52
Area 7	2.81	Area 9	2.37	Area 21	1.35	Area 12	6.39	Area 28	0.44
Area 17	1.78	Area 19	3.62	Area 22	4.40	Area 23	3.84		
Area 18	3.04	Area 20	3.24			Area 24	5.53	_	
Total =	9.29	Total =	15.39	Total =	8.06	Total =	18.35	Total =	0.96

^{*} Please note that the total flow rate at each flume calculated above may not reflect the flow rate shown in the HydroCAD Model due to the inflow to the flume occuring at different times during the storm event. The calculation above reflects the peak flow rate.

### Results:

Based on the inlet sizing calculation, an 18" diameter inlet will convey the stormwater runoff from the largest flow rate to an inlet (Area 12).

Based on the Manning's calculation for flow within the pipe, the 12" diameter downslope pipe will accommodate the design flow for Flumes under open channel flow conditions. Although the flow for the downslope pipes can be handled by 12" dia. pipes, for ease of construction, all downslope pipes will be 18" dia with the exception of Flume 10.

Flume 10 will be constructed with a 12" dia pipe based on the drainage area and anticipated flow rate.

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# Client: WPL Calculations:

Job No. 25222260.00

### Size the downslope pipe inlet:

From the HydroCAD report (Reference #1), the maximum 25-year, 24-hour flow along a diversion berm is in HydroCAD model).

Project: Columbia Energy Center MOD 12 Subject: Downslope Pipe and Inlet Sizing

> 6.39 cfs Area 12 Inlet

Orifice Equation:  $Q = C * A * (2 * g * h)^{0.5}$ 

where: Q = flow rate (cfs) = 6.39 (From above)

C = orifice coefficient = 0.63 (See assumption #1)

A = orifice area (sf) = 1.77 (area of 18" diameter pipe) Actual Pipe Diameter = 18 inches

g = gravity (ft/sec²)= 32 h = orifice head acting on centerline (ft) h =  $(Q/(C*A))^2/(2*g) = 0.5$  ft

Given Assumption #2, depth of flow along diversion berm = h + D/2/12 = 1.26 ft = 15.1 inches

The diversion swale depth of 2 ft is sufficient to prevent overtopping at the downslope pipe inlet locations. The depth of the diversion berm increases at the entrance of the down slope pipes due to mounding of the soil

# Size the downslope flume pipe:

over the pipe.

Use Manning's equation to size the downslope pipe.

Manning's Equation:  $Q = (1.49/n) \times A \times R^{(2/3)} \times S^{(1/2)}$ 

where: Q = Flow Rate, cfs

n = Manning's Roughness Coefficient

A = Flow Area, sf

R = Hydraulic Radius, ft (= A/P)

S = Channel Slope, ft/ft

For flow rates < 20 cfs, assume a 12" diameter downslope flume:

Use 18.35 cfs to Flume 9 to check sizing (max flow to a flume that is  $\leq 20$  cfs)

Design Criteria

Pipe Diameter (in) = D = 12

Pipe Slope (ft/ft) = S = 0.25

Manning's Roughness Coefficient = n = 0.012

See Downslope Flume 7 pipe flow calculator on Sheet 3

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Calculations (Continued): Flume 9 18.35

# Manning Formula Uniform Pipe Flow at Given Slope and Depth

# Inputs:

Pipe Diameter, d₀	12	in
Manning Roughness,		
<u>n</u>	0.0120	
<u>Pressure slope</u>		
(possibly equal to		
pipe slope), So	0.2500	slope
Percent of (or ratio		
to) full depth (100%		
or 1 if flowing full)	0.7788	fraction

# **Results:**

Flow, Q	18.3513	ft^3/s
Velocity, v	27.9622	ft/s
Velocity head, hv	12.1517	ft
Flow Area, A	0.6563	ft^2
Wetted Perimeter, P	2.1623	ft
Hydraulic Radius	0.3035	ft
Top Width, T	0.8301	ft
Froude Number, F	5.63	
Shear Stress (tractive		
force), τ	12.1539	psf

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**HawsEDC Calculators** 

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Job No. 25222260.00 Project: Columbia Energy Center MOD 12 By: SJ
Client: WPL Subject: Downslope Pipe and Inlet Sizing Chk'd:

Calculations (Continued): Flume 9 18.35

# Manning Formula Uniform Pipe Flow at Given Slope and Depth

# Inputs:

Pipe Diameter, d _o	18	in
Manning Roughness,		
<u>n</u>	0.0120	
Pressure slope		
(possibly equal to		
pipe slope), So	0.2500	slope
Percent of (or ratio		
to) full depth (100%		
or 1 if flowing full)	0.3906	fraction

# **Results:**

Flow, Q	18.3546	ft^3/s
Velocity, v	28.7059	ft/s
Velocity head, hv	12.8066	ft
Flow Area, A	0.6394	ft^2
Wetted Perimeter, P	2.0253	ft
Hydraulic Radius	0.3157	ft
Top Width, T	1.4637	ft
Froude Number, F	7.77	
Shear Stress (tractive		
force), τ	9.1435	psf

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**HawsEDC Calculators** 

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Client: WPL	Subject: Energy Dissipator Sizing	Chk'd: RJG	Date: 8/28/23		

### Purpose:

To size an energy dissipator structure and riprap apron at the outlet of the downslope flume pipes.

### References:

- 1. "Hydraulic Design of Energy Dissipators for Culverts and Channels," HEC-14, Third Edition, July 2006, USDOT FHWA.
- 2. Downslope Pipe and Inlet Sizing calculation (for pipe size, flow rate, and pipe velocity).
- 3. HydroCAD Report: COL_Mod12_HydroCAD Report
- 4. Facilities Development Manual Chapter 13, Section 13-30 Rock Riprap Lined Chutes.
- 5. WisDOT FDM Table 25.1

## Approach:

Use the downslope pipe outlet velocity to size an energy dissipator structure (USBR Type VI Impact Basin) following the design approach outlined in Section 9.4 of Reference #1.

Use Rock Chute Data Spreadsheet, FDM 13-30-30 Attachment 30.1 (from Reference #5) to design the rock chute. For construction purposes use the maximum flow to size all dissipators and riprap apron.

### **Assumptions:**

- 1. Riprap specific gravity = 2.65
- 2. From the HydroCAD Report, the 25-year, 24-hour peak discharge to each downslope flume is as follows*:

Flume 1	I	lume 2		Flume 3 Existing)	Flume 4 (Existing)	Flume 5 (Existing)	
Area 1	3.32	Area 3	1.06				
Area 2	1.71	Area 4	0.92				
Area 13	4.83	Area 15	1.93				
Area 14	1.94	Area 16	2.80	<u> </u>	<u></u>		
Total =	11.80		6.71	0	0	0	

Flume 6		Flume 7		Flume 8		Flume 9		Flume 10
Area 6	1.66	Area 8	6.16	Area 10	2.31	Area 11	2.59	This flume discharges directly into a
Area 7	2.81	Area 9	2.37	Area 21	1.35	Area 12	6.39	concrete catch basin at the toe of slope, therefore, no energy
Area 17	1.78	Area 19	3.62	Area 22	4.40	Area 23	3.84	dissipator is needed.
Area 18	3.04	Area 20	3.24	_		Area 24	5.53	_
Total =	9.29	Total =	15.39	Total =	8.06	Total =	18.35	

^{*} Please note that the total flow rate at each flume calculated above may not reflect the flow rate shown in the HydroCAD Model due to the inflow to the flume occuring at different times during the storm event. The calculation above reflects the peak flow rate.

Using Figure 9.14 (See Sheet 4), enter the Froude Number and the Energy from Step 2 to determine the from the downslope flume pipe and inlet sizing calculation.

### Results:

The energy dissipator structures for the 18" dia. downslope flume pipes will consist of dissipator structures with widths ( $W_B$ ) of 6 feet, with the remaining dimensions from Table 9.2 on Sheets 5 and 6.

Riprap at the Flume energy dissipator outlets will consist of WisDOT Select Crush Material (D50= 2.2 inches) (See Page 3).

The riprap apron footprint will be based on the energy dissipator width, the outlet swale geometry, and as shown on the Plan Set.

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# Client: WPL Calculations:

### For 18" dia. downslope flume pipes

From Reference #2:

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Flow rate (Q) = 18.35 cfs Pipe velocity (V) = 28.7 ft/s Flow area (A) = Q/V = 0.64 sf

Design procedure from pg. 9-40 of Reference #1:

# Step 1: Compute the Equivalent Depth of Flow Entering Dissipator:

$$Y_e = (A/2)^{1/2}$$
 where:  $Y_e = Equivalent depth$   $A = Area (from above)$ 

 $Y_{e} = 0.57 \text{ ft}$ 

# Step 2: Compute the Froude Number and the energy at the end of the pipe:

 $Fr = V/[(g*Y_e)^{1/2}] \qquad \text{where:} \quad Fr = Froude \ Number} \\ V = Velocity \ (from \ above) \\ g = Gravity \ constant \ (32.2 \ ft/sec^2) \\ Y_e = Equivalent \ depth \ (from \ Step \ 1 \ above) \\ H_o = Y_e + V^2/2g \qquad \text{where:} \quad H_o = Energy \ at \ the \ end \ of \ the \ pipe \\ Y_e = Equivalent \ depth \ (from \ above) \\ V = Velocity \ (from \ above) \\ V = Velocity \ (from \ above) \\ Ho = 13.4 \ ft \qquad g = Gravity \ constant \ (32.2 \ ft/sec^2)$ 

### Step 3: Determine H_a/W_B and calculate the required width of the energy dissipator:

Using Figure 9.14 (See Sheet 4), enter the Froude Number and the Energy from Step 2 to determine the width of the energy dissipator.

From Figure 9.14,  $H_o/W_B =$  2.55  $W_B = H_o/(H_o/W_B) \qquad W_B = 5.2 \text{ ft.}$  Use  $W_B =$  6.0 ft.

Step 4: Obtain the remaining energy dissipator dimensions from Table 9.2 from Reference #1 (see Sheets 5 and 6)

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### Step 5: Size the riprap at the structure outlet

From Reference #5, use Rock Chute Design spreadsheet (see Sheet 3)

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### Calculations:

Step 5: Determine the exit velocity from the energy dissiaptor structure and size the riprap apron at the structure outlet. Use the relationship:

$$H_B = Q/(W_B \times V_B) + V_B^2 / 2g = H_o \times (1-H_L/H_o)$$

Where:

Q = 18.4 cfs, flowrate

 $W_B = 6.0$  ft, width of energy dissipator

 $g = 32.2 \text{ ft/s}^2$ , gravity

H_O 13.4 Energy at end of pipe

 $H_L/H_O$  75 %, Energy loss (From Figure 9.15 from Reference #1, see Sheet 3)

 $V_B = Velocity$  at exit of dissipator (ft/s)

HB = Energy at exit of dissipator (ft)

Calculate HB using the second part of the equation:

 $H_B = H_o \times (1-H_L/H_o)$ 

 $H_R =$ 3.41

Using trial and error, select values for  $V_B$  and use the first part of the equation to calculate  $H_B$ :

Try  $V_B =$ 

1.255 ft/s

3.41

Based on the energy dissipator structure exit velocity, calculate the riprap size at the dissipator outlet. From Equation 10.6 from Reference #1:

$$D_{50} = (0.692 / (S-1)) \times (V^2/2g)$$

Where:

S = 2.65 Specific gravity (See Assumption #1)

V = 1.26 Velocity =  $V_B$  from above.

 $D_{50}$  = riprap size

 $D_{50 \text{ Calc'd}} =$ 

0.010 feet

Round the calculated  $D_{50}$  up to the nearest IDOT standard riprap size:

D_{50 Design} =

0.18

Use = Select Crushed Material

with geotextile

Type R

Riprap Type	D ₅₀ (inches)	D ₅₀ (feet)	Riprap Thickness (in)	Geotextile Type
Select Crushed Material	2.2	0.18	5	Type R
Light Riprap	10	0.83	12	Type R
Medium Riprap	12.5	1.04	18	Type HR
Heavy Riprap	16	1.33	24	Type HR
Extra-Heavy Riprap	20	1.67	30	Type HR

from Reference 5

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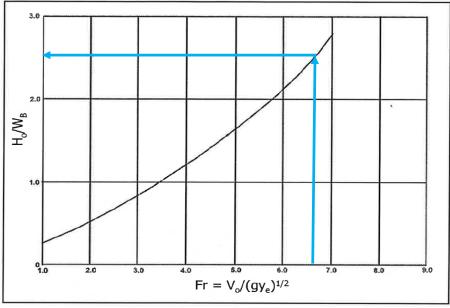


Figure 9.14. Design Curve for USBR Type VI Impact Basin

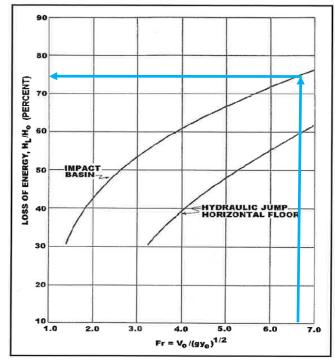


Figure 9.15. Energy Loss of USBR Type VI Impact Basin versus Hydraulic Jump

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# Table 9.2 (CU). USBR Type VI Impact Basin Dimensions (ft) (AASHTO, 2005)

W ₅	fh)	h ₂	h ₂	h ₄	L	L ₁	L ₂
4.	3.08	1.50	0.67	1.67	5.42	2.33	3.08
5.	3.83	1.92	0.83	2.08	6.67	2.92	3.83
6.	4.58	2.25	1.00	2.50	8.00	3,42	4.58
7.	5.42	2.58	1.17	2.92	9.42	4.00	5.42
8.	6.17	3.00	1.33	3.33	10.67	4.58	6.17
9.	6.92	3.42	1.50	3.75	12.00	5.17	5.92
10.	7.58	3.75	1.67	4.17	13.42	5.75	7.67
11.	8.42	4.17	1.83	4.58	14.58	6.33	8.42
12.	9.17	4.50	2.00	5.00	16.00	6.83	9.17
13.	10.17	4.92	2.17	5.42	17.33	7.42	10.00
14.	10.75	5.25	2.33	5.83	18.67	8.00	10.75
15.	11.50	5.58	2.50	6.25	20.00	8.50	11.50
16.	12.25	6.00	2.67	6.67	21.33	9.08	12.25
17.	13.00	6.33	2.83	7.08	21.50	9.67	13.00
18.	13.75	6.67	3.00	7.50	23.92	10.25	13.75
19.	14.58	7.08	3.17	7.92	25.33	10.83	14.58
20.	15.33	7.50	3.33	8.33	26.58	11.42	15.33

Ws	W ₁	W ₂	t,	t ₂	t ₃	t.	t _s
4.	0.33	1.08	0.50	0.50	0.50	0.50	0.25
5.	0.42	1.42	0.50	0.50	0.50	0.50	0.25
6.	0.50	1.67	0.50	0.50	0.50	0.50	0.25
7.	0.50	1.92	0.50	0.50	0.50	0.50	0.25
8.	0.58	2.17	0.50	0.58	0.58	0.50	0.25
9.	0.67	2.50	0.58	0.58	0.67	0.58	0.25
10.	0.75	2.75	0.67	0.67	0.75	0.67	0.25
11.	0.83	3.00	0.67	0.75	0.75	0.67	0.33
12.	0.92	3.00	0.67	0.83	0.83	0.75	0.33
13.	1.00	3.00	0.67	0.92	0,83	0.83	D.33
14.	1.08	3.00	0.67	1.00	0.92	0.92	0.42
15.	1.17	3.00	0.67	1.00	1.00	1.00	0.42
16.	1.25	3.00	0.75	1.00	1.00	1.00	0.50
17.	1.33	3.00	0.75	1.08	1.00	1.00	0.50
18.	1.33	3.00	0.75	1.08	1.08	1.08	0.58
19.	1.42	3.00	0.83	1.17	1.08	1.08	0.58
20.	1.50	3.00	0.83	1.17	1.17	1.17	D.67

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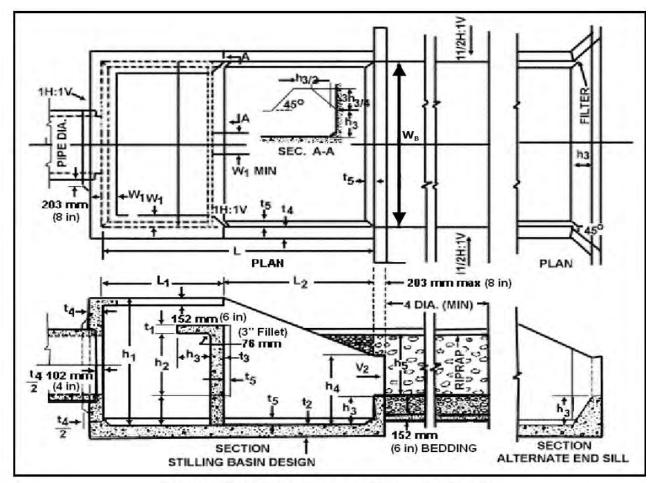


Figure 9.13. USBR Type VI Impact Basin

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Calculations (Continued):

# Downslope Flume 9 - Velocity Calculator (Q = 18.35 cfs)

# Manning Formula Uniform Pipe Flow at Given Slope and Depth

# Inputs:

Pipe Diameter, d₀	18	in
Manning Roughness, n		
	0.0120	
Pressure slope (possibly equal to pipe slope), S₀	0.2500	slope
Percent of (or ratio to) full depth (100% or 1 if flowing full)		
	0.3906	fraction

# **Results:**

Flow, Q	18.3546	ft^3/s
Velocity, v	28.7059	ft/s
Velocity head, hv	153.6794	in
Flow Area, A	0.6394	ft^2
Wetted Perimeter, P	2.0253	ft
Hydraulic Radius	0.3157	ft
Top Width, T	1.4637	ft
Froude Number, F	7.77	
Shear Stress		
(tractive force), τ	9.1435	psf

Version 2.0 (20 June 2017)

**HawsEDC Calculators** 



SCS ENGI	Sheet No: 1 of 5		
2 2 2 2 2 2 2		Calc. No.	
		Rev. No.	3
Job No. 25222260.00	Job: Columbia Energy Center MOD12	By: SJL	Date: 8/28/23
Client: WPL	Subject: Rock Chute Sizing & Riprap Size	Chk'd: RJG	Date: 8/28/23

### Purpose:

To size the rock chutes to accommodate the 25-year, 24-hour storm event.

### References:

- 1. Rock Chute Design Data spreadsheet Version WI-April-2005, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998.
- 2. HydroCAD Report: COL_Mod12_HydroCAD Report
- 3. Figure 1 Final Grades (Module 12)
- 4. Stable 25.1 Typical Particle Sizes of Native Sands at 75 Percent Passing (D75) from WisDOT Facilities. Development Manual (FDM).

### Approach:

- 1. Enter Inlet Channel data based on culvert apron or swale geometry Reference #2 and #3.
- 2. Enter Chute data based on slope from Reference #3, start the width, Bw equal to inlet channel Bw.
- 3. Enter Outlet Channel data based on Reference #3, start the width, Bw equal to inlet channel Bw.
- 4. Enter drainage area, apron elevations, flow (Q), and rainfall.
- 5. Adjust Bw for Chute and Outlet Channel until spreadsheet shows the rock chute "will" function adequately.
- 6. Determine rip rap classification based on D50 weight per Reference #4.

### Assumptions:

- 1. Assume side slopes of chute and outlet channel are 2:1.
- 2. Assume Factor of Safey is 1.2.
- 3. n-value is based on proposed conditions at the channel.
- 4. Assume Outlet apron depth, d is 1.0 ft.
- 5. Freeboard is 1.0 ft.
- 6. Use 25-year, 24-hour storm event flow (Reference #2) for  $Q_{high}$  and  $Q_{low}$ .
- 7. Classification of riprap is based on weight (Reference #4).

# Calculations:

See attached spreadsheet calcs for each rock chute.

### Results:

The rock chutes are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

Rock	Width	Thickness	Apron	Apron	D ₅₀	
Chute	(ft)	(in)	Width (ft)	Length (ft)	(in)	WisDOT Rip Rap Classification
RC1	8	4	8	2	2	Select Crushed Material, Type R
RC2	6	12	6	7	5.9	Light Riprap Type R
RC3	6	8	6	5	3.8	Light Riprap Type R
RC4	6	9	6	6	3.8	Light Riprap, Type R

(Version WI-April-2005, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998)

Revised for WisDOT 9/2010 Project: COL - Mod 12 RC1 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 0.0 Bottom Width = 8.0 Bottom Width = 8.0 Side slopes = 2.0 (m:1) Side slopes = 4.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.030 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0600 ft./ft. Bed slope = **0.1769** ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Outlet 806.0 ft. --- (H_{drop} = Note: The total required capacity is routed Apron elev. --- Inlet = 829.0 ft. --Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw):  $Q_{high} = 2.3$ cfs High flow storm through chute ➤ Tw (ft.) = Program  $Q_{low} = 2.3$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 3.03 \text{ ft.} (3.03 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 3.06 \text{ ft.}$  $h_{cv} = 0.07 \text{ ft.} (0.07 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.2 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.1 \text{ ft.}$  $H_{\rm p} = 0.03 \, \text{ft}$  $d_1 = 0.09 \text{ ft.}$ Inlet  $(0.03 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height, d₂ = 0.19 ft. (0.19 ft.) Channe (0.09 ft.)Inlet Apron  1   $y_{n} = 0.38$  ft.  $10y_{c} =$ Tw+d = 1.28 ft. - Tw o.k.(0.38 ft.) (1.28 ft.) - Tw o.k. 40*Design D₅₀ = 6 ft Velocity_{inlet} = 3.93 fps radius 0.28 ft. (0.28 ft.) Outlet Channel at normal depth Critical Slope check upstream is unstable Slope = 0.005 ft./ft Geotextile ¹ **Note**: When the normal depth  $(y_n)$  in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 0.95 fpsVelocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness =  $d_2 =$ 0.19 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 8.2 ft

(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)

Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12 RC2 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 6.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 1.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.012 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0001 ft./ft. Bed slope = 0.2319 ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Apron elev. --- Inlet = 820.0 ft. --Outlet 804.0 ft. --- (H_{drop} = Note: The total required capacity is routed Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw): Q_{high}= 11.7 cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 11.7$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.12 \text{ ft.} (0.12 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.75 \text{ ft.}$  $h_{cv} = 0.21 \text{ ft.} (0.21 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.67 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.33 \text{ ft.}$  $H_{\rm p} = 0.63 \, \text{ft}$  $d_1 = 0.28 \text{ ft.}$ Inlet  $(0.63 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height,  $d_2 = 0.71$  ft. (0.71 ft.) Channe (0.28 ft.) Inlet Apron  1   $y_{n} = 1.3$  ft.  $10y_c = 5$ Tw+d = 1.85 ft. - Tw o.k.(1.3 ft.) (1.85 ft.) - Tw o.k. 40*Design D₅₀ = 16 ft Velocity_{inlet} = 1.23 fps radius 0.85 ft. (0.85 ft.) Outlet Channel at normal depth Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.79 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness = 11.6 in.  $d_2 =$ 0.71 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 6.8 ft

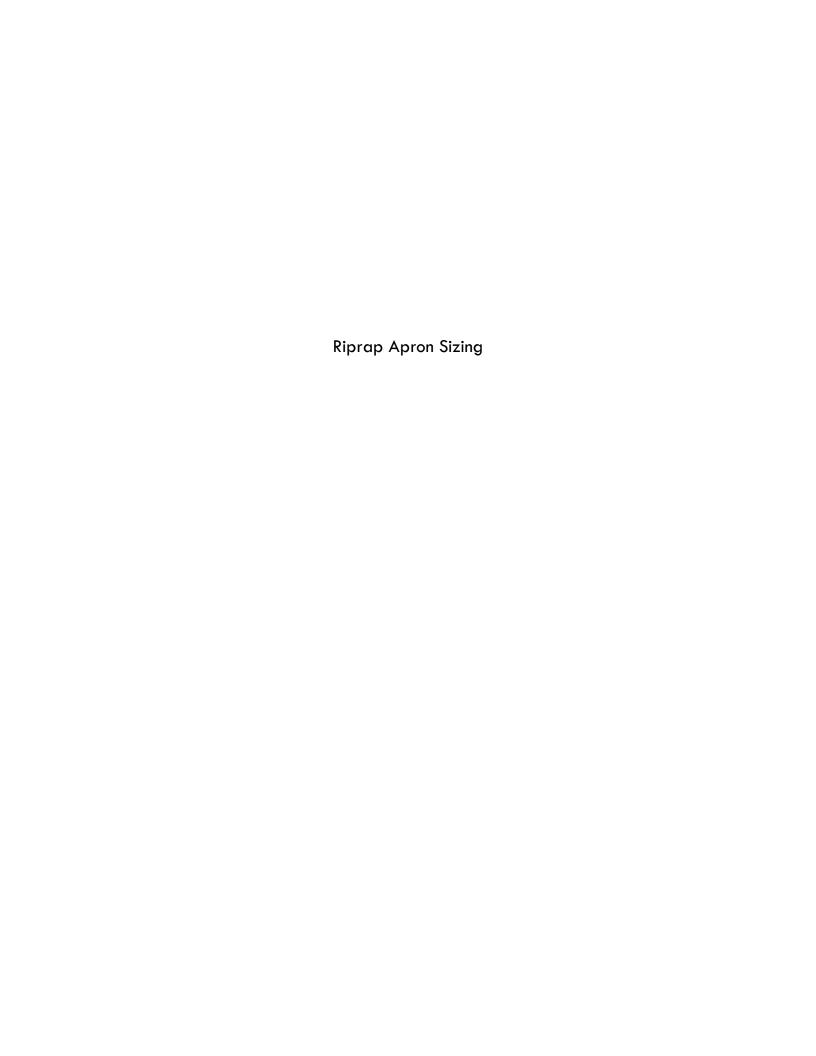
(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)
Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12 RC3 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 6.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 1.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.012 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0001 ft./ft. Bed slope = **0.1545** ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Apron elev. --- Inlet = 821.0 ft. --Outlet 804.0 ft. --- (H_{drop} = Note: The total required capacity is routed Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw): Q_{high}= **6.6** cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 6.6$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.09 \text{ ft.} (0.09 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.52 \text{ ft.}$  $h_{cv} = 0.15 \text{ ft.} (0.15 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.47 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.23 \text{ ft}$  $H_{\rm p} = 0.43 \, \text{ft}$  $d_1 = 0.21 \text{ ft.}$ Inlet  $(0.43 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height, d₂ = 0.46 ft. (0.46 ft.) Channe (0.21 ft.)Inlet Apron  1   $y_{n} = 0.93$  ft. Tw+d = 1.61 ft. - Tw o.k.(0.93 ft.) (1.61 ft.) - Tw o.k. 40*Design D₅₀ = 11 ft Velocity_{inlet} = 1.02 fps radius 0.61 ft. (0.61 ft.) Outlet Channel at normal depth Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.49 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge .04 cfs/ft Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness = 7.7 in.  $d_2 =$ 0.46 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 6.8 ft

(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)

Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12 RC4 County: Columbia Designer: RJG Checked by: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 4.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 4.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.045 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0288 ft./ft. Bed slope = 0.2212 ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Outlet 817.6 ft. --- (H_{drop} = Note: The total required capacity is routed Apron elev. --- Inlet = 824.9 ft. -Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw):  $Q_{high} = 7.0$ cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 7.0$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.13 \text{ ft.} (0.13 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.56 \text{ ft.}$  $h_{cv} = 0.15 \text{ ft.} (0.15 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.49 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.24 \text{ ft.}$  $H_{\rm p} = 0.43 \, \text{ft}$  $d_1 = 0.21 \text{ ft.}$ Inlet  $(0.43 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height,  $d_2 = 0.51$  ft. (0.51 ft.) Channe (0.21 ft.)Inlet Apron  1   $y_{n} = 0.45$  ft. Tw+d = 1.63 ft. - Tw o.k.(0.45 ft.) (1.63 ft.) - Tw o.k. 40*Design D₅₀ = 12 ft Velocity_{inlet} = 2.72 fps radius 0.63 ft. (0.63 ft.) Outlet Channel at normal depth Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.52 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness 8.8 in Tw + d =Tailwater above outlet apron Rock thickness = 8.8 in.  $d_2 =$ 0.51 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d₂. B' = 6.8 ft



363	SCS ENGINEERS			
		Calc. No.		
		Rev. No.		
Job No. 25222260.00	Job: Columbia Energy Center MOD 12	By: SJL Date: 8/28/23		
Client: WPL	Subject: Riprap Sizing at Culvert Outlet	Chk'd: RJG Date: 8/28/23		

### Purpose:

To size the riprap apron dimensions at proposed culvert locations C2, C3, C4, C5, C6, C8, C9, and C11 based on a 25-year, 24 hour storm event:

### References

- 1. "Energy Dissipators," Wisconsin Department of Transportation (WisDOT), Facilities Development Manual (FDM) 13-35-5.
- 2. HydroCAD Report: COL_Mod12_HydroCAD Report
- 3. "Rock Riprap Lined Channels," WisDOT FDM 13-30-25.
- 4. Culvert Sizing Calculation.
- 5. WisDOT FDM Chapter 13, Section 30 Rock Riprap Lined Chutes

### Approach:

Use the equations in Section 5.2 - Riprap Blanket of WisDOT FDM 13-35-5 (Energy Dissipators) to determine the average size of stone  $(d_{50})$  and riprap apron length. Round up the calculated  $d_{50}$  to the nearest WisDOT standard riprap size.

Use WisDOT FDM 13-35 Attachment 5.2 to determine the width of the riprap apron for discharges to a flat area. For discharges to channels, extend riprap across the channel bottom and up the sides.

### Assumptions:

Assume riprap apron thickness (T) is 2  *  d  $_{50}$  to protect against washout and undercutting of the riprap.

Assume tailwater depth,  $TW = 0.40 * D_0$ 

Assume max TW conditions for the riprap apron width.

Assume that when there are multiple culverts, the total discharge to the culverts is distributed evenly through each barrel.

### Calculation:

From WisDOT Section 5.2 - Riprap Blanket:

$$d_{50}/D_0 = 0.020 (D_o/TW) (Q/D_o^{5/2})^{4/3}$$

$$L_{sp}/D_o = 1.7 (Q/D_o^{5/2}) + 8$$

Or

$$d_{50} = 0.02 \times (D_o/TW) \times (Q/D_o^{5/2})^{4/3} \times D_o$$

$$L_{sp} = (1.7 (Q/D_o^{5/2}) + 8) \times D_o$$

where:  $D_o = Diameter or width of culvert (ft)$ 

 $Q = Flow \ rate \ (cfs) \ (discharge \ rate \ through \ culvert, \ from \ Worst \ Case \ Condition \ HydroCAD \ Model \ (Reference \ \#2))$ 

TW = Tail water depth (ft)

 $d_{50}$  = Average size of stone (ft)

 $L_{sp} = Length$  of stone protection (Apron Length) (ft)

Location	Total Flow (Q, cfs)	Number of Pipes	D _o (ft)	Q (cfs)	TW (ft)	d _{50 calculated}	d _{50 Design}	$L_{sp}$
Culvert C2a	10.41	2	1.5	5.2	0.60	0.18	0.18	17
Culvert C2b	3.50	2	1	1.8	0.40	0.11	0.18	11
Culvert C3	27.48	2	2.5	13.7	1.00	0.19	0.83	26
Culvert C4	32.85	2	2.5	16.4	1.00	0.25	0.83	27
Culvert C5	38.11	2	2.5	19.1	1.00	0.30	0.83	28
Culvert C6	30.03	2	2	15.0	0.80	0.37	0.83	25
Culvert C8	0.96	1	1	1.0	0.40	0.05	0.18	10
Culvert C9	0.22	1	1	0.2	0.40	0.01	0.18	8
Culvert C11	26.46	2	2.5	13.2	1.00	0.18	0.83	26

### Results

Below is a summary of the d₅₀, thickness (T), and configuration of the riprap apron. Also refer to WisDOT FDM Attachment 5.2 (Sheet 2) for details on apron layout. Use WisDOT Light Riprap at culvert discharge.

Location	d ₅₀ (in)	T (in)	L _{sp} (ft)	W _{sp} (ft)	WisDOT Riprap sizes
Culvert C2a	2.2	6	17	See Note 1	Select Crushed Material
Culvert C2b	2.2	6	11	See Note 1	Select Crushed Material
Culvert C3	10.0	20	26	See Note 1	Light Riprap
Culvert C4	10.0	20	27	See Note 1	Light Riprap
Culvert C5	10.0	20	28	See Note 1	Light Riprap
Culvert C6	10.0	20	25	See Note 1	Light Riprap
Culvert C8	2.2	6	10	See Note 1	Select Crushed Material
Culvert C9	2.2	6	8	See Note 1	Select Crushed Material
Culvert C11	10.0	20	26	See Note 1	Light Riprap

 $^{1. \} For \ discharges \ to \ channels, \ place \ riprap \ along \ channel \ bottom \ and \ up \ side \ of \ channel.$ 

E.6.2	Modules 12 and 13 Stormwater Calculations

SHEET NO.	1 of 4				
CALC. NO.					
REV. NO.		3			
ВҮ	SJL	DATE	8/28/23		
CHK'D	MPH	DATE	8/28/23		

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

# **Storm Water Management Calculations**

# Purpose:

The purpose of the storm water runoff calculations is to demonstrate that the existing storm water sedimentation basin and proposed storm water management features included in the Module 12 and 13 Plan Modification Request can accommodate and safely convey the runoff from a 25-year, 24-hour storm event and 100-year, 24-hour storm event during post closure conditions.

Items addressed in these calculations:

- Swales
- Culverts
- Diversion Berms
- Downslope Flumes & Energy Dissipators
- Rock Chutes
- Discharge Aprons
- Sedimentation Basin
- North Infiltration Area

The proposed storm water management conditions are shown on **Figure 1**. The calculations support the capacity check of the following existing storm water management feature:

Feature **Purpose Design Method** Convey storm water runoff from HydroCAD runoff modeling and Swales adjacent areas to culverts and offsite Swale Calculation during post construction conditions Convey storm water from the final HydroCAD runoff modeling and Culverts cover perimeter swales during post **HY-8 Culvert Model** construction conditions Diversion Berms Reduce storm water runoff from final HydroCAD runoff modeling and Diversion Berm Calculations cover slopes and to divert water to perimeter swales during post construction conditions Downslope Flumes & Convey storm water from diversion HydroCAD runoff modeling and **Energy Dissipators** berms down slope to swales and offsite Downslope Flume Calculations drainage features during post construction conditions **Rock Chutes** Erosion protection and convey storm HydroCAD runoff modeling and water from energy dissipators to Rock Chute Calculation existing swale during post construction conditions Erosion protection from culvert HydroCAD runoff modeling and Discharge Aprons discharge at culvert outlets Riprap Apron Calculation To safely handle 25-year, 24-hour HydroCAD runoff modeling Sedimentation Basin storm event without overtopping the 100-year, 24-hour spillway. To safely handle 25-year, 24-hour and North Infiltration Area HydroCAD runoff modeling 100-year, 24-hour storm events without overtopping or backing up the inlet pipe.

SHEET NO.	2 of 4				
CALC. NO.					
REV. NO.		3			
BY	SJL	DATE	8/28/23		
CHK'D.	MRH	DATE	8/28/23		

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

# Approach:

# Hydrograph Generation

HydroCAD was used to model the storm water management system and develop the hydrographs using TR-20 methodologies. The model is designed to simulate the surface runoff response of a watershed to a precipitation event. Input parameters for the model include precipitation depth for the design storm events from NOAA ATLAS 14, contributing drainage areas, runoff curve numbers, and time of concentration.

# Swale Sizing

The proposed swales were sized for the 25-year, 24-hour storm event. A WisDOT HEC-15 spreadsheet based on Manning's equation was used to calculate the depth of flow and velocity in the swales using the swale geometry and peak flow in the swales (as determined by the Hydrograph Generation models).

# Culvert Sizing

Culverts were sized for the 25-year, 24-hour storm event using the HY-8 computer model developed by the US Department of Transportation, Federal Highway Administration.

# **Diversion Berms**

Diversion berms were sized for the 25-year, 24-hour storm event. A WisDOT HEC-15 spreadsheet based on Manning's Equation was used to calculate the depth of flow and velocity in the swale using the swale geometry and peak flow for the storm event (as determined by the Hydrograph Generation Calculations).

# Downslope Flumes and Energy Dissipators Sizing

Flumes and energy dissipators were sized for the 25-year, 24-hour storm event. Manning's equation and the orifice equation were used to size the flumes. Energy dissipators were sized using tables from the reference book "Hydraulic Design of Energy Dissipators for Culverts and Channels" US Department of Transportation, Federal Highway Administration, July 2006.

# Rock Chute Sizing

Rock chutes were sized for the 25-year, 24-hour storm event. Rock Chutes were sized based on the flow to each culvert location. The Iowa NRCS Rock Chute Design spreadsheet was used to size the chute and riprap.

# Discharge Apron Sizing

Riprap aprons were sized for the 25-year, 24-hour storm event using equations in Section 5.2 – Riprap Blanket of WisDOT FDM 13-35-5. The riprap aprons were sized based on the flow to the culvert location. The riprap stone sizing was used to specify the thickness and geometry of the riprap discharge apron.

# Sedimentation Basin Sizing

Route the proposed construction and existing drainage runoff through the sedimentation basin to confirm the basin can handle the 25-year, 24-hour storm event and to safely pass the 100-year, 24-hour storm event. HydroCAD was used to model the runoff flow through the basin outfall (as determined by the Hydrograph Generation model).

SHEET NO.		3 of	4
CALC. NO.			
REV. NO.		3	
BY	SJL	DATE	8/28/23
CHK'D.	MRH	DATE	8/28/23

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

# North Infiltration Area Verification

The depressional area located north of the Module 12 Plan Modification construction area acts as a infiltration area and accepts portions of the drainage runoff. Route the proposed construction and existing drainage runoff flowing to the North Infiltration Area to confirm the area can handle the 25-year, 24-hour and 100-year, 24-hour storm events without overtopping or backing up the inlet pipe. HydroCAD was used to model the runoff flow into this area (as determined by the Hydrograph Generation model).

# **Key Assumptions:**

- Drainage areas and time of concentration flow paths are as shown on Figure 1 for Post Construction Conditions.
- An MSE4 rainfall distribution was used based on NRCS Wisconsin rainfall distribution regions.

The precipitation depth for the 25-year, 24-hour storm was assumed to be <u>4.91 inches</u>, based on NOAA ATLAS 14 Point Precipitation Frequency Estimates (NOAA's National Weather Service Hydrometeorological Design Studies Center Precipitation Frequency Data Server).

The precipitation depth for the 100-year, 24-hour storm was assumed to be <u>6.59 inches</u>, based on NOAA ATLAS 14 Point Precipitation Frequency Estimates.

 Runoff curve numbers were based on tables presented in Urban Hydrology for Small Watersheds, and were assumed as follows and as listed in the modeling.

Cover Type	CN
Final Cover	69 – Pasture/grassland/range in good condition,
	hydrologic soil group (HSG) (B/C assumed mid value
	between each soil group)
Pasture, grassland or range	39 – Pasture/grassland/range, Good, HSG A
Gravel	96 – Gravel, HSG A
Water Surface	98 – Water Surface, HSG A

- Type A soil group for non-disturbed areas outside the landfill as soils are loamy sand.
- Other assumptions are included with the calculations attached to this appendix.

# Results:

# Hydrograph Generation

The hydrograph modeling results for the 25-year and 100-year, 24-hour storm events are included in the Post Construction Conditions Hydrograph Generation section.

## Swale Sizing

The proposed swales will be constructed as shown on the Drawings. The swales have the capacity to safely convey the both the 25-year, 24-hour storm events and maintain a minimum 0.5 foot of freeboard. Refer to the Swale Sizing section.

Appropriate erosion control product was selected based on the velocities and shear stress in the swales. Refer to the Swale Sizing section below for the evaluation.

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CALC. NO.			
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CHK'D.	MRH	DATE	8/28/23

Job No.	25222260.00	Job	Columbia Dry Ash Disposal
Client	WPL	Subject	Storm Water Management

# Culvert Sizing

Culverts will be as shown in the Drawings. The culverts have the capacity to safely convey the 25-year, 24-hour storm event. Refer to the Culvert Sizing Section for the detailed calculations.

# **Diversion Berm Sizing**

The proposed final berms will be constructed as shown on the Drawings. The diversion berms will contain the runoff from the 25-year, 24-hour storm event. Refer to the Diversion Berm Design section.

# Downslope Flume and Energy Dissipator Sizing

The downslope flumes and energy dissipaters will be constructed as shown on the Drawings. The downslope flumes are designed to contain the runoff from the 25-year, 24-hour storm event. Energy dissipators at the bottom of the downslope flumes have been designed to handle the peak velocities. Refer to the Downslope Flume and Energy Dissipator Sizing section below for detailed calculations.

# Rock Chute Sizing

The proposed rock chutes will be constructed as shown in the Drawings. The rock chutes will accommodate the runoff from the 25-year, 24-hour storm event. Refer to the Rock Chute Sizing section.

# Discharge Apron Sizing

The proposed riprap aprons will be constructed as shown in the Drawings. The aprons will accommodate the runoff from the 25-year, 24-hour storm event. Refer to Discharge Apron Sizing for design calculations.

# Sedimentation Basin Sizing

The existing sedimentation basin has the capacity to safely contain the 25-year, 24-hour storm event and safely pass the 100-year, 24-hour storm event through the emergency spillway.

As shown in the HydroCAD model, the water elevation in both basin areas for each storm event is provided below:

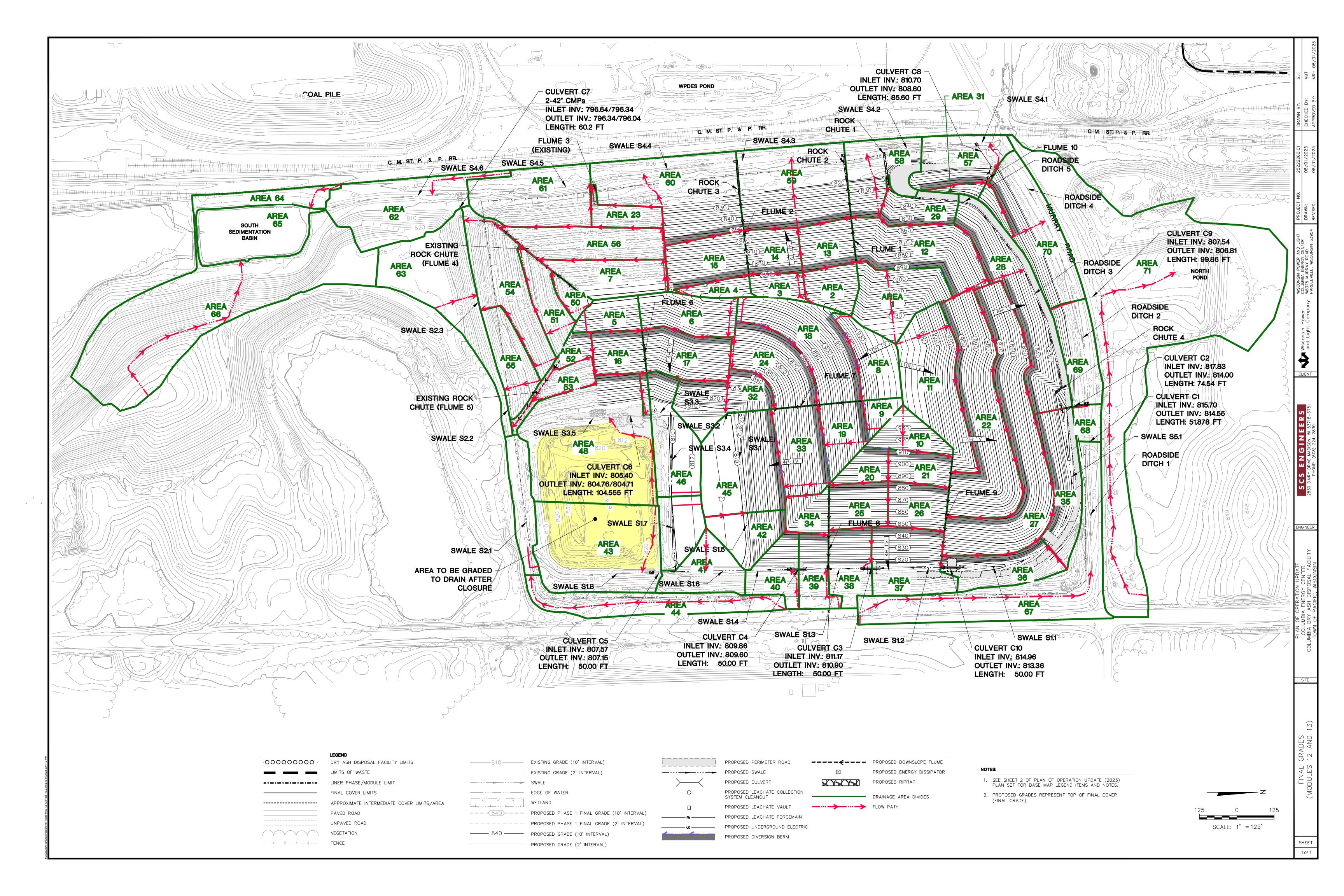
Basin Area	Basin	Basin	Peak	Peak
	Crest	Spillway	Elevation 25-	Elevation 100-
	Elevation	Elevation	year storm	year storm
	(ft MSL)	(ft MSL)	(ft MSL)	(ft MSL)
Existing Sedimentation Basin	794.00	793.00	792.00	793.20

# North Infiltration Area Verification

The North Infiltration Area can safely contain the 25-year, 24-hour storm event and the 100-year, 24-hour storm event without overtopping or backing up the inlet pipe at Murray Road.

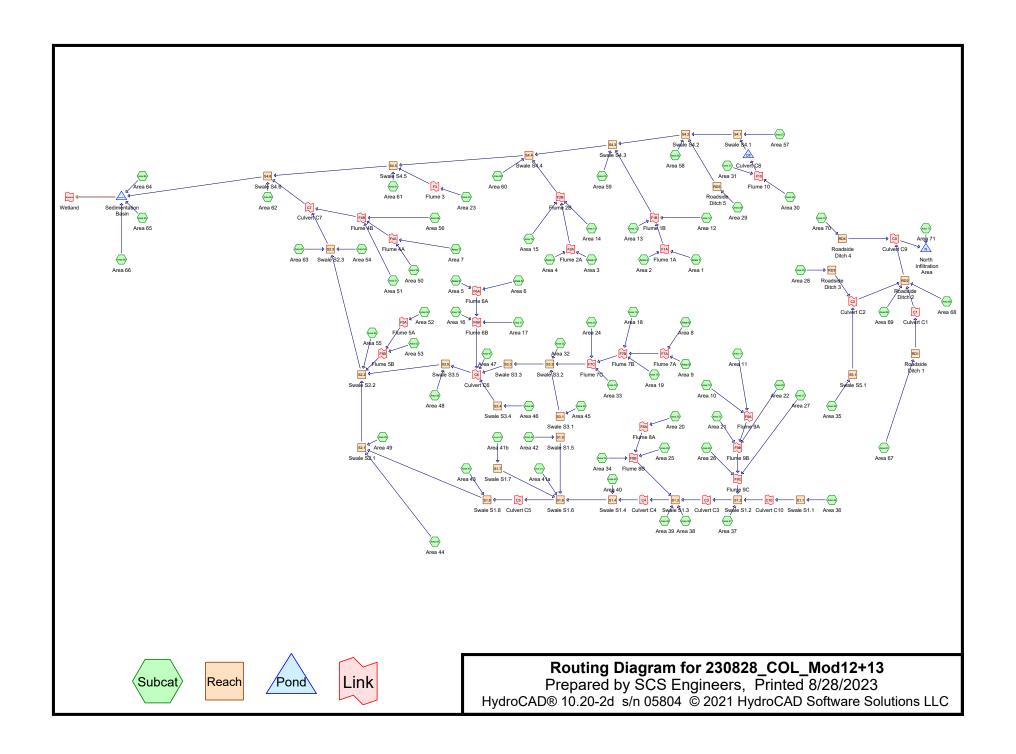
As shown in the HydroCAD model, the water elevation in both basin areas for each storm event is provided below:

Basin Area	Basin Crest Elevation (ft MSL)	Basin Inlet Pipe Elevation (ft MSL)	Peak Elevation 25- year storm (ft MSL)	Peak Elevation 100- year storm (ft MSL)
North Basin	810.00	806.81	803.86	805.23



# Post Construction Conditions Hydrograph Generation

- 25-year, 24-hour Storm Event
- 100-year, 24-hour Storm Event



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# **Rainfall Events Listing**

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	25-yr, 24-hr	MSE 24-hr	4	Default	24.00	1	4.91	2
2	100-yr, 24-hr	MSE 24-hr	4	Default	24.00	1	6.59	2

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentArea 1: Area 1	Runoff Area=1.296 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=364' Tc=6.7 min CN=69 Runoff=3.75 cfs 0.204 af
SubcatchmentArea 10: Area 10	Runoff Area=0.573 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=321' Tc=6.4 min CN=69 Runoff=1.68 cfs 0.090 af
SubcatchmentArea 11: Area 11	Runoff Area=1.872 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=796' Tc=7.6 min CN=69 Runoff=5.22 cfs 0.295 af
SubcatchmentArea 12: Area 12	Runoff Area=1.610 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=712' Tc=5.3 min CN=69 Runoff=4.83 cfs 0.254 af
SubcatchmentArea 13: Area 13	Runoff Area=0.626 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=333' Tc=4.5 min CN=69 Runoff=1.94 cfs 0.099 af
SubcatchmentArea 14: Area 14	Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=331' Tc=4.4 min CN=69 Runoff=1.93 cfs 0.098 af
SubcatchmentArea 15: Area 15	Runoff Area=0.943 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=464' Tc=5.9 min CN=69 Runoff=2.80 cfs 0.149 af
SubcatchmentArea 16: Area 16	Runoff Area=0.571 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=315' Tc=4.4 min CN=69 Runoff=1.78 cfs 0.090 af
SubcatchmentArea 17: Area 17	Runoff Area=0.990 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=424' Tc=4.7 min CN=69 Runoff=3.04 cfs 0.156 af
SubcatchmentArea 18: Area 18	Runoff Area=1.656 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=508' Tc=4.8 min CN=69 Runoff=5.06 cfs 0.261 af
SubcatchmentArea 19: Area 19	Runoff Area=0.689 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=286' Tc=4.3 min CN=69 Runoff=2.16 cfs 0.109 af
SubcatchmentArea 2: Area 2	Runoff Area=0.557 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=303' Tc=4.7 min CN=69 Runoff=1.71 cfs 0.088 af
SubcatchmentArea 20: Area 20	Runoff Area=0.381 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=306' Tc=4.3 min CN=69 Runoff=1.19 cfs 0.060 af
SubcatchmentArea 21: Area 21	Runoff Area=0.516 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=312' Tc=4.3 min CN=69 Runoff=1.62 cfs 0.081 af
SubcatchmentArea 22: Area 22	Runoff Area=2.579 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=1,146' Tc=6.4 min CN=69 Runoff=7.54 cfs 0.407 af
SubcatchmentArea 23: Area 23	Runoff Area=0.427 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=362' Tc=3.8 min CN=69 Runoff=1.38 cfs 0.067 af

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SubcatchmentArea 24: Area 24 Runoff Area=1.177 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=385' Tc=4.7 min CN=69 Runoff=3.61 cfs 0.186 af Runoff Area=0.682 ac 0.00% Impervious Runoff Depth=1.89" SubcatchmentArea 25: Area 25 Flow Length=349' Tc=4.5 min CN=69 Runoff=2.12 cfs 0.108 af Subcatchment Area 26: Area 26 Runoff Area=0.677 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=341' Tc=4.5 min CN=69 Runoff=2.10 cfs 0.107 af SubcatchmentArea 27: Area 27 Runoff Area=2.594 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=1,357' Tc=6.9 min CN=69 Runoff=7.46 cfs 0.409 af Runoff Area=2.159 ac 0.00% Impervious Runoff Depth=2.29" SubcatchmentArea 28: Area 28 Flow Length=125' Slope=0.2500 '/' Tc=3.9 min CN=74 Runoff=8.44 cfs 0.412 af SubcatchmentArea 29: Area 29 Runoff Area=0.616 ac 0.00% Impervious Runoff Depth=2.13" Flow Length=109' Slope=0.2500 '/' Tc=3.8 min CN=72 Runoff=2.25 cfs 0.109 af Runoff Area=0.348 ac 0.00% Impervious Runoff Depth=1.89" Subcatchment Area 3: Area 3 Flow Length=298' Tc=4.8 min CN=69 Runoff=1.06 cfs 0.055 af Runoff Area=0.149 ac 0.00% Impervious Runoff Depth=1.89" SubcatchmentArea 30: Area 30 Flow Length=211' Tc=1.2 min CN=69 Runoff=0.52 cfs 0.023 af Runoff Area=0.126 ac 0.00% Impervious Runoff Depth=1.89" SubcatchmentArea 31: Area 31 Flow Length=59' Slope=0.2500 '/' Tc=2.5 min CN=69 Runoff=0.44 cfs 0.020 af Runoff Area=0.457 ac 0.00% Impervious Runoff Depth=1.97" SubcatchmentArea 32: Area 32 Flow Length=122' Slope=0.2500 '/' Tc=3.9 min CN=70 Runoff=1.53 cfs 0.075 af Subcatchment Area 33: Area 33 Runoff Area=1.056 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=419' Tc=4.7 min CN=69 Runoff=3.24 cfs 0.167 af SubcatchmentArea 34: Area 34 Runoff Area=0.434 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=374' Tc=4.5 min CN=69 Runoff=1.35 cfs 0.068 af Runoff Area=1.218 ac 0.00% Impervious Runoff Depth=2.29" SubcatchmentArea 35: Area 35 Flow Length=104' Tc=4.6 min CN=74 Runoff=4.58 cfs 0.233 af SubcatchmentArea 36: Area 36 Runoff Area=1.185 ac 0.00% Impervious Runoff Depth=1.59" Flow Length=106' Tc=4.1 min CN=65 Runoff=3.12 cfs 0.157 af SubcatchmentArea 37: Area 37 Runoff Area=1.291 ac 0.00% Impervious Runoff Depth=1.38" Flow Length=120' Slope=0.2500 '/' Tc=3.9 min CN=62 Runoff=2.93 cfs 0.149 af Subcatchment Area 38: Area 38 Runoff Area=0.795 ac 0.00% Impervious Runoff Depth=1.45" Flow Length=155' Tc=4.3 min CN=63 Runoff=1.86 cfs 0.096 af Subcatchment Area 39: Area 39 Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.00"

Flow Length=168' Tc=9.6 min CN=56 Runoff=0.72 cfs 0.051 af

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SubcatchmentArea 50: Area 50

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SubcatchmentArea 4: Area 4 Runoff Area=0.288 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=296' Tc=4.0 min CN=69 Runoff=0.92 cfs 0.045 af

SubcatchmentArea 40: Area 40

Runoff Area=0.739 ac 0.00% Impervious Runoff Depth=1.00"

Flow Length=141' Tc=7.2 min CN=56 Runoff=0.98 cfs 0.061 af

SubcatchmentArea 41a: Area 41a Runoff Area=0.871 ac 0.00% Impervious Runoff Depth=1.59"

Flow Length=144' Slope=0.0500 '/' Tc=7.8 min CN=65 Runoff=2.00 cfs 0.116 af

SubcatchmentArea 41b: Area 41b Runoff Area=0.712 ac 0.00% Impervious Runoff Depth=2.05"

Flow Length=102' Slope=0.0500 '/' Tc=7.3 min CN=71 Runoff=2.19 cfs 0.122 af

SubcatchmentArea 42: Area 42 Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=2.14 cfs 0.121 af

SubcatchmentArea 43: Area 43 Runoff Area=2.792 ac 0.00% Impervious Runoff Depth=1.32"

Flow Length=419' Tc=14.7 min CN=61 Runoff=3.88 cfs 0.306 af

SubcatchmentArea 44: Area 44 Runoff Area=1.416 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=941' Slope=0.0260 '/' Tc=22.0 min CN=39 Runoff=0.06 cfs 0.022 af

SubcatchmentArea 45: Area 45 Runoff Area=2.044 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=138' Tc=4.0 min CN=69 Runoff=6.52 cfs 0.322 af

SubcatchmentArea 46: Area 46 Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=2.14 cfs 0.121 af

SubcatchmentArea 47: Area 47 Runoff Area=0.079 ac 0.00% Impervious Runoff Depth=2.13"

Flow Length=143' Tc=5.1 min CN=72 Runoff=0.27 cfs 0.014 af

SubcatchmentArea 48: Area 48 Runoff Area=3.726 ac 0.00% Impervious Runoff Depth=2.05"

Flow Length=391' Tc=11.1 min CN=71 Runoff=9.80 cfs 0.636 af

SubcatchmentArea 49: Area 49 Runoff Area=0.698 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=100' Slope=0.0600 '/' Tc=6.8 min CN=39 Runoff=0.03 cfs 0.011 af

**SubcatchmentArea 5: Area 5**Runoff Area=0.504 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=258' Tc=3.4 min CN=69 Runoff=1.66 cfs 0.079 af

Flow Length=296' Tc=3.7 min CN=69 Runoff=0.72 cfs 0.035 af

SubcatchmentArea 51: Area 51 Runoff Area=0.655 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=642' Tc=5.0 min CN=69 Runoff=1.98 cfs 0.103 af

SubcatchmentArea 52: Area 52 Runoff Area=0.237 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=348' Tc=4.0 min CN=69 Runoff=0.76 cfs 0.037 af

Runoff Area=0.223 ac 0.00% Impervious Runoff Depth=1.89"

SubcatchmentArea 53: Area 53 Runoff Area = 0.475 ac 0.00% Impervious Runoff Depth = 1.89"

Flow Length=425' Tc=4.5 min CN=69 Runoff=1.48 cfs 0.075 af

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SubcatchmentArea 54: Area 54 Runoff Area=1.618 ac 0.00% Impervious Runoff Depth=1.18" Flow Length=384' Tc=4.2 min CN=59 Runoff=2.98 cfs 0.160 af

Runoff Area=0.826 ac 0.00% Impervious Runoff Depth=1.12" SubcatchmentArea 55: Area 55 Flow Length=126' Slope=0.2500 '/' Tc=3.9 min CN=58 Runoff=1.45 cfs 0.077 af

SubcatchmentArea 56: Area 56 Runoff Area=1.228 ac 0.00% Impervious Runoff Depth=1.89"

Flow Length=647' Tc=5.7 min CN=69 Runoff=3.66 cfs 0.194 af

Runoff Area=1.089 ac 0.00% Impervious Runoff Depth=1.12" SubcatchmentArea 57: Area 57 Flow Length=158' Tc=4.3 min CN=58 Runoff=1.86 cfs 0.102 af

Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=1.18" Subcatchment Area 58: Area 58 Flow Length=221' Tc=4.4 min CN=59 Runoff=2.17 cfs 0.118 af

Subcatchment Area 59: Area 59 Runoff Area=2.220 ac 0.00% Impervious Runoff Depth=0.88"

Flow Length=240' Tc=4.6 min CN=54 Runoff=2.69 cfs 0.162 af

Subcatchment Area 6: Area 6 Runoff Area=0.936 ac 0.00% Impervious Runoff Depth=1.89" Flow Length=541' Tc=5.3 min CN=69 Runoff=2.81 cfs 0.148 af

Runoff Area=2.476 ac 0.00% Impervious Runoff Depth=1.12" SubcatchmentArea 60: Area 60

Flow Length=263' Tc=4.7 min CN=58 Runoff=4.13 cfs 0.231 af

Runoff Area=1.683 ac 0.00% Impervious Runoff Depth=0.82" SubcatchmentArea 61: Area 61 Flow Length=146' Slope=0.2345 '/' Tc=4.1 min CN=53 Runoff=1.90 cfs 0.115 af

Runoff Area=2.001 ac 0.00% Impervious Runoff Depth=0.33" SubcatchmentArea 62: Area 62

Subcatchment Area 63: Area 63 Runoff Area=2.177 ac 0.00% Impervious Runoff Depth=0.18"

Flow Length=149' Tc=8.2 min CN=39 Runoff=0.09 cfs 0.033 af

Runoff Area=0.594 ac 0.00% Impervious Runoff Depth=0.66" SubcatchmentArea 64: Area 64 Flow Length=147' Slope=0.0544 '/' Tc=7.6 min CN=50 Runoff=0.39 cfs 0.032 af

Runoff Area=1.509 ac 100.00% Impervious Runoff Depth=4.67" SubcatchmentArea 65: Area 65

Tc=0.0 min CN=98 Runoff=10.73 cfs 0.588 af

SubcatchmentArea 66: Area 66 Runoff Area=5.227 ac 0.00% Impervious Runoff Depth=0.18" Flow Length=701' Tc=7.9 min CN=39 Runoff=0.22 cfs 0.079 af

SubcatchmentArea 67: Area 67 Runoff Area=3.035 ac 0.00% Impervious Runoff Depth=0.51"

Flow Length=886' Slope=0.0068 '/' Tc=38.9 min CN=47 Runoff=0.61 cfs 0.128 af

Flow Length=318' Tc=20.8 min CN=43 Runoff=0.24 cfs 0.055 af

SubcatchmentArea 68: Area 68 Runoff Area=0.251 ac 0.00% Impervious Runoff Depth=0.29" Flow Length=52' Slope=0.1154 '/' Tc=3.1 min CN=42 Runoff=0.03 cfs 0.006 af

SubcatchmentArea 69: Area 69 Runoff Area=0.913 ac 0.00% Impervious Runoff Depth=0.46" Flow Length=86' Slope=0.2326 '/' Tc=3.5 min CN=46 Runoff=0.36 cfs 0.035 af HydroCAD® 10.20-2d s/n 05804 © 2021 HydroCAD Software Solutions LLC

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Runoff Area=0.986 ac 0.00% Impervious Runoff Depth=1.89"
Flow Length=608' Tc=6.6 min CN=69 Runoff=2.87 cfs 0.155 af

Runoff Area=1.671 ac 0.00% Impervious Runoff Depth=0.71"
Flow Length=126' Tc=4.0 min CN=51 Runoff=1.52 cfs 0.099 af

Runoff Area=9.875 ac 0.00% Impervious Runoff Depth=0.29"
Flow Length=1,337' Tc=16.6 min CN=42 Runoff=0.99 cfs 0.238 af

Runoff Area=35,545 sf 0.00% Impervious Runoff Depth=1.89"
Flow Length=348' Tc=3.6 min CN=69 Runoff=2.66 cfs 0.129 af

Runoff Area=10,716 sf 0.00% Impervious Runoff Depth=1.89"
Flow Length=144' Tc=2.3 min CN=69 Runoff=0.86 cfs 0.039 af

**Reach RD1: Roadside Ditch 1** Avg. Flow Depth=0.10' Max Vel=1.34 fps Inflow=0.61 cfs 0.128 af n=0.030 L=440.6' S=0.0188 '/' Capacity=47.16 cfs Outflow=0.60 cfs 0.128 af

**Reach RD2: Roadside Ditch 2** Avg. Flow Depth=0.39' Max Vel=2.69 fps Inflow=11.23 cfs 0.814 af n=0.030 L=433.0' S=0.0162 '/' Capacity=72.77 cfs Outflow=10.38 cfs 0.814 af

**Reach RD3: Roadside Ditch 3** Avg. Flow Depth=0.67' Max Vel=3.99 fps Inflow=8.44 cfs 0.412 af n=0.030 L=821.0' S=0.0288 '/' Capacity=20.76 cfs Outflow=7.09 cfs 0.412 af

**Reach RD4: Roadside Ditch 4** Avg. Flow Depth=0.12' Max Vel=1.05 fps Inflow=1.52 cfs 0.099 af n=0.030 L=495.6' S=0.0090 '/' Capacity=54.26 cfs Outflow=0.92 cfs 0.099 af

**Reach RD5: Roadside Ditch 5** Avg. Flow Depth=0.38' Max Vel=3.65 fps Inflow=2.25 cfs 0.109 af n=0.030 L=288.0' S=0.0531 '/' Capacity=28.18 cfs Outflow=2.01 cfs 0.109 af

**Reach S1.1: Swale S1.1**Avg. Flow Depth=0.14' Max Vel=2.32 fps Inflow=3.12 cfs 0.157 af n=0.030 L=321.0' S=0.0319 '/' Capacity=338.34 cfs Outflow=2.76 cfs 0.157 af

**Reach S1.2: Swale S1.2**Avg. Flow Depth=0.76' Max Vel=3.60 fps Inflow=30.31 cfs 1.696 af n=0.030 L=202.8' S=0.0108 '/' Capacity=196.86 cfs Outflow=28.58 cfs 1.696 af

**Reach S1.3: Swale S1.3**Avg. Flow Depth=0.76' Max Vel=4.15 fps Inflow=34.68 cfs 2.079 af n=0.030 L=72.2' S=0.0144 '/' Capacity=227.36 cfs Outflow=34.14 cfs 2.079 af

**Reach S1.4: Swale S1.4**Avg. Flow Depth=1.03' Max Vel=2.73 fps Inflow=35.12 cfs 2.141 af n=0.030 L=148.0' S=0.0045 '/' Capacity=126.50 cfs Outflow=33.65 cfs 2.141 af

**Reach S1.5: Swale S1.5**Avg. Flow Depth=0.20' Max Vel=1.12 fps Inflow=2.14 cfs 0.121 af n=0.030 L=179.7' S=0.0050 '/' Capacity=134.06 cfs Outflow=1.95 cfs 0.121 af

**Reach S1.6: Swale S1.6**Avg. Flow Depth=1.04' Max Vel=3.05 fps Inflow=39.20 cfs 2.499 af n=0.030 L=252.0' S=0.0054 '/' Capacity=139.68 cfs Outflow=37.41 cfs 2.499 af

**Reach S1.7: Swale S1.7**Avg. Flow Depth=0.31' Max Vel=1.88 fps Inflow=2.19 cfs 0.122 af n=0.030 L=245.8' S=0.0099 '/' Capacity=90.14 cfs Outflow=1.99 cfs 0.122 af

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Reach S1.8: Swale S1.8	Avg. Flow Depth=0.98' Max Vel=3.45 fps Inflow=41.27 cfs 2.805 af n=0.030 L=422.0' S=0.0075 '/' Capacity=163.67 cfs Outflow=39.57 cfs 2.805 af
Reach S2.1: Swale S2.1	Avg. Flow Depth=1.31' Max Vel=3.17 fps Inflow=39.58 cfs 2.837 af n=0.030 L=389.0' S=0.0054 '/' Capacity=97.05 cfs Outflow=38.03 cfs 2.837 af
Reach S2.2: Swale S2.2	Avg. Flow Depth=1.29' Max Vel=3.33 fps Inflow=65.64 cfs 5.558 af n=0.030 L=411.0' S=0.0049 '/' Capacity=152.61 cfs Outflow=64.61 cfs 5.558 af
Reach S2.3: Swale S2.3	Avg. Flow Depth=0.99' Max Vel=4.71 fps Inflow=65.54 cfs 5.750 af n=0.030 L=307.0' S=0.0130 '/' Capacity=249.72 cfs Outflow=64.79 cfs 5.750 af
Reach S3.1: Swale S3.1	Avg. Flow Depth=0.37' Max Vel=1.62 fps Inflow=6.52 cfs 0.322 af n=0.030 L=357.0' S=0.0050 '/' Capacity=133.76 cfs Outflow=5.46 cfs 0.322 af
Reach S3.2: Swale S3.2	Avg. Flow Depth=0.79' Max Vel=2.48 fps Inflow=22.25 cfs 1.287 af n=0.030 L=34.0' S=0.0050 '/' Capacity=133.95 cfs Outflow=21.76 cfs 1.287 af
Reach S3.3: Swale S3.3	Avg. Flow Depth=0.60' Max Vel=3.48 fps Inflow=21.76 cfs 1.287 af n=0.030 L=200.0' S=0.0130 '/' Capacity=215.99 cfs Outflow=20.93 cfs 1.287 af
Reach S3.4: Swale S3.4	Avg. Flow Depth=0.33' Max Vel=1.67 fps Inflow=2.14 cfs 0.121 af n=0.030 L=283.0' S=0.0071 '/' Capacity=76.21 cfs Outflow=1.94 cfs 0.121 af
Reach S3.5: Swale S3.5	Avg. Flow Depth=1.27' Max Vel=2.26 fps Inflow=39.57 cfs 2.531 af n=0.030 L=318.5' S=0.0024 '/' Capacity=93.14 cfs Outflow=36.85 cfs 2.531 af
Reach S4.1: Swale S4.1	Avg. Flow Depth=0.14' Max Vel=1.62 fps Inflow=2.74 cfs 0.145 af n=0.030 L=240.0' S=0.0153 '/' Capacity=70.22 cfs Outflow=2.38 cfs 0.145 af
Reach S4.2: Swale S4.2	Avg. Flow Depth=0.26' Max Vel=2.12 fps Inflow=6.29 cfs 0.372 af n=0.030 L=259.3' S=0.0127'/ Capacity=63.88 cfs Outflow=5.84 cfs 0.372 af
Reach S4.3: Swale S4.3	Avg. Flow Depth=0.74' Max Vel=1.83 fps Inflow=19.18 cfs 1.179 af n=0.030 L=362.9' S=0.0027 '/' Capacity=108.12 cfs Outflow=17.09 cfs 1.179 af
Reach S4.4: Swale S4.4	Avg. Flow Depth=0.75' Max Vel=2.28 fps Inflow=22.46 cfs 1.757 af n=0.030 L=495.6' S=0.0040 '/' Capacity=132.85 cfs Outflow=21.52 cfs 1.757 af
Reach S4.5: Swale S4.5	Avg. Flow Depth=0.60' Max Vel=3.08 fps Inflow=22.64 cfs 1.939 af n=0.030 L=411.1' S=0.0097 '/' Capacity=465.89 cfs Outflow=22.15 cfs 1.939 af
Reach S4.6: Swale S4.6	Avg. Flow Depth=1.24' Max Vel=5.00 fps Inflow=89.26 cfs 8.232 af n=0.030 L=537.0' S=0.0112 '/' Capacity=499.25 cfs Outflow=87.87 cfs 8.232 af
Reach S5.1: Swale S5.1	Avg. Flow Depth=0.22' Max Vel=2.09 fps Inflow=4.58 cfs 0.233 af n=0.030 L=478.0' S=0.0154 '/' Capacity=235.22 cfs Outflow=3.89 cfs 0.233 af
Pond C8: Culvert C8	Peak Elev=811.21' Storage=0.000 af Inflow=0.95 cfs 0.043 af 12.0" Round Culvert n=0.012 L=85.6' S=0.0245 '/' Outflow=0.96 cfs 0.043 af

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Pond N: North Infiltration Area Peak Elev=803.86' Storage=20,107 cf Inflow=11.66 cfs 1.151 af

Outflow=1.38 cfs 1.151 af

**Pond Sed Pond: SedimentationBasin** Peak Elev=792.00' Storage=164,114 cf Inflow=89.59 cfs 8.931 af 6.100 af Primary=10.00 cfs 2.831 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=15.47 cfs 8.931 af

Link C1: Culvert C1	Inflow=0.60 cfs 0.128 af Primary=0.60 cfs 0.128 af
Link C10: Culvert C10	Inflow=2.76 cfs 0.157 af Primary=2.76 cfs 0.157 af
Link C2: Culvert C2	Inflow=10.95 cfs 0.645 af Primary=10.95 cfs 0.645 af
Link C3: Culvert C3	Inflow=28.58 cfs 1.696 af Primary=28.58 cfs 1.696 af
Link C4: Culvert C4	Inflow=34.14 cfs 2.079 af Primary=34.14 cfs 2.079 af
Link C5: Culvert C5	Inflow=37.41 cfs 2.499 af Primary=37.41 cfs 2.499 af
Link C6: Culvert C6	Inflow=30.67 cfs 1.895 af Primary=30.67 cfs 1.895 af
Link C7: Culvert C7	Inflow=67.17 cfs 6.238 af Primary=67.17 cfs 6.238 af
Link C9: Culvert C9	Inflow=11.16 cfs 0.913 af Primary=11.16 cfs 0.913 af
Link F10: Flume 10	Inflow=0.95 cfs 0.043 af Primary=0.95 cfs 0.043 af
Link F1A: Flume 1A	Inflow=5.38 cfs 0.292 af Primary=5.38 cfs 0.292 af
Link F1B: Flume 1B	Inflow=12.06 cfs 0.645 af Primary=12.06 cfs 0.645 af
Link F2A: Flume 2A	Inflow=1.98 cfs 0.100 af Primary=1.98 cfs 0.100 af
Link F2B: Flume 2B	Inflow=6.58 cfs 0.347 af Primary=6.58 cfs 0.347 af
Link F3: Flume 3	Inflow=1.38 cfs 0.067 af Primary=1.38 cfs 0.067 af

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Prepared	by Sc	S Engineers

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91" Printed 8/28/2023

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Link F4A: Flume 4A	Inflow=3.47 cfs 0.191 af
	Primary=3.47 cfs 0.191 af
Link F4B: Flume 4B	Inflow=9.10 cfs 0.488 af Primary=9.10 cfs 0.488 af
Link F5A: Flume 5A	Inflow=0.76 cfs 0.037 af
	Primary=0.76 cfs 0.037 af
Link F5B: Flume 5B	Inflow=2.23 cfs 0.112 af Primary=2.23 cfs 0.112 af
Link F6A: Flume 6A	Inflow=4.38 cfs 0.227 af
	Primary=4.38 cfs 0.227 af
Link F6B: Flume 6B	Inflow=9.20 cfs 0.473 af Primary=9.20 cfs 0.473 af
Link F7A: Flume 7A	Inflow=3.51 cfs 0.167 af
LIIIK F/A. Fluille /A	Primary=3.51 cfs 0.167 af
Link F7B: Flume 7B	Inflow=10.65 cfs 0.537 af
Link F70. Floor a 70	Primary=10.65 cfs 0.537 af
Link F7C: Flume 7C	Inflow=17.47 cfs 0.889 af Primary=17.47 cfs 0.889 af
Link F8A: Flume 8A	Inflow=1.19 cfs 0.060 af
	Primary=1.19 cfs 0.060 af
Link F8B: Flume 8B	Inflow=4.66 cfs 0.236 af Primary=4.66 cfs 0.236 af
Link F9A: Flume 9A	Inflow=6.88 cfs 0.386 af
	Primary=6.88 cfs 0.386 af
Link F9B: Flume 9B	Inflow=15.84 cfs 0.874 af Primary=15.84 cfs 0.874 af
Link F9C: Flume 9C	Inflow=25.24 cfs 1.389 af
	Primary=25.24 cfs 1.389 af
Link Wetland: Wetland	Inflow=10.00 cfs 2.831 af Primary=10.00 cfs 2.831 af
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

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#### **Summary for Subcatchment Area 1: Area 1**

Runoff = 3.75 cfs @ 12.14 hrs, Volume= 0.204 af, Depth= 1.89"

Routed to Link F1A: Flume 1A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
1.296 69 Pasture/grassland/range, Fair, HSG B								
	1.	296	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	5.2	93	0.1000	0.30		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.5	7	0.2500	0.26		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.7	151	0.2500	3.50		Shallow Concentrated Flow,		
_	0.3	113	0.0200	6.74	80.87	Short Grass Pasture Kv= 7.0 fps  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
	6.7	364	Total					

#### **Summary for Subcatchment Area 10: Area 10**

Runoff = 1.68 cfs @ 12.14 hrs, Volume= 0.090 af, Depth= 1.89"

Routed to Link F9A: Flume 9A

	Area	(ac) C	N Des	cription					
_	0.573 69 Pasture/grassland/range, Fair, HSG B								
_	0.	573	100.	00% Pervi	ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	4.6	79	0.1000	0.29		Sheet Flow,			
	1.1	21	0.2500	0.32		Grass: Short n= 0.150 P2= 2.77"  Sheet Flow,			
	0.3	53	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	0.4	168	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
-	6.4	321	Total			· U			

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#### **Summary for Subcatchment Area 11: Area 11**

Runoff = 5.22 cfs @ 12.15 hrs, Volume= 0.295 af, Depth= 1.89"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription						
1.	1.872 69 Pasture/grassland/range, Fair, HSG B								
1.	872	100.	00% Pervi	ous Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.4	96	0.1000	0.30		Sheet Flow,				
					Grass: Short n= 0.150 P2= 2.77"				
0.3	4	0.2500	0.23		Sheet Flow,				
					Grass: Short n= 0.150 P2= 2.77"				
0.4	90	0.2500	3.50		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
1.5	606	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm				
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'				
					n= 0.030 Earth, grassed & winding				
7.6	796	Total							

#### **Summary for Subcatchment Area 12: Area 12**

Runoff = 4.83 cfs @ 12.13 hrs, Volume= 0.254 af, Depth= 1.89"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
	1.	610 6	9 Past	ure/grassl	and/range,	Fair, HSG B		
1.610 100.00% Pervious Area								
	_				_			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	31	0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	1.4	581	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm		
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'		
_						n= 0.030 Earth, grassed & winding		
	5.3	712	Total					

5.3 712 Total

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#### **Summary for Subcatchment Area 13: Area 13**

Runoff = 1.94 cfs @ 12.11 hrs, Volume= 0.099 af, Depth= 1.89"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription		
	0.	626 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	626	100.	00% Pervi	ous Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.5	183	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
_	4.5	333	Total	•		

### **Summary for Subcatchment Area 14: Area 14**

Runoff = 1.93 cfs @ 12.11 hrs, Volume= 0.098 af, Depth= 1.89"

Routed to Link F2B: Flume 2B

Area	(ac) C	N Desc	cription							
0.	0.620 69 Pasture/grassland/range, Fair, HSG B									
0.	620	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
3.8	100	0.2500	0.43	•	Sheet Flow,					
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
4.4	331	Total			o.oo Earth, grassou a Winding					

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### **Summary for Subcatchment Area 15: Area 15**

Runoff = 2.80 cfs @ 12.14 hrs, Volume= 0.149 af, Depth= 1.89"

Routed to Link F2B: Flume 2B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	a (ac) C	N Des	cription		
	0.943	69 Past	ure/grassl	and/range,	Fair, HSG B
	0.943	100.	00% Pervi	ous Area	
To (min)	5	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	100	0.1500	0.35		Sheet Flow,
0.5	95	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	269	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
5.9	464	Total			

### **Summary for Subcatchment Area 16: Area 16**

Runoff = 1.78 cfs @ 12.11 hrs, Volume= 0.090 af, Depth= 1.89"

Routed to Link F6B: Flume 6B

	Area	(ac) C	N Des	cription				
0.571 69 Pasture/grassland/range, Fair, HSG B								
	0.	571	100.	00% Pervi	ous Area			
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	3.8	100	0.2500	0.43		Sheet Flow,		
	0.2	44	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	0.4	171	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
Ī	4.4	315	Total					

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#### **Summary for Subcatchment Area 17: Area 17**

Runoff = 3.04 cfs @ 12.12 hrs, Volume= 0.156 af, Depth= 1.89"

Routed to Link F6B: Flume 6B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription				
0.990 69 Pasture/grassland/range, Fair, HSG B							
0	.990	100.	00% Pervi	ous Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.8	100	0.2500	0.43		Sheet Flow,		
0.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
0.7	278	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm		
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
4.7	424	Total			II- 0.030 Eaitii, grasseu α Willding		

# Summary for Subcatchment Area 18: Area 18

Runoff = 5.06 cfs @ 12.12 hrs, Volume= 0.261 af, Depth= 1.89"

Routed to Link F7B: Flume 7B

_	Area	(ac) C	N Des	cription		
	1.	656 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	656	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.8	88	0.2045	0.39	, ,	Sheet Flow,
	1.0	420	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	4 8	508	Total			

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#### **Summary for Subcatchment Area 19: Area 19**

Runoff = 2.16 cfs @ 12.11 hrs, Volume= 0.109 af, Depth= 1.89"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
_	0.	689 6	9 Past	ure/grassl	and/range,	Fair, HSG B		
_	0.	689	100.	00% Pervi	0% Pervious Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	3.8	100	0.2500	0.43		Sheet Flow,		
	0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	0.3	136	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm		
_						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
	4.3	286	Total					

# Summary for Subcatchment Area 2: Area 2

Runoff = 1.71 cfs @ 12.12 hrs, Volume= 0.088 af, Depth= 1.89"

Routed to Link F1A: Flume 1A

	Area	(ac) C	N Desc	cription				
	0.	557 6	9 Past	sture/grassland/range, Fair, HSG B				
	0.	557	100.	00% Pervi	ous Area			
(	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	4.2	100	0.1950	0.39		Sheet Flow,		
	0.1	22	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'		
						n= 0.030 Earth, grassed & winding		
	4.7	303	Total					

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#### **Summary for Subcatchment Area 20: Area 20**

Runoff = 1.19 cfs @ 12.11 hrs, Volume= 0.060 af, Depth= 1.89"

Routed to Link F8A: Flume 8A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Des	cription			
 0.	381 6	9 Past	ure/grassl	and/range,	Fair, HSG B	
0.	381	100.	00% Pervi	vious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
3.8	100	0.2500	0.43		Sheet Flow,	
0.0	7	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
0.5	199	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
43	306	Total			<del>-</del>	

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### **Summary for Subcatchment Area 21: Area 21**

Runoff = 1.62 cfs @ 12.11 hrs, Volume= 0.081 af, Depth= 1.89"

Routed to Link F9B: Flume 9B

Area	(ac) C	N Desc	cription		
0.	516 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	516	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.0	7	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.5	205	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
4.3	312	Total			n= 0.030 Earth, grassed & winding

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#### **Summary for Subcatchment Area 22: Area 22**

Runoff = 7.54 cfs @ 12.14 hrs, Volume= 0.407 af, Depth= 1.89"

Routed to Link F9B: Flume 9B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	2.	579 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	2.	579	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.1	21	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	2.5	1,025	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	6.4	1 146	Total			<u>-</u>

#### .4 1,146 Lotal

### **Summary for Subcatchment Area 23: Area 23**

Runoff = 1.38 cfs @ 12.10 hrs, Volume= 0.067 af, Depth= 1.89"

Routed to Link F3: Flume 3

_	Area	(ac) C	N Desc	cription		
	0.	427 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	427	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.9	70	0.2500	0.40		Sheet Flow,
	0.0	007	0.0000	4.00	00.00	Grass: Short n= 0.150 P2= 2.77"
	8.0	227	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
	0.1	65	0.2500	12.26	441.43	n= 0.030 Earth, grassed & winding Trap/Vee/Rect Channel Flow, Riprap Flume
	0.1	00	0.2000	12.20	441.40	Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
_						n= 0.078 Riprap, 12-inch
	3.8	362	Total			

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#### **Summary for Subcatchment Area 24: Area 24**

Runoff = 3.61 cfs @ 12.12 hrs, Volume= 0.186 af, Depth= 1.89"

Routed to Link F7C: Flume 7C

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription		
	1.	177 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	177	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.3	60	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.6	225	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	4.7	385	Total			-

### **Summary for Subcatchment Area 25: Area 25**

Runoff = 2.12 cfs @ 12.11 hrs, Volume= 0.108 af, Depth= 1.89"

Routed to Link F8B: Flume 8B

Area	(ac) C	N Desc	cription				
0.682 69 Pasture/grassland/range, Fair, HSG B							
0.	682	100.	00% Pervi	ous Area			
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.8	100	0.2500	0.43		Sheet Flow,		
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
 0.5	199	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
4.5	349	Total					

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#### **Summary for Subcatchment Area 26: Area 26**

Runoff = 2.10 cfs @ 12.11 hrs, Volume= 0.107 af, Depth= 1.89"

Routed to Link F9C: Flume 9C

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription		
	0.	677 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	677	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.5	191	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
_	4.5	341	Total			11 0.000 Earth, grassed & Wilding

### **Summary for Subcatchment Area 27: Area 27**

Runoff = 7.46 cfs @ 12.15 hrs, Volume= 0.409 af, Depth= 1.89"

Routed to Link F9C: Flume 9C

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	2.	594 6	Fair, HSG B			
	2.	594	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.8	99	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	3.1	1,258	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	6.0	4 257	Tatal			

6.9 1,357 Total

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#### **Summary for Subcatchment Area 28: Area 28**

Runoff = 8.44 cfs @ 12.10 hrs, Volume= 0.412 af, Depth= 2.29"

Routed to Reach RD3: Roadside Ditch 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	Area (ac) CN Description						
	1.	735	69 Past	ure/grassl	and/range,	Fair, HSG B		
_	0.	424	96 Grav					
2.159 74 Weighted Average								
	2.	159	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	25	0.2500	3.50		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	3.9	125	Total					

#### **Summary for Subcatchment Area 29: Area 29**

Runoff = 2.25 cfs @ 12.10 hrs, Volume= 0.109 af, Depth= 2.13"

Routed to Reach RD5: Roadside Ditch 5

	Area	(ac) C	N Des	cription				
	0.	543 6	39 Past	ture/grassl	and/range,	Fair, HSG B		
	0.	073 9	96 Grav	,				
_	0.616 72 Weighted Average							
	0.616 100.00% Pervious Area							
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•		
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.0	9	0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
_	3.8	109	Total			•		

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#### **Summary for Subcatchment Area 3: Area 3**

Runoff = 1.06 cfs @ 12.12 hrs, Volume= 0.055 af, Depth= 1.89"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

 Area	(ac) C	N Desc	cription		
0.	348 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	348	100.	00% Pervi	ous Area	
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	100	0.1950	0.39		Sheet Flow,
0.2	36	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	162	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
 4.8	298	Total			

### **Summary for Subcatchment Area 30: Area 30**

Runoff = 0.52 cfs @ 12.08 hrs, Volume= 0.023 af, Depth= 1.89"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription					
	0.	149 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
0.149 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	0.7	12	0.2500	0.28		Sheet Flow,			
	0.5	199	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
_	1 2	211	Total						

1.2 211 Total

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### **Summary for Subcatchment Area 31: Area 31**

Runoff = 0.44 cfs @ 12.09 hrs, Volume=

0.020 af, Depth= 1.89"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription						
	0.126 69 Pasture/grassland/range, Fair, HSG B									
	0.126 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	2.5	59	0.2500	0.39		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"		

#### **Summary for Subcatchment Area 32: Area 32**

Runoff = 1.53 cfs @ 12.11 hrs, Volume=

0.075 af, Depth= 1.97"

Routed to Reach S3.2 : Swale S3.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	Area (ac) CN Description						
	0.	435	69 Pas	ture/grassl	and/range,	Fair, HSG B		
	0.	022	96 Gra	vel surface	, HSG A			
	0.457 70 Weighted Average							
0.457 100.00% Pervious Area								
	Тс	Length	•	•	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	22	0.2500	3.50		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	3.9	122	Total					

#### **Summary for Subcatchment Area 33: Area 33**

Runoff = 3.24 cfs @ 12.12 hrs, Volume= 0.167 af, Depth= 1.89"

Routed to Link F7C: Flume 7C

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	Area	(ac) C	N Des	cription		
_				-		
_	1.	056 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	056	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
_	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.3	57	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.6	262	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
						n= 0.030 Earth, grassed & winding
_	4.7	419	Total			, , , , , , , , , , , , , , , , , , , ,

#### **Summary for Subcatchment Area 34: Area 34**

Runoff = 1.35 cfs @ 12.11 hrs, Volume= 0.068 af, Depth= 1.89"

Routed to Link F8B: Flume 8B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	0.	434 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	434	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.8	100	0.2500	0.43		Sheet Flow,
	0.1	15	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Crass Posture, Kur 7.0 fee
	0.6	259	0.0200	6.74	80.87	Short Grass Pasture Kv= 7.0 fps <b>Trap/Vee/Rect Channel Flow, Diversion Berm</b> Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
-	4.5	274	T-4-1			, , , , , , , , , , , , , , , , , , ,

#### 4.5 374 Total

## **Summary for Subcatchment Area 35: Area 35**

Runoff = 4.58 cfs @ 12.11 hrs, Volume= 0.233 af, Depth= 2.29" Routed to Reach S5.1 : Swale S5.1

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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	Area	(ac) C	N Desc	cription		
	0.	986 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	232 9	6 Grav	el surface	, HSG A	
	1.	218 7	'4 Weig	ghted Aver	age	
1.218 100.00% Pervious Area						
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.3	70	0.1736	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.6	20	0.0050	0.53		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.7	10	0.1766	0.24		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.0	4	0.1766	2.94		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	4.6	104	Total			

#### **Summary for Subcatchment Area 36: Area 36**

Runoff = 3.12 cfs @ 12.11 hrs, Volume= 0.157 af, Depth= 1.59"

Routed to Reach S1.1: Swale S1.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription					
0.322 39 Pasture/grassland/range, Good, HSG A									
0.696 69 Pasture/grassland/range, Fair, HSG B									
0.167 96 Gravel surface, HSG A									
_	1.185 65 Weighted Average								
	1.	185	100.	00% Pervi	ous Area				
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.4	72	0.1736	0.35		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.6	20	0.0050	0.53		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 2.77"			
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	4.1	106	Total						

### **Summary for Subcatchment Area 37: Area 37**

Runoff = 2.93 cfs @ 12.11 hrs, Volume= 0.149 af, Depth= 1.38"

Routed to Reach S1.2 : Swale S1.2

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	Area	(ac) (	ON Des	cription					
	0.415 39 Pasture/grassland/range, Good, HSG A								
	0.	743	69 Pas	ture/grassl	and/range,	Fair, HSG B			
	0.133 96 Gravel surface, HSG A								
	1.291 62 Weighted Average								
1.291 100.00% Pervious Area									
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	20	0.2500	3.50		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	3.9	120	Total			·			

#### **Summary for Subcatchment Area 38: Area 38**

Runoff = 1.86 cfs @ 12.11 hrs, Volume= 0.096 af, Depth= 1.45"

Routed to Reach S1.3: Swale S1.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
	0.	263 3	39 Past	ure/grassl	and/range,	Good, HSG A		
	0.	409 6	9 Past	ure/grassl	and/range,	Fair, HSG B		
_	0.	123 9	96 Grav	el surface	, HSG A			
0.795 63 Weighted Average								
	0.	795	100.	00% Pervi	ous Area			
	_							
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	14	0.2500	3.50		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.2	17	0.0050	1.44		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	0.2	24	0.0833	2.02		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	4.3	155	Total					

#### **Summary for Subcatchment Area 39: Area 39**

Runoff = 0.72 cfs @ 12.19 hrs, Volume= 0.051 af, Depth= 1.00"

Routed to Reach S1.3: Swale S1.3

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Area	(ac) C	N Desc	cription		
0.	.317 3	89 Past	ure/grassla	and/range,	Good, HSG A
0.	.243 6				Fair, HSG B
			el surface		, -
			ghted Aver	<i></i>	
_	.620		00% Pervi		
0.	.020	100.	00 /0 T CIVI	ous Alca	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.2	81	0.0245	0.16	, ,	Sheet Flow,
0.2	01	0.0210	0.10		Grass: Short n= 0.150 P2= 2.77"
1.0	19	0.2500	0.31		Sheet Flow,
1.0	10	0.2000	0.01		Grass: Short n= 0.150 P2= 2.77"
0.1	29	0.2500	3.50		Shallow Concentrated Flow,
0.1	29	0.2300	3.30		·
0.0	20	0.0050	1 11		Short Grass Pasture Kv= 7.0 fps
0.2	20	0.0050	1.44		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.1	19	0.1053	2.27		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
9.6	168	Total			

#### Summary for Subcatchment Area 4: Area 4

Runoff = 0.92 cfs @ 12.11 hrs, Volume= 0.045 af, Depth= 1.89"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Desc	cription		
0.	.288 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	.288	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	49	0.0820	0.24	,	Sheet Flow,
0.6	247	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.0	296	Total	·	·	

### **Summary for Subcatchment Area 40: Area 40**

Runoff = 0.98 cfs @ 12.16 hrs, Volume= 0.061 af, Depth= 1.00"

Routed to Reach S1.4: Swale S1.4

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Area	(ac) C	N Desc	cription		
0.	.389 3	39 Past	ure/grassl	and/range,	Good, HSG A
			•	•	Fair, HSG B
0.	.080 9	<u>6 Grav</u>	<u>el surface</u>	<u>, HSG A</u>	
		56 Weig	ghted Aver	age	
0.	.739	100.	00% Pervi	ous Area	
-		01		0 :	D
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.5	49	0.0408	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
2.1	47	0.2500	0.37		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	4	0.0050	0.39		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 2.77"
0.2	19	0.0050	1.44		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.2	22	0.1136	2.36		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	141	Total			·

### Summary for Subcatchment Area 41a: Area 41a

Runoff = 2.00 cfs @ 12.16 hrs, Volume= 0.116 af, Depth= 1.59" Routed to Reach S1.6 : Swale S1.6

_	Area	(ac) C	N Des	cription		
0.249 39 Pasture/grassland/range, G						Good, HSG A
0.489 69 Pasture/grassland/range,					and/range,	Fair, HSG B
_	0.	133	96 Grav	el surface	, HSG A	
0.871 65 Weighted Average						
0.871 100.00% Pervious Area						
	_		01			B
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.3	100	0.0500	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.5	44	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	7.8	144	Total			

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### Summary for Subcatchment Area 41b: Area 41b

Runoff = 2.19 cfs @ 12.15 hrs, Volume= 0.122 af, Depth= 2.05"

Routed to Reach S1.7: Swale S1.7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription			
0.655 69 Pasture/grassland/range, Fa						Fair, HSG B	
_	0.	057	96 Grav	/el surface	, HSG A		
0.712 71 Weighted Average					age		
	0.	712	100.	00% Pervi	ous Area		
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.3	100	0.0500	0.23		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.0	2	0.0500	1.57		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	7.3	102	Total				

#### **Summary for Subcatchment Area 42: Area 42**

Runoff = 2.14 cfs @ 12.15 hrs, Volume= 0.121 af, Depth= 1.89"

Routed to Reach S1.5: Swale S1.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription					
	0.769 69 Pasture/grassland/range, Fair, HSG B								
	0.	.769	100.	00% Pervi	ous Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	7.3	100	0.0500	0.23	•	Sheet Flow,			
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
-	7.7	139	Total						

#### **Summary for Subcatchment Area 43: Area 43**

Runoff = 3.88 cfs @ 12.25 hrs, Volume= 0.306 af, Depth= 1.32"

Routed to Reach S1.8: Swale S1.8

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Area	(ac) C	N Desc	cription						
0.	.797 3	9 Past	ure/grassl	and/range,	Good, HSG A				
1.	.938 6	9 Past	Pasture/grassland/range, Fair, HSG B						
0.	.057 9	6 Grav	/el surface	, HSG A					
2.	.792 6	31 Weig	ghted Aver	age					
2.	.792	100.	00% Pervi	ous Area					
_									
Тс	Length	Slope	Velocity	Capacity	Description				
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.3	100	0.0500	0.23		Sheet Flow,				
					Grass: Short n= 0.150 P2= 2.77"				
1.3	119	0.0500	1.57		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.1	22	0.1905	3.06		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
6.0	178	0.0050	0.49		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
14.7	419	Total							

### **Summary for Subcatchment Area 44: Area 44**

Runoff 0.06 cfs @ 13.20 hrs, Volume= 0.022 af, Depth= 0.18"

Routed to Reach S2.1 : Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription		
	1.	416 3	9 Past	ure/grassla	and/range,	Good, HSG A
	1.	416	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	9.5	100	0.0260	0.18		Sheet Flow,
	2.9	194	0.0260	1.13		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	9.6	647	0.0260	1.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps  Short Grass Pasture Kv= 7.0 fps
	22.0	941	Total			•

#### **Summary for Subcatchment Area 45: Area 45**

6.52 cfs @ 12.11 hrs, Volume= 0.322 af, Depth= 1.89"

Routed to Reach S3.1: Swale S3.1

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_	Area	(ac) C	N Desc	cription				
2.044 69 Pasture/grassland/range, Fair, HSG B								
	2.	044	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
_	3.8	100	0.2500	0.43	, ,	Sheet Flow,		
	0.2	38	0.2632	3.59		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
_	4.0	138	Total					

#### **Summary for Subcatchment Area 46: Area 46**

Runoff = 2.14 cfs @ 12.15 hrs, Volume= 0.121 af, Depth= 1.89"

Routed to Reach S3.4: Swale S3.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription					
	0.769 69 Pasture/grassland/range, Fair, HSG B								
0.769 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	7.3	100	0.0500	0.23	,	Sheet Flow,			
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
_	7.7	139	Total		·				

### **Summary for Subcatchment Area 47: Area 47**

Runoff = 0.27 cfs @ 12.12 hrs, Volume= 0.014 af, Depth= 2.13"

Routed to Link C6: Culvert C6

_	Area (ac)	CN	Description
	0.070	69	Pasture/grassland/range, Fair, HSG B
_	0.009	96	Gravel surface, HSG A
	0.079	72	Weighted Average
	0.079		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.5	90	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.2	10	0.0500	0.14		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	10	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	15	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	18	0.1390	2.61		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	5.1	143	Total			

#### **Summary for Subcatchment Area 48: Area 48**

Runoff = 9.80 cfs @ 12.20 hrs, Volume= 0.636 a

0.636 af, Depth= 2.05"

Routed to Reach S3.5: Swale S3.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription		
	3.	470 6	9 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	256 g	96 Grav	∕el surface	, HSG A	
	3.	726 7	/1 Wei	ghted Aver	age	
	3.	726	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.3	100	0.0500	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	83	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.9	208	0.0289	1.19		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	11.1	391	Total			

### **Summary for Subcatchment Area 49: Area 49**

Runoff = 0.03 cfs @ 12.51 hrs, Volume= 0.011 af, Depth= 0.18"

Routed to Reach S2.1: Swale S2.1

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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	Area	(ac) C	N Des	cription						
0.698 39 Pasture/grassland/range, Good, HSG A										
	0.698 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	6.8	100	0.0600	0.25		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"		_

#### **Summary for Subcatchment Area 5: Area 5**

Runoff = 1.66 cfs @ 12.10 hrs, Volume= 0.079 af, Depth= 1.89"

Routed to Link F6A: Flume 6A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
_	0.	504 6	9 Past	Fair, HSG B		
0.504 100.00% Pervious Area					ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.9	72	0.2500	0.41		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.5	186	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	3.4	258	Total			

## **Summary for Subcatchment Area 50: Area 50**

Runoff = 0.72 cfs @ 12.10 hrs, Volume= 0.035 af, Depth= 1.89"

Routed to Link F4A: Flume 4A

_	Area (ac)	CN	Description
	0.223	69	Pasture/grassland/range, Fair, HSG B
	0.223		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.1	77	0.2500	0.41		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.5	157	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
						n= 0.030 Earth, grassed & winding
	0.1	62	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
						n= 0.078 Riprap, 12-inch
	3.7	296	Total			

#### **Summary for Subcatchment Area 51: Area 51**

Runoff = 1.98 cfs @ 12.12 hrs, Volume= 0.103 af, Depth= 1.89"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription		
	0.	655 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	655	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.8	100	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.1	11	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	8.0	314	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	0.3	217	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
_	5.0	642	Total		•	

#### **Summary for Subcatchment Area 52: Area 52**

Runoff = 0.76 cfs @ 12.11 hrs, Volume= 0.037 af, Depth= 1.89"

Routed to Link F5A: Flume 5A

MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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	Area	(ac) C	N Des	cription		
	0.	237 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	237	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.3	83	0.2500	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.5	138	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
						n= 0.030 Earth, grassed & winding
	0.2	127	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
_	4.0	348	Total			

#### **Summary for Subcatchment Area 53: Area 53**

Runoff = 1.48 cfs @ 12.11 hrs, Volume= 0.075 af, Depth= 1.89"

Routed to Link F5B: Flume 5B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription		
	0.	475 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	(min)         (feet)         (ft/ft)         (ft/sec)           3.5         90         0.2500         0.43           0.8         219         0.0200         4.80         2				
				,	Capacity (cfs)	Description
-	3.5	90	0.2500	0.43		Sheet Flow,
		0.40		4.00	00.00	Grass: Short n= 0.150 P2= 2.77"
	0.8	219	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding
	0.2	116	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
	4.5	425	Total			

## **Summary for Subcatchment Area 54: Area 54**

Runoff = 2.98 cfs @ 12.12 hrs, Volume= 0.160 af, Depth= 1.18" Routed to Reach S2.3 : Swale S2.3

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Area (ac) CN Description						
						Fair, HSG B Good, HSG A
-	1.		9 Wei	ghted Aver .00% Pervi	age	G000, FISG A
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
•	3.1	77	0.2500	0.41	(===)	Sheet Flow,
	0.8	237	0.0200	4.80	23.38	Grass: Short n= 0.150 P2= 2.77" <b>Trap/Vee/Rect Channel Flow, Existing Diversion Berm</b> Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
	0.3	70	0.2500	3.50		n= 0.030 Earth, grassed & winding  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps
•	4.2	384	Total		_	

#### **Summary for Subcatchment Area 55: Area 55**

Runoff = 1.45 cfs @ 12.11 hrs, Volume= 0.077 af, Depth= 1.12"

Routed to Reach S2.2 : Swale S2.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Des	cription				
	0.	520	69 Past	ture/grassl	and/range,	Fair, HSG B		
Area (ac) CN Description  0.520 69 Pasture/grassland/range, 0.306 39 Pasture/grassland/range, 0.826 58 Weighted Average 0.826 100.00% Pervious Area  Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs) 3.8 100 0.2500 0.43						Good, HSG A		
	Tc	Length		,	. ,	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.8	100	0.2500	0.43		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.1	26	0.2500	3.50		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	3.9	126	Total					

### **Summary for Subcatchment Area 56: Area 56**

Runoff = 3.66 cfs @ 12.13 hrs, Volume= 0.194 af, Depth= 1.89"

Routed to Link F4B: Flume 4B

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	Area	(ac) C	N Des	cription		
	1.	228 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	228	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	100	0.2500	0.43		Sheet Flow,
	0.0	6	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Posture, Kyr. 7.0 fps
	1.9	541	0.0200	4.80	23.38	Short Grass Pasture Kv= 7.0 fps  Trap/Vee/Rect Channel Flow, Existing Diversion Berm  Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
-	5.7	647	Total			n= 0.030 Earth, grassed & winding

#### **Summary for Subcatchment Area 57: Area 57**

Runoff = 1.86 cfs @ 12.12 hrs, Volume= 0.102 af, Depth= 1.12" Routed to Reach S4.1 : Swale S4.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription				
	0.	526 3	Good, HSG A					
	Fair, HSG B							
0.158 96 Gravel surface, HSG A								
				ghted Aver				
	1.	089	100.	00% Pervi	ous Area			
	-		01		0 "	D 18		
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.5	80	0.2000	0.38		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.77"		
	0.2	20	0.0500	1.34		Sheet Flow,		
						Smooth surfaces n= 0.011 P2= 2.77"		
	0.1	10	0.0050	1.44		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	0.5	48	0.0625	1.75		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	4.3	158	Total					

#### **Summary for Subcatchment Area 58: Area 58**

Runoff = 2.17 cfs @ 12.12 hrs, Volume= 0.118 af, Depth= 1.18" Routed to Reach S4.2 : Swale S4.2

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	Area	(ac) C	N Des	cription		
	0.	580				Good, HSG A
	0.	433	39 Past	ure/grassl	and/range,	Fair, HSG B
_	0.	181	96 Grav	/el surface	, HSG B	
	1.	194	59 Weig	ghted Aver	age	
	1.	194	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.6	121	0.2314	3.37		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	4 4	221	Total			<u> </u>

#### **Summary for Subcatchment Area 59: Area 59**

Runoff = 2.69 cfs @ 12.13 hrs, Volume= 0.162 af, Depth= 0.88"

Routed to Reach S4.3: Swale S4.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	۸۳۵۵	(aa) C	N Doo	orintion			
Area (ac) CN Description							
	1.	134	39 Past	ure/grassl	and/range,	Good, HSG A	
	1.	086 6	39 Past	ure/grassl	and/range,	Fair, HSG B	
2.220 54 Weighted Average							
	2.	220		00% Pervi	•		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.8	100	0.2500	0.43		Sheet Flow,	_
						Grass: Short n= 0.150 P2= 2.77"	
	0.8	140	0.1857	3.02		Shallow Concentrated Flow,	
	3.0			0.02		Short Grass Pasture Kv= 7.0 fps	
_	4.6	240	Total				

### Summary for Subcatchment Area 6: Area 6

Runoff = 2.81 cfs @ 12.13 hrs, Volume= 0.148 af, Depth= 1.89"

Routed to Link F6A: Flume 6A

Area (ac)	CN	Description
0.936	69	Pasture/grassland/range, Fair, HSG B
0.936		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.1	100	0.2070	0.40		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	46	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.0	395	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
_						n= 0.030 Earth, grassed & winding
	5.3	541	Total			

#### **Summary for Subcatchment Area 60: Area 60**

Runoff = 4.13 cfs @ 12.13 hrs, Volume= 0.231 af, Depth= 1.12"

Routed to Reach S4.4 : Swale S4.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
	0.	Good, HSG A				
_	1.	537 (	69 Past	ure/grassi	<u>and/range,</u>	Fair, HSG B
	2.	476	58 Weig	ghted Aver	age	
	2.	476		00% Pervi		
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'
	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	163	0.1718	2.90		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	4.7	263	Total			· · ·

### **Summary for Subcatchment Area 61: Area 61**

Runoff = 1.90 cfs @ 12.12 hrs, Volume= 0.115 af, Depth= 0.82"

Routed to Reach S4.5 : Swale S4.5

 Area (ac)	CN	Description					
0.898	39	Pasture/grassland/range, Good, HSG A					
 0.785	69	Pasture/grassland/range, Fair, HSG B					
1.683	53	Weighted Average					
1.683		100.00% Pervious Area					

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Tc	Length	•	,		Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.9	100	0.2345	0.42		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	46	0.2345	3.39		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
4.1	146	Total			

### Summary for Subcatchment Area 62: Area 62

Runoff = 0.24 cfs @ 12.55 hrs, Volume= 0.055 af, Depth= 0.33"

Routed to Reach S4.6: Swale S4.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	(ac) C	N Des	cription		
1.871 39 Pasture/grassland/range, G					Good, HSG A
0.	000	96 Grav	el surface	, HSG A	
0.	130	96 Grav	el surface	, HSG A	
2.	001 4	13 Weig	ghted Aver	age	
2.	001	100.	00% Pervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.7	100	0.0074	0.11		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
4.7	169	0.0074	0.60		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.4	49	0.0800	1.98		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
20.8	318	Total			

### **Summary for Subcatchment Area 63: Area 63**

Runoff = 0.09 cfs @ 12.53 hrs, Volume= 0.033 af, Depth= 0.18"

Routed to Reach S2.3 : Swale S2.3

 Area (ac)	CN	Description
2.177	39	Pasture/grassland/range, Good, HSG A
2.177		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	66	0.0303	0.17		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
1.6	34	0.2500	0.35		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	49	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.2	149	Total			·

#### **Summary for Subcatchment Area 64: Area 64**

0.39 cfs @ 12.18 hrs, Volume= 0.032 af, Depth= 0.66" Runoff

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

_	Area	(ac) C	N Desc	cription			
0.479 39 Pasture/grassland/range, Good, HSG A						Good, HSG A	
_	0.	115 9	<u>96 Grav</u>	∕el surface	, HSG A		_
	0.	594 5	0 Weig	ghted Aver	age		
	0.	594	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'	
	7.1	100	0.0544	0.24		Sheet Flow,	_
						Grass: Short n= 0.150 P2= 2.77"	
	0.5	47	0.0544	1.63		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	7.6	147	Total			·	_

### **Summary for Subcatchment Area 65: Area 65**

Runoff 10.73 cfs @ 12.04 hrs, Volume= 0.588 af, Depth= 4.67"

Routed to Pond Sed Pond: Sedimentation Basin

	Area	(ac)	CN	Desc	cription		
	1.	509	98	Wate	er Surface,	HSG A	
	1.509 100.00% Impervious Area						a
	Tc (min)	Lengt		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	0.0	(feet	.)	(11/11)	(II/Sec)	(CIS)	Direct Entry,

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#### **Summary for Subcatchment Area 66: Area 66**

Runoff = 0.22 cfs @ 12.53 hrs, Volume= 0.079 af, Depth= 0.18"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription			
5.227 39 Pasture/grassland/range, Good, HSG A							
	5.	227	100.	00% Pervi	ous Area		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	4.5	75	0.0933	0.28		Sheet Flow,	
	4.0	0.5	0.0500	0.00		Grass: Short n= 0.150 P2= 2.77"	
	1.3	25	0.2500	0.33		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"	
	0.0	10	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	0.8	381	0.0265	7.85	109.92	Trap/Vee/Rect Channel Flow, Bot.W=0.00' D=2.00' Z= 4.0 & 3.0 '/' Top.W=14.00'	
	0.8	162	0.2500	3.50		n= 0.030 Earth, grassed & winding  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps	
_	0.5	48	0.0500	1.57		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
_	7.9	701	Total				

### **Summary for Subcatchment Area 67: Area 67**

Runoff = 0.61 cfs @ 12.73 hrs, Volume= 0.128 af, Depth= 0.51"

Routed to Reach RD1: Roadside Ditch 1

 Area (ac)	CN	Description
2.616	39	Pasture/grassland/range, Good, HSG A
0.039	69	Pasture/grassland/range, Fair, HSG B
 0.380	96	Gravel surface, HSG A
3.035	47	Weighted Average
3.035		100.00% Pervious Area

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		Length		,		Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.2	100	0.0068	0.10		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	22.7	786	0.0068	0.58		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	38.9	886	Total			

### **Summary for Subcatchment Area 68: Area 68**

Runoff = 0.03 cfs @ 12.29 hrs, Volume= 0.006 af, Depth= 0.29"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Area	a (ac)	CN	Desc	Description							
(	).227	39	Past	Pasture/grassland/range, Good, HSG A							
(	0.024	69 Pasture/grassland/range, Fair, HSG B									
(	0.251 42 Weighted Average										
(	0.251 100.00% Pervious Area										
To	J		Slope	Velocity	Capacity	Description					
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
3.1	Ę	52 (	0.1154	0.28		Sheet Flow,					
						Grass: Short	n= 0.150	P2= 2.77"			

#### **Summary for Subcatchment Area 69: Area 69**

Runoff = 0.36 cfs @ 12.14 hrs, Volume= 0.035 af, Depth= 0.46"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) (	CN	Desc	cription			
	0.	695	39	Past	ure/grassla	and/range,	Good, HSG A	
0.218 69 Pasture/grassland/range, Fair, HSG B								
0.913 46 Weighted Average								
0.913 100.00% Pervious Area								
	Tc	Length	n SI	ope	Velocity	Capacity	Description	
	(min)	(feet)	) (1	ft/ft)	(ft/sec)	(cfs)		
	3.5	86	0.2	326	0.41		Sheet Flow,	
							O O	0.450 DO 0.778

Grass: Short n= 0.150 P2= 2.77"

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## **Summary for Subcatchment Area 7: Area 7**

Runoff = 2.87 cfs @ 12.14 hrs, Volume= 0.155 af, Depth= 1.89"

Routed to Link F4A: Flume 4A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Des	cription		
0.986 69 Pasture/grassland/range, Fair, HSG B						
	0.	986	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	2.2	31	0.1000	0.24	,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	2.8	69	0.2500	0.40		Sheet Flow,
	0.0	9	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	1.5	419	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
	0.1	80	0.2500	12.26	441.43	n= 0.030 Earth, grassed & winding  Trap/Vee/Rect Channel Flow, Riprap Flume  Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'  n= 0.078 Riprap, 12-inch
	6.6	608	Total			

# **Summary for Subcatchment Area 70: Area 70**

Runoff = 1.52 cfs @ 12.12 hrs, Volume= 0.099 af, Depth= 0.71"

Routed to Reach RD4: Roadside Ditch 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) (	CN Des	cription			
1.016 39 Pasture/grassland/range, Good, HSG A					Good, HSG A		
	0.	620	69 Past	ure/grassl	and/range,	Fair, HSG B	
	0.	035	96 Grav	el surface/	, HSG A		
	1.	671	51 Weig	ghted Aver	age		
	1.	671	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.8	100	0.2500	0.43		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.2	26	0.1538	2.75		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	4.0	126	Total			·	

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## **Summary for Subcatchment Area 71: Area 71**

Runoff = 0.99 cfs @ 12.52 hrs, Volume=

0.238 af, Depth= 0.29"

Routed to Pond N: North Infiltration Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

	Area	(ac) C	N Desc	cription		
	9.	360 3	39 Past	ure/grassl	and/range,	Good, HSG A
	0.	515 9	96 Grav	el surface	, HSG A	
9.875 42 Weighted Average 9.875 100.00% Pervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	10.5	100	0.0200	0.16	` '	Sheet Flow,
	1.7	100	0.0200	0.99		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	4.4	1,137	0.0193	4.31	32.30	Trap/Vee/Rect Channel Flow, Roadside Ditch Bot.W=0.00' D=1.00' Z= 5.0 & 10.0 '/' Top.W=15.00' n= 0.030
-	16.6	1,337	Total			

## **Summary for Subcatchment Area 8: Area 8**

Runoff = 2.66 cfs @ 12.10 hrs, Volume= 0.129 af, Depth= 1.89"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Α	rea (sf)	CN D	escription		
	35,545	69 P	asture/gra	ssland/ran	ge, Fair, HSG B
	35,545	1	00.00% Pe	ervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	59	0.1695	0.33		Sheet Flow,
0.7	289	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
3.6	348	Total			<del>-</del>

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### Summary for Subcatchment Area 9: Area 9

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 0.039 af, Depth= 1.89"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

Д	rea (sf)	CN E	Description		
	10,716	69 F	Pasture/gra	ssland/ran	ge, Fair, HSG B
	10,716	1	00.00% Pe	ervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.1	48	0.2500	0.38	, ,	Sheet Flow,
0.2	96	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
2.3	144	Total			

#### Summary for Reach RD1: Roadside Ditch 1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 0.51" for 25-yr, 24-hr event

Inflow = 0.61 cfs @ 12.73 hrs, Volume= 0.128 af

Outflow = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af, Atten= 2%, Lag= 9.5 min

Routed to Link C1: Culvert C1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.34 fps, Min. Travel Time= 5.5 min Avg. Velocity = 0.62 fps, Avg. Travel Time= 11.8 min

Peak Storage= 197 cf @ 12.80 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 5.08' Bank-Full Depth= 1.00' Flow Area= 9.5 sf, Capacity= 47.16 cfs

4.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 3.0 8.0 '/' Top Width= 15.00'

Length= 440.6' Slope= 0.0188 '/'

Inlet Invert= 824.00', Outlet Invert= 815.70'



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## Summary for Reach RD2: Roadside Ditch 2

Inflow Area = 7.576 ac, 0.00% Impervious, Inflow Depth = 1.29" for 25-yr, 24-hr event

Inflow = 11.23 cfs @ 12.20 hrs, Volume= 0.814 af

Outflow = 10.38 cfs @ 12.28 hrs, Volume= 0.814 af, Atten= 8%, Lag= 4.8 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.69 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.79 fps, Avg. Travel Time= 9.1 min

Peak Storage= 1,686 cf @ 12.24 hrs

Average Depth at Peak Storage= 0.39', Surface Width= 13.85' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 72.77 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 433.0' Slope= 0.0162 '/'

Inlet Invert= 814.55', Outlet Invert= 807.54'



# Summary for Reach RD3: Roadside Ditch 3

Inflow Area = 2.159 ac, 0.00% Impervious, Inflow Depth = 2.29" for 25-yr, 24-hr event

Inflow = 8.44 cfs @ 12.10 hrs, Volume= 0.412 af

Outflow = 7.09 cfs @ 12.20 hrs, Volume= 0.412 af, Atten= 16%, Lag= 5.7 min

Routed to Link C2: Culvert C2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.99 fps, Min. Travel Time= 3.4 min

Avg. Velocity = 1.34 fps, Avg. Travel Time= 10.2 min

Peak Storage= 1,491 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.67', Surface Width= 5.39'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 20.76 cfs

0.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

Length= 821.0' Slope= 0.0288 '/'

Inlet Invert= 841.47', Outlet Invert= 817.83'

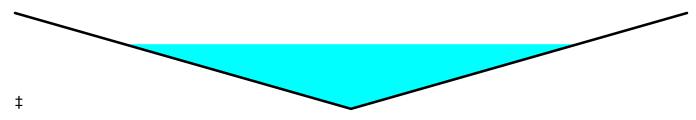
MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91"

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#### Summary for Reach RD4: Roadside Ditch 4

Inflow Area = 1.671 ac, 0.00% Impervious, Inflow Depth = 0.71" for 25-yr, 24-hr event

Inflow = 1.52 cfs @ 12.12 hrs, Volume= 0.099 af

Outflow = 0.92 cfs @ 12.35 hrs, Volume= 0.099 af, Atten= 39%, Lag= 13.6 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.05 fps, Min. Travel Time= 7.9 min

Avg. Velocity = 0.37 fps, Avg. Travel Time= 22.6 min

Peak Storage= 447 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.12', Surface Width= 8.49' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 54.26 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 495.6' Slope= 0.0090 '/'

Inlet Invert= 812.00', Outlet Invert= 807.54'



# Summary for Reach RD5: Roadside Ditch 5

Inflow Area = 0.616 ac, 0.00% Impervious, Inflow Depth = 2.13" for 25-yr, 24-hr event

Inflow = 2.25 cfs @ 12.10 hrs, Volume= 0.109 af

Outflow = 2.01 cfs @ 12.14 hrs, Volume= 0.109 af, Atten= 11%, Lag= 2.5 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.65 fps, Min. Travel Time= 1.3 min

Avg. Velocity = 1.38 fps, Avg. Travel Time= 3.5 min

Peak Storage= 164 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.38', Surface Width= 3.02'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 28.18 cfs

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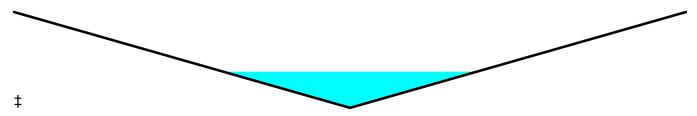
MSE 24-hr 4 25-yr, 24-hr Rainfall=4.91" Printed 8/28/2023

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0.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 8.00' Length= 288.0' Slope= 0.0531 '/' Inlet Invert= 841.47', Outlet Invert= 826.18'



#### **Summary for Reach S1.1: Swale S1.1**

Inflow Area = 1.185 ac, 0.00% Impervious, Inflow Depth = 1.59" for 25-yr, 24-hr event

Inflow = 3.12 cfs @ 12.11 hrs, Volume= 0.157 af

Outflow = 2.76 cfs @ 12.18 hrs, Volume= 0.157 af, Atten= 12%, Lag= 4.2 min

Routed to Link C10: Culvert C10

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.32 fps, Min. Travel Time= 2.3 min

Avg. Velocity = 0.74 fps, Avg. Travel Time= 7.3 min

Peak Storage= 398 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.14', Surface Width= 9.16' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 338.34 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 321.0' Slope= 0.0319 '/'

Inlet Invert= 825.20', Outlet Invert= 814.96'



## Summary for Reach S1.2: Swale S1.2

Inflow Area = 11.287 ac, 0.00% Impervious, Inflow Depth = 1.80" for 25-yr, 24-hr event

Inflow = 30.31 cfs @ 12.14 hrs, Volume= 1.696 af

Outflow = 28.58 cfs @ 12.17 hrs, Volume= 1.696 af, Atten= 6%, Lag= 1.5 min

Routed to Link C3: Culvert C3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.60 fps, Min. Travel Time= 0.9 min

Avg. Velocity = 1.00 fps, Avg. Travel Time= 3.4 min

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Peak Storage= 1,690 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.76', Surface Width= 14.05' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 196.86 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 202.8' Slope= 0.0108 '/'

Inlet Invert= 813.36', Outlet Invert= 811.17'



## **Summary for Reach S1.3: Swale S1.3**

Inflow Area = 14.199 ac, 0.00% Impervious, Inflow Depth = 1.76" for 25-yr, 24-hr event

Inflow = 34.68 cfs @ 12.16 hrs, Volume= 2.079 af

Outflow = 34.14 cfs @ 12.16 hrs, Volume= 2.079 af, Atten= 2%, Lag= 0.4 min

Routed to Link C4: Culvert C4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.15 fps, Min. Travel Time= 0.3 min Avg. Velocity = 1.18 fps, Avg. Travel Time= 1.0 min

Peak Storage= 602 cf @ 12.16 hrs Average Depth at Peak Storage= 0.76', Surface Width= 14.05' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 227.36 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 72.2' Slope= 0.0144 '/'

Inlet Invert= 810.90', Outlet Invert= 809.86'



# Summary for Reach S1.4: Swale S1.4

Inflow Area = 14.938 ac, 0.00% Impervious, Inflow Depth = 1.72" for 25-yr, 24-hr event

Inflow = 35.12 cfs @ 12.16 hrs, Volume= 2.141 af

Outflow = 33.65 cfs @ 12.19 hrs, Volume= 2.141 af, Atten= 4%, Lag= 1.8 min

Routed to Reach S1.6: Swale S1.6

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.73 fps, Min. Travel Time= 0.9 min

Avg. Velocity = 0.80 fps, Avg. Travel Time= 3.1 min

Peak Storage= 1,854 cf @ 12.18 hrs

Average Depth at Peak Storage= 1.03', Surface Width= 16.27'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 126.50 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 148.0' Slope= 0.0045 '/'

Inlet Invert= 809.60', Outlet Invert= 808.94'



#### **Summary for Reach S1.5: Swale S1.5**

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 2.14 cfs @ 12.15 hrs, Volume= 0.121 af

Outflow = 1.95 cfs @ 12.23 hrs, Volume= 0.121 af, Atten= 9%, Lag= 4.7 min

Routed to Reach S1.6: Swale S1.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.12 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.32 fps, Avg. Travel Time= 9.2 min

Peak Storage= 316 cf @ 12.19 hrs

Average Depth at Peak Storage= 0.20', Surface Width= 9.60'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 134.06 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 179.7' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.95'



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## Summary for Reach S1.6: Swale S1.6

Inflow Area = 17.290 ac, 0.00% Impervious, Inflow Depth = 1.73" for 25-yr, 24-hr event

Inflow 39.20 cfs @ 12.19 hrs, Volume= 2.499 af

37.41 cfs @ 12.24 hrs, Volume= Outflow 2.499 af, Atten= 5%, Lag= 2.6 min

Routed to Link C5: Culvert C5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.05 fps, Min. Travel Time= 1.4 min

Avg. Velocity = 0.86 fps, Avg. Travel Time= 4.9 min

Peak Storage= 3,198 cf @ 12.21 hrs

Average Depth at Peak Storage= 1.04', Surface Width= 16.34'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 139.68 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 252.0' Slope= 0.0054 '/'

Inlet Invert= 808.94', Outlet Invert= 807.57'



# Summary for Reach S1.7: Swale S1.7

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 2.05" for 25-yr, 24-hr event

2.19 cfs @ 12.15 hrs, Volume= 0.122 af Inflow

Outflow 1.99 cfs @ 12.21 hrs, Volume= 0.122 af, Atten= 9%, Lag= 3.8 min

Routed to Reach S1.6: Swale S1.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.88 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.56 fps, Avg. Travel Time= 7.3 min

Peak Storage= 265 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.31', Surface Width= 4.38'

Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 90.14 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 245.8' Slope= 0.0099 '/'

Inlet Invert= 810.00', Outlet Invert= 807.57'

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#### Summary for Reach S1.8: Swale S1.8

Inflow Area = 20.082 ac, 0.00% Impervious, Inflow Depth = 1.68" for 25-yr, 24-hr event

Inflow = 41.27 cfs @ 12.24 hrs, Volume= 2.805 af

Outflow = 39.57 cfs @ 12.30 hrs, Volume= 2.805 af, Atten= 4%, Lag= 3.7 min

Routed to Reach S2.1: Swale S2.1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.45 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.95 fps, Avg. Travel Time= 7.4 min

Peak Storage= 4,932 cf @ 12.26 hrs

Average Depth at Peak Storage= 0.98', Surface Width= 15.84' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 163.67 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 422.0' Slope= 0.0075 '/'

‡

Inlet Invert= 807.15', Outlet Invert= 804.00'



## Summary for Reach S2.1: Swale S2.1

Inflow Area = 22.196 ac, 0.00% Impervious, Inflow Depth = 1.53" for 25-yr, 24-hr event

Inflow = 39.58 cfs @ 12.30 hrs, Volume= 2.837 af

Outflow = 38.03 cfs @ 12.36 hrs, Volume= 2.837 af, Atten= 4%, Lag= 3.7 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.17 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.97 fps, Avg. Travel Time= 6.7 min

Peak Storage= 4,707 cf @ 12.33 hrs

Average Depth at Peak Storage= 1.31', Surface Width= 14.48'

Bank-Full Depth= 2.00' Flow Area= 24.0 sf, Capacity= 97.05 cfs

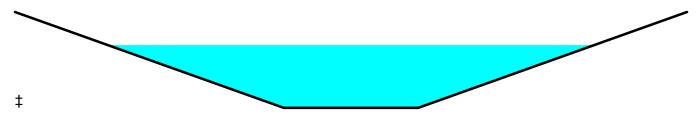
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4.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 20.00' Length= 389.0' Slope= 0.0054 '/' Inlet Invert= 806.10', Outlet Invert= 804.00'



### Summary for Reach S2.2: Swale S2.2

Inflow Area = 39.450 ac, 0.00% Impervious, Inflow Depth = 1.69" for 25-yr, 24-hr event

Inflow = 65.64 cfs @ 12.31 hrs, Volume= 5.558 af

Outflow = 64.61 cfs @ 12.37 hrs, Volume= 5.558 af, Atten= 2%, Lag= 3.6 min

Routed to Reach S2.3: Swale S2.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 3.33 fps, Min. Travel Time= 2.1 min

Avg. Velocity = 0.89 fps, Avg. Travel Time= 7.7 min

Peak Storage= 8,029 cf @ 12.33 hrs

Average Depth at Peak Storage= 1.29', Surface Width= 20.31' Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 152.61 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 411.0' Slope= 0.0049 '/' Inlet Invert= 804.00', Outlet Invert= 802.00'



### Summary for Reach S2.3: Swale S2.3

Inflow Area = 43.245 ac, 0.00% Impervious, Inflow Depth = 1.60" for 25-yr, 24-hr event

Inflow = 65.54 cfs @ 12.37 hrs, Volume= 5.750 af

Outflow = 64.79 cfs @ 12.40 hrs, Volume= 5.750 af, Atten= 1%, Lag= 1.9 min

Routed to Link C7: Culvert C7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 4.71 fps, Min. Travel Time= 1.1 min

Avg. Velocity = 1.26 fps, Avg. Travel Time= 4.0 min

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Peak Storage= 4,252 cf @ 12.38 hrs

Average Depth at Peak Storage= 0.99', Surface Width= 17.93'

Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 249.72 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 307.0' Slope= 0.0130 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



## Summary for Reach S3.1: Swale S3.1

Inflow Area = 2.044 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 6.52 cfs @ 12.11 hrs, Volume= 0.322 af

Outflow = 5.46 cfs @ 12.21 hrs, Volume= 0.322 af, Atten= 16%, Lag= 5.9 min

Routed to Reach S3.2: Swale S3.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.62 fps, Min. Travel Time= 3.7 min Avg. Velocity = 0.42 fps, Avg. Travel Time= 14.2 min

Peak Storage= 1,242 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.37', Surface Width= 10.94'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.76 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 357.0' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.07'



### Summary for Reach S3.2: Swale S3.2

Inflow Area = 8.141 ac, 0.00% Impervious, Inflow Depth = 1.90" for 25-yr, 24-hr event

Inflow = 22.25 cfs @ 12.12 hrs, Volume= 1.287 af

Outflow = 21.76 cfs @ 12.13 hrs, Volume= 1.287 af, Atten= 2%, Lag= 0.6 min

Routed to Reach S3.3: Swale S3.3

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.48 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.66 fps, Avg. Travel Time= 0.9 min

Peak Storage= 299 cf @ 12.13 hrs

Average Depth at Peak Storage= 0.79', Surface Width= 14.30'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.95 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 34.0' Slope= 0.0050 '/'

Inlet Invert= 798.00', Outlet Invert= 797.83'



#### Summary for Reach S3.3: Swale S3.3

Inflow Area = 8.141 ac, 0.00% Impervious, Inflow Depth = 1.90" for 25-yr, 24-hr event

Inflow = 21.76 cfs @ 12.13 hrs, Volume= 1.287 af

Outflow = 20.93 cfs @ 12.16 hrs, Volume= 1.287 af, Atten= 4%, Lag= 1.7 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.48 fps, Min. Travel Time= 1.0 min

Avg. Velocity = 0.91 fps, Avg. Travel Time= 3.7 min

Peak Storage= 1,253 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.60', Surface Width= 12.82'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 215.99 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 200.0' Slope= 0.0130 '/'

Inlet Invert= 808.00', Outlet Invert= 805.40'



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### Summary for Reach S3.4: Swale S3.4

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 2.14 cfs @ 12.15 hrs, Volume= 0.121 af

Outflow = 1.94 cfs @ 12.24 hrs, Volume= 0.121 af, Atten= 9%, Lag= 5.0 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.67 fps, Min. Travel Time= 2.8 min

Avg. Velocity = 0.50 fps, Avg. Travel Time= 9.5 min

Peak Storage= 332 cf @ 12.19 hrs

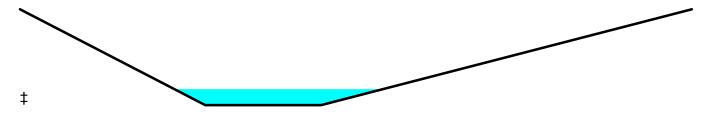
Average Depth at Peak Storage= 0.33', Surface Width= 4.51' Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 76.21 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 283.0' Slope= 0.0071 '/'

Inlet Invert= 810.00', Outlet Invert= 808.00'



# Summary for Reach S3.5: Swale S3.5

Inflow Area = 15.716 ac, 0.00% Impervious, Inflow Depth = 1.93" for 25-yr, 24-hr event

Inflow = 39.57 cfs @ 12.15 hrs, Volume= 2.531 af

Outflow = 36.85 cfs @ 12.23 hrs, Volume= 2.531 af, Atten= 7%, Lag= 4.3 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.26 fps, Min. Travel Time= 2.3 min

Avg. Velocity = 0.60 fps, Avg. Travel Time= 8.9 min

Peak Storage= 5,280 cf @ 12.19 hrs

Average Depth at Peak Storage= 1.27', Surface Width= 18.15'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 93.14 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 318.5' Slope= 0.0024 '/'

Inlet Invert= 804.76', Outlet Invert= 803.99'

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#### Summary for Reach S4.1: Swale S4.1

Inflow Area = 1.364 ac, 0.00% Impervious, Inflow Depth = 1.28" for 25-yr, 24-hr event

Inflow = 2.74 cfs @ 12.10 hrs, Volume= 0.145 af

Outflow = 2.38 cfs @ 12.18 hrs, Volume= 0.145 af, Atten= 13%, Lag= 4.4 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.62 fps, Min. Travel Time= 2.5 min

Avg. Velocity = 0.43 fps, Avg. Travel Time= 9.3 min

Peak Storage= 363 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.14', Surface Width= 11.01' Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 70.22 cfs

10.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00'

Length= 240.0' Slope= 0.0153 '/'

Inlet Invert= 811.94', Outlet Invert= 808.26'



# Summary for Reach S4.2: Swale S4.2

Inflow Area = 3.174 ac, 0.00% Impervious, Inflow Depth = 1.41" for 25-yr, 24-hr event

Inflow = 6.29 cfs @ 12.15 hrs, Volume= 0.372 af

Outflow = 5.84 cfs @ 12.21 hrs, Volume= 0.372 af, Atten= 7%, Lag= 3.6 min

Routed to Reach S4.3: Swale S4.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.12 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.53 fps, Avg. Travel Time= 8.1 min

Peak Storage= 732 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.26', Surface Width= 11.81'

Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 63.88 cfs

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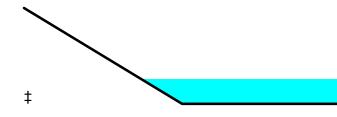
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10.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00' Length= 259.3' Slope= 0.0127 '/' Inlet Invert= 808.26', Outlet Invert= 804.97'



#### **Summary for Reach S4.3: Swale S4.3**

Inflow Area = 9.483 ac, 0.00% Impervious, Inflow Depth = 1.49" for 25-yr, 24-hr event

Inflow = 19.18 cfs @ 12.14 hrs, Volume= 1.179 af

Outflow = 17.09 cfs @ 12.24 hrs, Volume= 1.179 af, Atten= 11%, Lag= 5.7 min

Routed to Reach S4.4: Swale S4.4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 1.83 fps, Min. Travel Time= 3.3 min

Avg. Velocity = 0.46 fps, Avg. Travel Time= 13.1 min

Peak Storage= 3,406 cf @ 12.18 hrs

Average Depth at Peak Storage= 0.74', Surface Width= 15.21' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 108.12 cfs

 $10.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 362.9' Slope= 0.0027 '/'

Inlet Invert= 804.97', Outlet Invert= 804.00'



# Summary for Reach S4.4: Swale S4.4

Inflow Area = 14.158 ac, 0.00% Impervious, Inflow Depth = 1.49" for 25-yr, 24-hr event

Inflow = 22.46 cfs @ 12.20 hrs, Volume= 1.757 af

Outflow = 21.52 cfs @ 12.31 hrs, Volume= 1.757 af, Atten= 4%, Lag= 6.4 min

Routed to Reach S4.5: Swale S4.5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.28 fps, Min. Travel Time= 3.6 min Avg. Velocity = 0.58 fps, Avg. Travel Time= 14.2 min

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Peak Storage= 4,726 cf @ 12.25 hrs

Average Depth at Peak Storage= 0.75', Surface Width= 15.28' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 132.85 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 495.6' Slope= 0.0040 '/'

Inlet Invert= 804.00', Outlet Invert= 802.00'



#### Summary for Reach S4.5: Swale S4.5

Inflow Area = 16.268 ac, 0.00% Impervious, Inflow Depth = 1.43" for 25-yr, 24-hr event

Inflow = 22.64 cfs @ 12.30 hrs, Volume= 1.939 af

Outflow = 22.15 cfs @ 12.37 hrs, Volume= 1.939 af, Atten= 2%, Lag= 3.9 min

Routed to Reach S4.6: Swale S4.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.08 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.85 fps, Avg. Travel Time= 8.0 min

Peak Storage= 2,967 cf @ 12.33 hrs

Average Depth at Peak Storage= 0.60', Surface Width= 14.18'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf. Capacity= 465.89 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 411.1' Slope= 0.0097 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



# Summary for Reach S4.6: Swale S4.6

Inflow Area = 64.606 ac, 0.00% Impervious, Inflow Depth = 1.53" for 25-yr, 24-hr event

Inflow = 89.26 cfs @ 12.39 hrs, Volume= 8.232 af

Outflow = 87.87 cfs @ 12.44 hrs, Volume= 8.232 af, Atten= 2%, Lag= 3.2 min

Routed to Pond Sed Pond : Sedimentation Basin

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.00 fps, Min. Travel Time= 1.8 min

Avg. Velocity = 1.37 fps, Avg. Travel Time= 6.5 min

Peak Storage= 9,534 cf @ 12.41 hrs

Average Depth at Peak Storage= 1.24', Surface Width= 18.67'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf, Capacity= 499.25 cfs

 $10.00' \times 3.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 537.0' Slope= 0.0112 '/'

Inlet Invert= 798.00', Outlet Invert= 792.00'



#### **Summary for Reach S5.1: Swale S5.1**

Inflow Area = 1.218 ac, 0.00% Impervious, Inflow Depth = 2.29" for 25-yr, 24-hr event

Inflow = 4.58 cfs @ 12.11 hrs, Volume= 0.233 af

Outflow = 3.89 cfs @ 12.21 hrs, Volume= 0.233 af, Atten= 15%, Lag= 6.0 min

Routed to Link C2 : Culvert C2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.09 fps, Min. Travel Time= 3.8 min

Avg. Velocity = 0.57 fps, Avg. Travel Time= 14.0 min

Peak Storage= 926 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.22', Surface Width= 9.75'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 235.22 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 478.0' Slope= 0.0154 '/'

Inlet Invert= 825.20', Outlet Invert= 817.83'



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## **Summary for Pond C8: Culvert C8**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af

Outflow = 0.96 cfs @ 12.08 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.1 min

Primary = 0.96 cfs @ 12.08 hrs, Volume= 0.043 af

Routed to Reach S4.1: Swale S4.1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 811.21' @ 12.08 hrs Surf.Area= 0.000 ac Storage= 0.000 af Flood Elev= 819.00' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= 0.2 min calculated for 0.043 af (100% of inflow) Center-of-Mass det. time= 0.2 min (830.2 - 830.0)

Volume Invert Avail.Storage Storage Description
#1 810.70' 0.001 af 3.00'D x 7.00'H Vertical Cone/Cylinder

Device Routing Invert Outlet Devices

#1 Primary 810.70' **12.0" Round Culvert** 

L= 85.6' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 810.70' / 808.60' S= 0.0245 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.89 cfs @ 12.08 hrs HW=811.18' (Free Discharge) 1=Culvert (Inlet Controls 0.89 cfs @ 2.37 fps)

## **Summary for Pond N: North Infiltration Area**

Inflow Area = 19.122 ac, 0.00% Impervious, Inflow Depth = 0.72" for 25-yr, 24-hr event

Inflow = 11.66 cfs @ 12.29 hrs, Volume= 1.151 af

Outflow = 1.38 cfs @ 13.97 hrs, Volume= 1.151 af, Atten= 88%, Lag= 100.5 min

Primary = 1.38 cfs @ 13.97 hrs, Volume= 1.151 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 803.86' @ 13.97 hrs Surf.Area= 16,535 sf Storage= 20,107 cf

Plug-Flow detention time= 176.3 min calculated for 1.150 af (100% of inflow)

Center-of-Mass det. time= 176.2 min ( 1,062.2 - 886.0 )

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	802.00'	256,569 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
802.00	5,140	0	0
804.00	17,424	22,564	22,564
806.00	32,191	49,615	72,179
808.00	46,130	78,321	150,500
810.00	59.939	106.069	256.569

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Device	Routing	Invert	Outlet Devices
#1	Primary	802.00'	3.600 in/hr Exfiltration over Surface area

Primary OutFlow Max=1.38 cfs @ 13.97 hrs HW=803.86' (Free Discharge) 1=Exfiltration (Exfiltration Controls 1.38 cfs)

### **Summary for Pond Sed Pond: Sedimentation Basin**

Inflow Area =	71.936 ac,	2.10% Impervious, Inflow D	Depth = 1.49" for 25-yr, 24-hr event			
Inflow =	89.59 cfs @	12.44 hrs, Volume=	8.931 af			
Outflow =	15.47 cfs @	13.52 hrs, Volume=	8.931 af, Atten= 83%, Lag= 64.8 min			
Discarded =	5.48 cfs @	13.52 hrs, Volume=	6.100 af			
Primary =	10.00 cfs @	13.52 hrs, Volume=	2.831 af			
Routed to Link	: Wetland : We	etland				
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af			
Routed to Link	: Wetland : We	etland				
Tertiary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af			
Routed to Link Wetland : Wetland						

Routed to Link Wetland : Wetland

#3

#4

#5

Device 1

Device 1

Device 1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 792.00' @ 13.52 hrs Surf.Area= 65,709 sf Storage= 164,114 cf Flood Elev= 794.00' Surf.Area= 75,797 sf Storage= 304,443 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 185.0 min (1,047.1 - 862.1)

790.50'

790.00'

789.00'

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	789.0	0' 304,44	43 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
789.0	00	27,325	0	0	
790.0	00	55,972	41,649	41,649	
791.0	00	61,532	58,752	100,401	
792.0	00	65,703	63,618	164,018	
793.0	00	69,675	67,689	231,707	
794.0	00	75,797	72,736	304,443	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	787.70'	15.0" Round	Culvert	
			L= 40.0' RCF	P, mitered to cor	nform to fill, Ke= 0.700
			Inlet / Outlet In	nvert= 787.70' /	787.50' S= 0.0050 '/' Cc= 0.900
			n= 0.011 Cor	ncrete pipe, strai	ight & clean, Flow Area= 1.23 sf
#2	Device 1	791.00'	30.0" Horiz. (	Orifice/Grate C	C= 0.600
			Limited to wei	r flow at low hea	ads

**0.8" Vert. Orifice/Grate X 4.00** C= 0.600

**0.8" Vert. Orifice/Grate X 4.00** C= 0.600

0.5" Vert. Orifice/Grate X 14.00 columns

X 6 rows with 6.0" cc spacing C= 0.600

Limited to weir flow at low heads

Limited to weir flow at low heads

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			Limited to weir flow at low heads
#6	Secondary	792.50'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	Ť		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#7	Tertiary	703 NN'	158.0' long x 10.0' breadth Broad-Crested Rectangular Weir
,,,,	i Citiai y	133.00	100.0 long x 10.0 bicadin bioad-orested rectangular wen
""	Tortiary	7 33.00	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
,,,	rordary	733.00	

**Discarded OutFlow** Max=5.48 cfs @ 13.52 hrs HW=792.00' (Free Discharge) **8=Exfiltration** (Exfiltration Controls 5.48 cfs)

```
Primary OutFlow Max=10.00 cfs @ 13.52 hrs HW=792.00' (Free Discharge)

1=Culvert (Inlet Controls 10.00 cfs @ 8.15 fps)

2=Orifice/Grate (Passes < 23.65 cfs potential flow)

3=Orifice/Grate (Passes < 0.08 cfs potential flow)

4=Orifice/Grate (Passes < 0.09 cfs potential flow)

5=Orifice/Grate (Passes < 0.70 cfs potential flow)
```

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=789.00' (Free Discharge) 6=Broad-Crested Rectangular Weir( Controls 0.00 cfs)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=789.00' (Free Discharge)

7=Broad-Crested Rectangular Weir( Controls 0.00 cfs)

# Summary for Link C1: Culvert C1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 0.51" for 25-yr, 24-hr event Inflow = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af

Primary = 0.60 cfs @ 12.89 hrs, Volume= 0.128 af, Atten= 0%, Lag= 0.0 min Routed to Reach RD2 : Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C10: Culvert C10

Inflow Area = 1.185 ac, 0.00% Impervious, Inflow Depth = 1.59" for 25-yr, 24-hr event Inflow = 2.76 cfs @ 12.18 hrs, Volume= 0.157 af

Primary = 2.76 cfs @ 12.18 hrs, Volume= 0.157 af, Atten= 0%, Lag= 0.0 min Routed to Reach S1.2 : Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C2: Culvert C2

Inflow Area = 3.377 ac, 0.00% Impervious, Inflow Depth = 2.29" for 25-yr, 24-hr event Inflow = 10.95 cfs @ 12.20 hrs, Volume= 0.645 af

Primary = 10.95 cfs @ 12.20 hrs, Volume= 0.645 af, Atten= 0%, Lag= 0.0 min Routed to Reach RD2 : Roadside Ditch 2

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# **Summary for Link C3: Culvert C3**

Inflow Area = 11.287 ac, 0.00% Impervious, Inflow Depth = 1.80" for 25-yr, 24-hr event

Inflow = 28.58 cfs @ 12.17 hrs, Volume= 1.696 af

Primary = 28.58 cfs @ 12.17 hrs, Volume= 1.696 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link C4: Culvert C4**

Inflow Area = 14.199 ac, 0.00% Impervious, Inflow Depth = 1.76" for 25-yr, 24-hr event

Inflow = 34.14 cfs @ 12.16 hrs, Volume= 2.079 af

Primary = 34.14 cfs @ 12.16 hrs, Volume= 2.079 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### **Summary for Link C5: Culvert C5**

Inflow Area = 17.290 ac, 0.00% Impervious, Inflow Depth = 1.73" for 25-yr, 24-hr event

Inflow = 37.41 cfs @ 12.24 hrs, Volume= 2.499 af

Primary = 37.41 cfs @ 12.24 hrs, Volume= 2.499 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.8: Swale S1.8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C6: Culvert C6

Inflow Area = 11.990 ac, 0.00% Impervious, Inflow Depth = 1.90" for 25-yr, 24-hr event

Inflow = 30.67 cfs @ 12.15 hrs, Volume= 1.895 af

Primary = 30.67 cfs @ 12.15 hrs, Volume= 1.895 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.5: Swale S3.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C7: Culvert C7

Inflow Area = 46.337 ac, 0.00% Impervious, Inflow Depth = 1.62" for 25-yr, 24-hr event

Inflow = 67.17 cfs @ 12.39 hrs, Volume= 6.238 af

Primary = 67.17 cfs @ 12.39 hrs, Volume= 6.238 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.6: Swale S4.6

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# **Summary for Link C9: Culvert C9**

Inflow Area = 9.247 ac, 0.00% Impervious, Inflow Depth = 1.18" for 25-yr, 24-hr event

Inflow = 11.16 cfs @ 12.29 hrs, Volume= 0.913 af

Primary = 11.16 cfs @ 12.29 hrs, Volume= 0.913 af, Atten= 0%, Lag= 0.0 min

Routed to Pond N: North Infiltration Area

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link F10: Flume 10**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af

Primary = 0.95 cfs @ 12.08 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.0 min

Routed to Pond C8: Culvert C8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link F1A: Flume 1A**

Inflow Area = 1.853 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 5.38 cfs @ 12.14 hrs, Volume= 0.292 af

Primary = 5.38 cfs @ 12.14 hrs, Volume= 0.292 af, Atten= 0%, Lag= 0.0 min

Routed to Link F1B: Flume 1B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F1B: Flume 1B

Inflow Area = 4.089 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 12.06 cfs @ 12.13 hrs, Volume= 0.645 af

Primary = 12.06 cfs @ 12.13 hrs, Volume= 0.645 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.3: Swale S4.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F2A: Flume 2A

Inflow Area = 0.636 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 1.98 cfs @ 12.11 hrs, Volume= 0.100 af

Primary = 1.98 cfs @ 12.11 hrs, Volume= 0.100 af, Atten= 0%, Lag= 0.0 min

Routed to Link F2B: Flume 2B

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## Summary for Link F2B: Flume 2B

Inflow Area = 2.199 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 6.58 cfs @ 12.12 hrs, Volume= 0.347 af

Primary = 6.58 cfs @ 12.12 hrs, Volume= 0.347 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.4: Swale S4.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F3: Flume 3

Inflow Area = 0.427 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 1.38 cfs @ 12.10 hrs, Volume= 0.067 af

Primary = 1.38 cfs @ 12.10 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.5: Swale S4.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link F4A: Flume 4A**

Inflow Area = 1.209 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 3.47 cfs @ 12.14 hrs, Volume= 0.191 af

Primary = 3.47 cfs @ 12.14 hrs, Volume= 0.191 af, Atten= 0%, Lag= 0.0 min

Routed to Link F4B: Flume 4B

Primary outflow = Inflow. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs.

# Summary for Link F4B: Flume 4B

Inflow Area = 3.092 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 9.10 cfs @ 12.13 hrs, Volume= 0.488 af

Primary = 9.10 cfs @ 12.13 hrs, Volume= 0.488 af, Atten= 0%, Lag= 0.0 min

Routed to Link C7: Culvert C7

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F5A: Flume 5A

Inflow Area = 0.237 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 0.76 cfs @ 12.11 hrs, Volume= 0.037 af

Primary = 0.76 cfs @ 12.11 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min

Routed to Link F5B: Flume 5B

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## Summary for Link F5B: Flume 5B

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 2.23 cfs @ 12.11 hrs, Volume= 0.112 af

Primary = 2.23 cfs @ 12.11 hrs, Volume= 0.112 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S2.2: Swale S2.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Summary for Link F6A: Flume 6A

Inflow Area = 1.440 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 4.38 cfs @ 12.11 hrs, Volume= 0.227 af

Primary = 4.38 cfs @ 12.11 hrs, Volume= 0.227 af, Atten= 0%, Lag= 0.0 min

Routed to Link F6B: Flume 6B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Summary for Link F6B: Flume 6B

Inflow Area = 3.001 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 9.20 cfs @ 12.11 hrs, Volume= 0.473 af

Primary = 9.20 cfs @ 12.11 hrs, Volume= 0.473 af, Atten= 0%, Lag= 0.0 min

Routed to Link C6: Culvert C6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F7A: Flume 7A**

Inflow Area = 1.062 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 3.51 cfs @ 12.10 hrs, Volume= 0.167 af

Primary = 3.51 cfs @ 12.10 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7B: Flume 7B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F7B: Flume 7B

Inflow Area = 3.407 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 10.65 cfs @ 12.11 hrs, Volume= 0.537 af

Primary = 10.65 cfs @ 12.11 hrs, Volume= 0.537 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7C: Flume 7C

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## **Summary for Link F7C: Flume 7C**

Inflow Area = 5.640 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 17.47 cfs @ 12.11 hrs, Volume= 0.889 af

Primary = 17.47 cfs @ 12.11 hrs, Volume= 0.889 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.2: Swale S3.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## **Summary for Link F8A: Flume 8A**

Inflow Area = 0.381 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 1.19 cfs @ 12.11 hrs, Volume= 0.060 af

Primary = 1.19 cfs @ 12.11 hrs, Volume= 0.060 af, Atten= 0%, Lag= 0.0 min

Routed to Link F8B: Flume 8B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Summary for Link F8B: Flume 8B

Inflow Area = 1.497 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 4.66 cfs @ 12.11 hrs, Volume= 0.236 af

Primary = 4.66 cfs @ 12.11 hrs, Volume= 0.236 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F9A: Flume 9A

Inflow Area = 2.445 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 6.88 cfs @ 12.15 hrs, Volume= 0.386 af

Primary = 6.88 cfs @ 12.15 hrs, Volume= 0.386 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9B: Flume 9B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F9B: Flume 9B

Inflow Area = 5.540 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 15.84 cfs @ 12.14 hrs, Volume= 0.874 af

Primary = 15.84 cfs @ 12.14 hrs, Volume= 0.874 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9C: Flume 9C

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### Summary for Link F9C: Flume 9C

Inflow Area = 8.811 ac, 0.00% Impervious, Inflow Depth = 1.89" for 25-yr, 24-hr event

Inflow = 25.24 cfs @ 12.14 hrs, Volume= 1.389 af

Primary = 25.24 cfs @ 12.14 hrs, Volume= 1.389 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.2: Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### **Summary for Link Wetland: Wetland**

Inflow Area = 71.936 ac, 2.10% Impervious, Inflow Depth = 0.47" for 25-yr, 24-hr event

Inflow = 10.00 cfs @ 13.52 hrs, Volume= 2.831 af

Primary = 10.00 cfs @ 13.52 hrs, Volume= 2.831 af, Atten= 0%, Lag= 0.0 min

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentArea 1: Area 1	Runoff Area=1.296 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=364' Tc=6.7 min CN=69 Runoff=6.34 cfs 0.344 af
SubcatchmentArea 10: Area 10	Runoff Area=0.573 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=321' Tc=6.4 min CN=69 Runoff=2.83 cfs 0.152 af
SubcatchmentArea 11: Area 11	Runoff Area=1.872 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=796' Tc=7.6 min CN=69 Runoff=8.84 cfs 0.496 af
SubcatchmentArea 12: Area 12	Runoff Area=1.610 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=712' Tc=5.3 min CN=69 Runoff=8.11 cfs 0.427 af
SubcatchmentArea 13: Area 13	Runoff Area=0.626 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=333' Tc=4.5 min CN=69 Runoff=3.29 cfs 0.166 af
SubcatchmentArea 14: Area 14	Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=331' Tc=4.4 min CN=69 Runoff=3.27 cfs 0.164 af
SubcatchmentArea 15: Area 15	Runoff Area=0.943 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=464' Tc=5.9 min CN=69 Runoff=4.72 cfs 0.250 af
SubcatchmentArea 16: Area 16	Runoff Area=0.571 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=315' Tc=4.4 min CN=69 Runoff=3.01 cfs 0.151 af
SubcatchmentArea 17: Area 17	Runoff Area=0.990 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=424' Tc=4.7 min CN=69 Runoff=5.14 cfs 0.262 af
SubcatchmentArea 18: Area 18	Runoff Area=1.656 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=508' Tc=4.8 min CN=69 Runoff=8.56 cfs 0.439 af
SubcatchmentArea 19: Area 19	Runoff Area=0.689 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=286' Tc=4.3 min CN=69 Runoff=3.65 cfs 0.183 af
SubcatchmentArea 2: Area 2	Runoff Area=0.557 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=303' Tc=4.7 min CN=69 Runoff=2.89 cfs 0.148 af
SubcatchmentArea 20: Area 20	Runoff Area=0.381 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=306' Tc=4.3 min CN=69 Runoff=2.02 cfs 0.101 af
SubcatchmentArea 21: Area 21	Runoff Area=0.516 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=312' Tc=4.3 min CN=69 Runoff=2.74 cfs 0.137 af
SubcatchmentArea 22: Area 22	Runoff Area=2.579 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=1,146' Tc=6.4 min CN=69 Runoff=12.74 cfs 0.684 af
SubcatchmentArea 23: Area 23	Runoff Area=0.427 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=362' Tc=3.8 min CN=69 Runoff=2.33 cfs 0.113 af

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SubcatchmentArea 39: Area 39

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SubcatchmentArea 24: Area 24	Runoff Area=1.177 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=385' Tc=4.7 min CN=69 Runoff=6.11 cfs 0.312 af
SubcatchmentArea 25: Area 25	Runoff Area=0.682 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=349' Tc=4.5 min CN=69 Runoff=3.58 cfs 0.181 af
SubcatchmentArea 26: Area 26	Runoff Area=0.677 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=341' Tc=4.5 min CN=69 Runoff=3.56 cfs 0.179 af
SubcatchmentArea 27: Area 27	Runoff Area=2.594 ac 0.00% Impervious Runoff Depth=3.18" ow Length=1,357' Tc=6.9 min CN=69 Runoff=12.62 cfs 0.688 af
SubcatchmentArea 28: Area 28 Flow Length=125'	Runoff Area=2.159 ac 0.00% Impervious Runoff Depth=3.69" Slope=0.2500 '/' Tc=3.9 min CN=74 Runoff=13.48 cfs 0.663 af
SubcatchmentArea 29: Area 29 Flow Length=109	Runoff Area=0.616 ac 0.00% Impervious Runoff Depth=3.48" 9' Slope=0.2500 '/' Tc=3.8 min CN=72 Runoff=3.67 cfs 0.179 af
SubcatchmentArea 3: Area 3	Runoff Area=0.348 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=298' Tc=4.8 min CN=69 Runoff=1.80 cfs 0.092 af
SubcatchmentArea 30: Area 30	Runoff Area=0.149 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=211' Tc=1.2 min CN=69 Runoff=0.86 cfs 0.039 af
SubcatchmentArea 31: Area 31 Flow Length=59	Runoff Area=0.126 ac 0.00% Impervious Runoff Depth=3.18" 9' Slope=0.2500 '/' Tc=2.5 min CN=69 Runoff=0.74 cfs 0.033 af
SubcatchmentArea 32: Area 32 Flow Length=122	Runoff Area=0.457 ac 0.00% Impervious Runoff Depth=3.28" 2' Slope=0.2500 '/' Tc=3.9 min CN=70 Runoff=2.55 cfs 0.125 af
SubcatchmentArea 33: Area 33	Runoff Area=1.056 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=419' Tc=4.7 min CN=69 Runoff=5.48 cfs 0.280 af
SubcatchmentArea 34: Area 34	Runoff Area=0.434 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=374' Tc=4.5 min CN=69 Runoff=2.28 cfs 0.115 af
SubcatchmentArea 35: Area 35	Runoff Area=1.218 ac 0.00% Impervious Runoff Depth=3.69" Flow Length=104' Tc=4.6 min CN=74 Runoff=7.33 cfs 0.374 af
SubcatchmentArea 36: Area 36	Runoff Area=1.185 ac 0.00% Impervious Runoff Depth=2.79" Flow Length=106' Tc=4.1 min CN=65 Runoff=5.57 cfs 0.275 af
SubcatchmentArea 37: Area 37 Flow Length=120	Runoff Area=1.291 ac 0.00% Impervious Runoff Depth=2.50" O' Slope=0.2500 '/' Tc=3.9 min CN=62 Runoff=5.48 cfs 0.269 af
SubcatchmentArea 38: Area 38	Runoff Area=0.795 ac 0.00% Impervious Runoff Depth=2.60" Flow Length=155' Tc=4.3 min CN=63 Runoff=3.43 cfs 0.172 af

Runoff Area=0.620 ac 0.00% Impervious Runoff Depth=1.96" Flow Length=168' Tc=9.6 min CN=56 Runoff=1.57 cfs 0.101 af

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SubcatchmentArea 4: Area 4 Runoff Area=0.288 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=296' Tc=4.0 min CN=69 Runoff=1.55 cfs 0.076 af

Runoff Area=0.739 ac 0.00% Impervious Runoff Depth=1.96" SubcatchmentArea 40: Area 40 Flow Length=141' Tc=7.2 min CN=56 Runoff=2.11 cfs 0.120 af

SubcatchmentArea 41a: Area 41a Runoff Area=0.871 ac 0.00% Impervious Runoff Depth=2.79" Flow Length=144' Slope=0.0500 '/' Tc=7.8 min CN=65 Runoff=3.58 cfs 0.202 af

Runoff Area=0.712 ac 0.00% Impervious Runoff Depth=3.38" SubcatchmentArea 41b: Area 41b Flow Length=102' Slope=0.0500 '/' Tc=7.3 min CN=71 Runoff=3.63 cfs 0.201 af

Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=3.18" SubcatchmentArea 42: Area 42 Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=3.63 cfs 0.204 af

Subcatchment Area 43: Area 43 Runoff Area=2.792 ac 0.00% Impervious Runoff Depth=2.41" Flow Length=419' Tc=14.7 min CN=61 Runoff=7.53 cfs 0.561 af

Runoff Area=1.416 ac 0.00% Impervious Runoff Depth=0.63" SubcatchmentArea 44: Area 44 Flow Length=941' Slope=0.0260 '/' Tc=22.0 min CN=39 Runoff=0.43 cfs 0.074 af

Runoff Area=2.044 ac 0.00% Impervious Runoff Depth=3.18" SubcatchmentArea 45: Area 45 Flow Length=138' Tc=4.0 min CN=69 Runoff=11.02 cfs 0.542 af

Runoff Area=0.769 ac 0.00% Impervious Runoff Depth=3.18" SubcatchmentArea 46: Area 46 Flow Length=139' Slope=0.0500 '/' Tc=7.7 min CN=69 Runoff=3.63 cfs 0.204 af

Runoff Area=0.079 ac 0.00% Impervious Runoff Depth=3.48" SubcatchmentArea 47: Area 47 Flow Length=143' Tc=5.1 min CN=72 Runoff=0.44 cfs 0.023 af

Subcatchment Area 48: Area 48 Runoff Area=3.726 ac 0.00% Impervious Runoff Depth=3.38" Flow Length=391' Tc=11.1 min CN=71 Runoff=16.26 cfs 1.050 af

Runoff Area=0.698 ac 0.00% Impervious Runoff Depth=0.63" SubcatchmentArea 49: Area 49 Flow Length=100' Slope=0.0600 '/' Tc=6.8 min CN=39 Runoff=0.32 cfs 0.036 af

Runoff Area=0.504 ac 0.00% Impervious Runoff Depth=3.18" SubcatchmentArea 5: Area 5 Flow Length=258' Tc=3.4 min CN=69 Runoff=2.81 cfs 0.134 af

SubcatchmentArea 50: Area 50 Runoff Area=0.223 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=296' Tc=3.7 min CN=69 Runoff=1.22 cfs 0.059 af

SubcatchmentArea 51: Area 51 Runoff Area=0.655 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=642' Tc=5.0 min CN=69 Runoff=3.35 cfs 0.174 af

Subcatchment Area 52: Area 52 Runoff Area=0.237 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=348' Tc=4.0 min CN=69 Runoff=1.28 cfs 0.063 af

Subcatchment Area 53: Area 53 Runoff Area=0.475 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=425' Tc=4.5 min CN=69 Runoff=2.50 cfs 0.126 af

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SubcatchmentArea 54: Area 54	Runoff Area=1.618 ac 0.00% Impervious Runoff Depth=2.23" Flow Length=384' Tc=4.2 min CN=59 Runoff=5.93 cfs 0.300 af
SubcatchmentArea 55: Area 55 Flow Length=126	Runoff Area=0.826 ac 0.00% Impervious Runoff Depth=2.13" S' Slope=0.2500 '/' Tc=3.9 min CN=58 Runoff=2.95 cfs 0.147 af
SubcatchmentArea 56: Area 56	Runoff Area=1.228 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=647' Tc=5.7 min CN=69 Runoff=6.17 cfs 0.325 af
SubcatchmentArea 57: Area 57	Runoff Area=1.089 ac 0.00% Impervious Runoff Depth=2.13" Flow Length=158' Tc=4.3 min CN=58 Runoff=3.79 cfs 0.194 af
SubcatchmentArea 58: Area 58	Runoff Area=1.194 ac 0.00% Impervious Runoff Depth=2.23" Flow Length=221' Tc=4.4 min CN=59 Runoff=4.33 cfs 0.221 af
Subcatchment Area 59: Area 59	Runoff Area=2.220 ac 0.00% Impervious Runoff Depth=1.78" Flow Length=240' Tc=4.6 min CN=54 Runoff=6.17 cfs 0.330 af
SubcatchmentArea 6: Area 6	Runoff Area=0.936 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=541' Tc=5.3 min CN=69 Runoff=4.72 cfs 0.248 af
SubcatchmentArea 60: Area 60	Runoff Area=2.476 ac 0.00% Impervious Runoff Depth=2.13" Flow Length=263' Tc=4.7 min CN=58 Runoff=8.43 cfs 0.441 af
Subcatchment Area 61: Area 61 Flow Length=146	Runoff Area=1.683 ac 0.00% Impervious Runoff Depth=1.70" S' Slope=0.2345 '/' Tc=4.1 min CN=53 Runoff=4.54 cfs 0.238 af
SubcatchmentArea 62: Area 62	Runoff Area=2.001 ac 0.00% Impervious Runoff Depth=0.90"

Flow Length=318' Tc=20.8 min CN=43 Runoff=1.18 cfs 0.150 af

SubcatchmentArea 63: Area 63 Runoff Area=2.177 ac 0.00% Impervious Runoff Depth=0.63" Flow Length=149' Tc=8.2 min CN=39 Runoff=0.97 cfs 0.114 af

SubcatchmentArea 64: Area 64 Runoff Area=0.594 ac 0.00% Impervious Runoff Depth=1.44" Flow Length=147' Slope=0.0544 '/' Tc=7.6 min CN=50 Runoff=1.14 cfs 0.071 af

SubcatchmentArea 65: Area 65 Runoff Area=1.509 ac 100.00% Impervious Runoff Depth=6.35" Tc=0.0 min CN=98 Runoff=14.43 cfs 0.799 af

SubcatchmentArea 66: Area 66 Runoff Area=5.227 ac 0.00% Impervious Runoff Depth=0.63" Flow Length=701' Tc=7.9 min CN=39 Runoff=2.36 cfs 0.273 af

SubcatchmentArea 67: Area 67 Runoff Area=3.035 ac 0.00% Impervious Runoff Depth=1.20" Flow Length=886' Slope=0.0068 '/' Tc=38.9 min CN=47 Runoff=1.99 cfs 0.304 af

Runoff Area=0.251 ac 0.00% Impervious Runoff Depth=0.83" SubcatchmentArea 68: Area 68 Flow Length=52' Slope=0.1154 '/' Tc=3.1 min CN=42 Runoff=0.26 cfs 0.017 af

SubcatchmentArea 69: Area 69 Runoff Area=0.913 ac 0.00% Impervious Runoff Depth=1.13" Flow Length=86' Slope=0.2326 '/' Tc=3.5 min CN=46 Runoff=1.50 cfs 0.086 af

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Runoff Area=9.875 ac 0.00% Impervious Runoff Depth=0.83" Flow Length=1,337' Tc=16.6 min CN=42 Runoff=5.65 cfs 0.684 af

Runoff Area=35,545 sf 0.00% Impervious Runoff Depth=3.18" Flow Length=348' Tc=3.6 min CN=69 Runoff=4.50 cfs 0.216 af

Runoff Area=10,716 sf 0.00% Impervious Runoff Depth=3.18" Flow Length=144' Tc=2.3 min CN=69 Runoff=1.44 cfs 0.065 af

Avg. Flow Depth=0.19' Max Vel=2.00 fps Inflow=1.99 cfs 0.304 af

Avg. Flow Depth=0.52' Max Vel=3.14 fps Inflow=19.31 cfs 1.445 af

Avg. Flow Depth=0.81' Max Vel=4.50 fps Inflow=13.48 cfs 0.663 af

Avg. Flow Depth=0.24' Max Vel=1.52 fps Inflow=3.98 cfs 0.213 af

Avg. Flow Depth=0.46' Max Vel=4.14 fps Inflow=3.67 cfs 0.179 af

Avg. Flow Depth=0.21' Max Vel=2.87 fps Inflow=5.57 cfs 0.275 af

Avg. Flow Depth=1.02' Max Vel=4.24 fps Inflow=52.19 cfs 2.880 af

Avg. Flow Depth=1.03' Max Vel=4.92 fps Inflow=61.05 cfs 3.550 af

Avg. Flow Depth=1.40' Max Vel=3.24 fps Inflow=62.44 cfs 3.671 af

Avg. Flow Depth=0.27' Max Vel=1.35 fps Inflow=3.63 cfs 0.204 af

Avg. Flow Depth=1.41' Max Vel=3.60 fps Inflow=69.21 cfs 4.277 af

Avg. Flow Depth=0.41' Max Vel=2.20 fps Inflow=3.63 cfs 0.201 af

n=0.030 L=440.6' S=0.0188 '/' Capacity=47.16 cfs Outflow=1.96 cfs 0.304 af

n=0.030 L=433.0' S=0.0162'/' Capacity=72.77 cfs Outflow=17.96 cfs 1.445 af

n=0.030 L=821.0' S=0.0288'/' Capacity=20.76 cfs Outflow=11.77 cfs 0.663 af

n=0.030 L=495.6' S=0.0090 '/' Capacity=54.26 cfs Outflow=2.99 cfs 0.213 af

n=0.030 L=288.0' S=0.0531 '/' Capacity=28.18 cfs Outflow=3.30 cfs 0.179 af

n=0.030 L=321.0' S=0.0319'/' Capacity=338.34 cfs Outflow=4.95 cfs 0.275 af

n=0.030 L=202.8' S=0.0108'/' Capacity=196.86 cfs Outflow=49.91 cfs 2.880 af

n=0.030 L=72.2' S=0.0144 '/' Capacity=227.36 cfs Outflow=60.33 cfs 3.550 af

n=0.030 L=148.0' S=0.0045 '/' Capacity=126.50 cfs Outflow=59.56 cfs 3.671 af

n=0.030 L=179.7' S=0.0050 '/' Capacity=134.06 cfs Outflow=3.33 cfs 0.204 af

n=0.030 L=252.0' S=0.0054 '/' Capacity=139.68 cfs Outflow=66.73 cfs 4.277 af

n=0.030 L=245.8' S=0.0099 '/' Capacity=90.14 cfs Outflow=3.32 cfs 0.201 af

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SubcatchmentArea 7: Area 7	Runoff Area=0.986 ac 0.00% Impervious Runoff Depth=3.18" Flow Length=608' Tc=6.6 min CN=69 Runoff=4.85 cfs 0.261 af
SubcatchmentArea 70: Area 70	Runoff Area=1.671 ac 0.00% Impervious Runoff Depth=1.53" Flow Length=126' Tc=4.0 min CN=51 Runoff=3.98 cfs 0.213 af

SubcatchmentArea 71: Area 71

SubcatchmentArea 8: Area 8

Subcatchment Area 9: Area 9

Reach RD1: Roadside Ditch 1

Reach RD2: Roadside Ditch 2

Reach RD3: Roadside Ditch 3

Reach RD4: Roadside Ditch 4

Reach RD5: Roadside Ditch 5

Reach S1.1: Swale S1.1

Reach S1.2: Swale S1.2

Reach S1.3: Swale S1.3

Reach S1.4: Swale S1.4

Reach S1.5: Swale S1.5

Reach S1.6: Swale S1.6

Reach S1.7: Swale S1.7

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Reach S1.8: Swale S1.8	Avg. Flow Depth=1.33' Max Vel=4.09 fps Inflow=74.10 cfs 4.838 af n=0.030 L=422.0' S=0.0075 '/' Capacity=163.67 cfs Outflow=70.85 cfs 4.838 af
Reach S2.1: Swale S2.1	Avg. Flow Depth=1.73' Max Vel=3.72 fps Inflow=71.34 cfs 4.949 af n=0.030 L=389.0' S=0.0054 '/' Capacity=97.05 cfs Outflow=68.62 cfs 4.949 af
Reach S2.2: Swale S2.2	Avg. Flow Depth=1.77' Max Vel=3.96 fps Inflow=120.78 cfs 9.518 af n=0.030 L=411.0' S=0.0049 '/' Capacity=152.61 cfs Outflow=118.35 cfs 9.518 af
Reach S2.3: Swale S2.3	Avg. Flow Depth=1.38' Max Vel=5.64 fps Inflow=120.84 cfs 9.932 af n=0.030 L=307.0' S=0.0130 '/' Capacity=249.72 cfs Outflow=119.37 cfs 9.932 af
Reach S3.1: Swale S3.1	Avg. Flow Depth=0.50' Max Vel=1.94 fps Inflow=11.02 cfs 0.542 af n=0.030 L=357.0' S=0.0050 '/' Capacity=133.76 cfs Outflow=9.75 cfs 0.542 af
Reach S3.2: Swale S3.2	Avg. Flow Depth=1.06' Max Vel=2.93 fps Inflow=38.61 cfs 2.162 af n=0.030 L=34.0' S=0.0050 '/' Capacity=133.95 cfs Outflow=37.80 cfs 2.162 af
Reach S3.3: Swale S3.3	Avg. Flow Depth=0.82' Max Vel=4.12 fps Inflow=37.80 cfs 2.162 af n=0.030 L=200.0' S=0.0130 '/' Capacity=215.99 cfs Outflow=36.67 cfs 2.162 af
Reach S3.4: Swale S3.4	Avg. Flow Depth=0.45' Max Vel=1.95 fps Inflow=3.63 cfs 0.204 af n=0.030 L=283.0' S=0.0071 '/' Capacity=76.21 cfs Outflow=3.29 cfs 0.204 af
Reach S3.5: Swale S3.5	Avg. Flow Depth=1.68' Max Vel=2.63 fps Inflow=68.29 cfs 4.234 af n=0.030 L=318.5' S=0.0024 '/' Capacity=93.14 cfs Outflow=64.19 cfs 4.234 af
Reach S4.1: Swale S4.1	Avg. Flow Depth=0.22' Max Vel=2.07 fps Inflow=5.28 cfs 0.267 af n=0.030 L=240.0' S=0.0153 '/' Capacity=70.22 cfs Outflow=4.68 cfs 0.267 af
Reach S4.2: Swale S4.2	Avg. Flow Depth=0.38' Max Vel=2.70 fps Inflow=11.92 cfs 0.667 af n=0.030 L=259.3' S=0.0127 '/' Capacity=63.88 cfs Outflow=11.06 cfs 0.667 af
Reach S4.3: Swale S4.3	Avg. Flow Depth=1.08' Max Vel=2.26 fps Inflow=36.14 cfs 2.080 af n=0.030 L=362.9' S=0.0027 '/' Capacity=108.12 cfs Outflow=33.37 cfs 2.080 af
Reach S4.4: Swale S4.4	Avg. Flow Depth=1.11' Max Vel=2.82 fps Inflow=45.34 cfs 3.104 af n=0.030 L=495.6' S=0.0040 '/' Capacity=132.85 cfs Outflow=43.47 cfs 3.104 af
Reach S4.5: Swale S4.5	Avg. Flow Depth=0.89' Max Vel=3.87 fps Inflow=45.91 cfs 3.454 af n=0.030 L=411.1' S=0.0097 '/' Capacity=465.89 cfs Outflow=44.58 cfs 3.454 af
Reach S4.6: Swale S4.6	Avg. Flow Depth=1.74' Max Vel=6.02 fps Inflow=168.73 cfs 14.356 af n=0.030 L=537.0' S=0.0112 '/' Capacity=499.25 cfs Outflow=165.72 cfs 14.356 af
Reach S5.1: Swale S5.1	Avg. Flow Depth=0.29' Max Vel=2.49 fps Inflow=7.33 cfs 0.374 af n=0.030 L=478.0' S=0.0154 '/' Capacity=235.22 cfs Outflow=6.57 cfs 0.374 af
Pond C8: Culvert C8	Peak Elev=811.38' Storage=0.000 af Inflow=1.60 cfs 0.073 af 12.0" Round Culvert n=0.012 L=85.6' S=0.0245 '/' Outflow=1.60 cfs 0.073 af

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Pond N: North Infiltration Area Peak Elev=805.23' Storage=49,470 cf Inflow=26.07 cfs 2.341 af

Outflow=2.21 cfs 2.341 af

**Pond Sed Pond: SedimentationBasin** Peak Elev=793.20' Storage=245,676 cf Inflow=169.96 cfs 15.500 af 3 af Primary=11.51 cfs 5.165 af Secondary=31.49 cfs 2.330 af Tertiary=34.88 cfs 0.772 af Outflow=83.78 cfs 15.500 af

Link C1: Culvert C1	Inflow=1.96 cfs 0.304 af Primary=1.96 cfs 0.304 af
Link C10: Culvert C10	Inflow=4.95 cfs 0.275 af Primary=4.95 cfs 0.275 af
Link C2: Culvert C2	Inflow=18.32 cfs 1.038 af Primary=18.32 cfs 1.038 af
Link C3: Culvert C3	Inflow=49.91 cfs 2.880 af Primary=49.91 cfs 2.880 af
Link C4: Culvert C4	Inflow=60.33 cfs 3.550 af Primary=60.33 cfs 3.550 af
Link C5: Culvert C5	Inflow=66.73 cfs 4.277 af Primary=66.73 cfs 4.277 af
Link C6: Culvert C6	Inflow=53.57 cfs 3.184 af Primary=53.57 cfs 3.184 af
Link C7: Culvert C7	Inflow=123.92 cfs 10.752 af Primary=123.92 cfs 10.752 af
Link C9: Culvert C9	Inflow=20.94 cfs 1.658 af Primary=20.94 cfs 1.658 af
Link F10: Flume 10	Inflow=1.60 cfs 0.073 af Primary=1.60 cfs 0.073 af
Link F1A: Flume 1A	Inflow=9.08 cfs 0.491 af Primary=9.08 cfs 0.491 af
Link F1B: Flume 1B	Inflow=20.35 cfs 1.084 af Primary=20.35 cfs 1.084 af
Link F2A: Flume 2A	Inflow=3.34 cfs 0.169 af Primary=3.34 cfs 0.169 af
Link F2B: Flume 2B	Inflow=11.13 cfs 0.583 af Primary=11.13 cfs 0.583 af
Link F3: Flume 3	Inflow=2.33 cfs 0.113 af Primary=2.33 cfs 0.113 af

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Primary=77.88 cfs 8.267 af

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Link F4A: Flume 4A	Inflow=5.87 cfs 0.320 af		
	Primary=5.87 cfs 0.320 af		
Link F4B: Flume 4B	Inflow=15.35 cfs 0.820 af		
	Primary=15.35 cfs 0.820 af		
Link F5A: Flume 5A	Inflow=1.28 cfs 0.063 af		
	Primary=1.28 cfs 0.063 af		
Link F5B: Flume 5B	Inflow=3.77 cfs 0.189 af		
Ellik 1 ob. 1 fallio ob	Primary=3.77 cfs 0.189 af		
Link F6A: Flume 6A	Inflow=7.42 cfs 0.382 af		
Eliki GA. Halile GA	Primary=7.42 cfs 0.382 af		
Link F6B: Flume 6B	Inflow=15.57 cfs 0.795 af		
Link i ob. i idine ob	Primary=15.57 cfs 0.795 af		
Link F7A: Flume 7A	Inflow=5.92 cfs 0.281 af		
Link F7A. Fidilie 7A	Primary=5.92 cfs 0.281 af		
Link F7B: Flume 7B	Inflow=18.00 cfs 0.903 af		
Link F7B. Fidine 7B	Primary=18.00 cfs 0.903 af		
Link F7C: Flume 7C	Inflow=29.56 cfs 1.495 af		
Link F7G. Fluine 7G	Primary=29.56 cfs 1.495 af		
Link F8A: Flume 8A	Inflow=2.02 cfs 0.101 af		
LINK FOA: Fluine 6A	Primary=2.02 cfs 0.101 af		
Link FOD: Flume OD	Inflow=7.00 of 0.007 of		
Link F8B: Flume 8B	Inflow=7.88 cfs 0.397 af Primary=7.88 cfs 0.397 af		
Link FOA: Floors OA	Inflamm44 CE ata 0 C40 at		
Link F9A: Flume 9A	Inflow=11.65 cfs 0.648 af Primary=11.65 cfs 0.648 af		
Link FOD: Flore OD	•		
Link F9B: Flume 9B	Inflow=26.78 cfs 1.468 af Primary=26.78 cfs 1.468 af		
Link F9C: Flume 9C	Inflow=42.66 cfs 2.335 af Primary=42.66 cfs 2.335 af		
	•		
Link Wetland: Wetland	Inflow=77.88 cfs 8.267 af		

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## **Summary for Subcatchment Area 1: Area 1**

Runoff 6.34 cfs @ 12.14 hrs, Volume= 0.344 af, Depth= 3.18"

Routed to Link F1A: Flume 1A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	1.296 69 Pasture/grassland/range, Fair, HSG B					
	1.296 100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	5.2	93	0.1000	0.30	,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.5	7	0.2500	0.26		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.7	151	0.2500	3.50		Shallow Concentrated Flow,
	0.0	440	0.0000	0.74	00.07	Short Grass Pasture Kv= 7.0 fps
	0.3	113	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
_						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	6.7	364	Total			

## **Summary for Subcatchment Area 10: Area 10**

Runoff 2.83 cfs @ 12.14 hrs, Volume= 0.152 af, Depth= 3.18"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
-	0.573 69 Pasture/grassland/range, Fair, HSG B					
-						
0.573 100.00% Pervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	4.6	79	0.1000	0.29		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.1	21	0.2500	0.32		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.3	53	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.4	168	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
_						n= 0.030 Earth, grassed & winding
	6.4	321	Total	<u> </u>		

321 | Total

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#### **Summary for Subcatchment Area 11: Area 11**

Runoff = 8.84 cfs @ 12.15 hrs, Volume= 0.496 af, Depth= 3.18"

Routed to Link F9A: Flume 9A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription		
1.	872 6	9 Past	ure/grassl	and/range,	Fair, HSG B
1.	872	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.4	96	0.1000	0.30		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.3	4	0.2500	0.23		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.4	90	0.2500	3.50		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
1.5	606	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
					n= 0.030 Earth, grassed & winding
7.6	796	Total			

#### **Summary for Subcatchment Area 12: Area 12**

Runoff = 8.11 cfs @ 12.12 hrs, Volume= 0.427 af, Depth= 3.18"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription						
	1.610 69 Pasture/grassland/range, Fair, HSG B									
	1.610 100.00% Pervious Area									
	_				_					
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.8	100	0.2500	0.43		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.77"				
	0.1	31	0.2500	3.50		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	1.4	581	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm				
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'				
_						n= 0.030 Earth, grassed & winding				
	5.3	712	Total							

5.3 712 Total

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#### **Summary for Subcatchment Area 13: Area 13**

Runoff = 3.29 cfs @ 12.11 hrs, Volume= 0.166 af, Depth= 3.18"

Routed to Link F1B: Flume 1B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Des	cription					
0.626 69 Pasture/grassland/range, Fair, HSG B								
0	.626	100.	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
3.8	100	0.2500	0.43		Sheet Flow,			
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
0.5	183	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm			
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
4.5	333	Total			,,,			

# **Summary for Subcatchment Area 14: Area 14**

Runoff = 3.27 cfs @ 12.11 hrs, Volume= 0.164 af, Depth= 3.18"

Routed to Link F2B: Flume 2B

Area	(ac) C	N Desc	cription					
0.620 69 Pasture/grassland/range, Fair, HSG B								
0.	620	100.	00% Pervi	ous Area				
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
3.8	100	0.2500	0.43		Sheet Flow,			
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
 0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
4.4	331	Total						

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## **Summary for Subcatchment Area 15: Area 15**

Runoff = 4.72 cfs @ 12.13 hrs, Volume= 0.250 af, Depth= 3.18"

Routed to Link F2B: Flume 2B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription				
0.943 69 Pasture/grassland/range, Fair, HSG B							
0.	.943	100.	00% Pervi	ous Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
4.7	100	0.1500	0.35		Sheet Flow,		
0.5	95	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
0.7	269	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm		
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
5.9	464	Total					

## **Summary for Subcatchment Area 16: Area 16**

Runoff = 3.01 cfs @ 12.11 hrs, Volume= 0.151 af, Depth= 3.18"

Routed to Link F6B: Flume 6B

Area	(ac) C	N Desc	cription						
0.	0.571 69 Pasture/grassland/range, Fair, HSG B								
0.	.571	100.	00% Pervi	ous Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
3.8	100	0.2500	0.43		Sheet Flow,				
0.2	44	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
0.4	171	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding				
4.4	315	Total							

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#### **Summary for Subcatchment Area 17: Area 17**

Runoff = 5.14 cfs @ 12.11 hrs, Volume= 0.262 af, Depth= 3.18"

Routed to Link F6B: Flume 6B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Desc	cription				
	0.990 69 Pasture/grassland/range, Fair, HSG B							
	0.	990	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	3.8	100	0.2500	0.43		Sheet Flow,		
	0.2	46	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	0.7	278	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'		
_						n= 0.030 Earth, grassed & winding		
	4.7	424	Total					

## **Summary for Subcatchment Area 18: Area 18**

Runoff = 8.56 cfs @ 12.12 hrs, Volume= 0.439 af, Depth= 3.18"

Routed to Link F7B: Flume 7B

_	Area	(ac) C	N Desc	cription					
	1.	656 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
	1.656 100.00% Pervious Area								
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	3.8	88	0.2045	0.39		Sheet Flow,			
_	1.0	420	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
	4.8	508	Total						

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# **Summary for Subcatchment Area 19: Area 19**

Runoff = 3.65 cfs @ 12.11 hrs, Volume= 0.183 af, Depth= 3.18"

Routed to Link F7B: Flume 7B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Des	cription					
0.689 69 Pasture/grassland/range, Fair, HSG B								
	.689	100.	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
3.8	100	0.2500	0.43		Sheet Flow,			
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
0.3	136	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm			
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding			
4.3	286	Total						

## **Summary for Subcatchment Area 2: Area 2**

Runoff = 2.89 cfs @ 12.11 hrs, Volume= 0.148 af, Depth= 3.18"

Routed to Link F1A: Flume 1A

Area	(ac) C	N Desc	cription		
0.	.557 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	.557	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.2	100	0.1950	0.39		Sheet Flow,
0.1	22	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	181	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.7	303	Total	·		

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#### **Summary for Subcatchment Area 20: Area 20**

Runoff = 2.02 cfs @ 12.11 hrs, Volume= 0.101 af, Depth= 3.18"

Routed to Link F8A: Flume 8A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription				
0.381 69 Pasture/grassland/range, Fair, HSG B								
	0.	381	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	3.8	100	0.2500	0.43		Sheet Flow,		
	0.0	7	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	0.5	199	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
	43	306	Total			<del>-</del>		

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# **Summary for Subcatchment Area 21: Area 21**

Runoff = 2.74 cfs @ 12.11 hrs, Volume= 0.137 af, Depth= 3.18"

Routed to Link F9B: Flume 9B

Area	(ac) C	N Desc	cription				
0.516 69 Pasture/grassland/range, Fair, HSG B							
0	.516	100.	00% Pervi	ous Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.8	100	0.2500	0.43		Sheet Flow,		
0.0	7	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
0.5	205	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm		
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
4.3	312	Total					

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#### **Summary for Subcatchment Area 22: Area 22**

Runoff = 12.74 cfs @ 12.14 hrs, Volume= 0.684 af, Depth= 3.18"

Routed to Link F9B: Flume 9B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription							
2.	2.579 69 Pasture/grassland/range, Fair, HSG B									
2.	579	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
3.8	100	0.2500	0.43		Sheet Flow,					
0.1	21	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
2.5	1,025	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm					
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
6.4	1,146	Total								

## **Summary for Subcatchment Area 23: Area 23**

Runoff = 2.33 cfs @ 12.10 hrs, Volume= 0.113 af, Depth= 3.18"

Routed to Link F3: Flume 3

_	Area (ac) CN Description								
	0.	427 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
	0.	427	100.	00% Pervi	ous Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	2.9	70	0.2500	0.40	, ,	Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	8.0	227	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm			
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'			
				10.00		n= 0.030 Earth, grassed & winding			
	0.1	65	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume			
						Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'			
_						n= 0.078 Riprap, 12-inch			
	3.8	362	Total						

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## **Summary for Subcatchment Area 24: Area 24**

Runoff = 6.11 cfs @ 12.11 hrs, Volume= 0.312 af, Depth= 3.18"

Routed to Link F7C: Flume 7C

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	Area (ac) CN Description									
1.	.177 6	9 Past	ure/grassl	and/range,	Fair, HSG B					
1.	.177	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
3.8	100	0.2500	0.43		Sheet Flow,					
0.3	60	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
0.6	225	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm					
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
4.7	385	Total								

# **Summary for Subcatchment Area 25: Area 25**

Runoff = 3.58 cfs @ 12.11 hrs, Volume= 0.181 af, Depth= 3.18"

Routed to Link F8B: Flume 8B

Area	(ac) C	N Desc	cription		
0.	682 6	9 Past	ure/grassl	and/range,	Fair, HSG B
0.	682	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	100	0.2500	0.43		Sheet Flow,
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.5	199	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
4.5	349	Total			

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#### **Summary for Subcatchment Area 26: Area 26**

Runoff = 3.56 cfs @ 12.11 hrs, Volume= 0.179 af, Depth= 3.18"

Routed to Link F9C: Flume 9C

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription			
0.	677 6	9 Past	ure/grassl	Fair, HSG B		
0.	677	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity Capacity (ft/sec) (cfs)		•	
3.8	100	0.2500	0.43		Sheet Flow,	
0.2	50	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
0.5	191	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm	
					Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
4.5	341	Total				

# **Summary for Subcatchment Area 27: Area 27**

Runoff = 12.62 cfs @ 12.14 hrs, Volume= 0.688 af, Depth= 3.18"

Routed to Link F9C: Flume 9C

Area	(ac) C	N Des	cription							
2.	2.594 69 Pasture/grassland/range, Fair, HSG B									
2.	.594	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
3.8	99	0.2500	0.43		Sheet Flow,					
3.1	1,258	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
6.9	1,357	Total								

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## **Summary for Subcatchment Area 28: Area 28**

Runoff = 13.48 cfs @ 12.10 hrs, Volume= 0.663 af, Depth= 3.69"

Routed to Reach RD3: Roadside Ditch 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Desc	cription			
_	1.	735 6	9 Past	ure/grassl	and/range,	Fair, HSG B	_
	0.	424 9	6 Grav	∕el surface	, HSG A		
_	2.	159 7	'4 Weig	hted Aver	age		_
	2.	159	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.8	100	0.2500	0.43		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.1	25	0.2500	3.50		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	3.9	125	Total				

#### **Summary for Subcatchment Area 29: Area 29**

Runoff = 3.67 cfs @ 12.10 hrs, Volume= 0.179 af, Depth= 3.48"

Routed to Reach RD5: Roadside Ditch 5

_	Area (ac) CN Description									
	0.	543 (	39 Past	ure/grassl	and/range,	Fair, HSG B				
	0.	073	,							
0.073 96 Gravel surface, HSG A 0.616 72 Weighted Average 0.616 100.00% Pervious Area										
01010										
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•				
	3.8	100	0.2500	0.43		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.77"				
	0.0	9	0.2500	3.50		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
_	3.8	109	Total			•				

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## **Summary for Subcatchment Area 3: Area 3**

Runoff 1.80 cfs @ 12.12 hrs, Volume= 0.092 af, Depth= 3.18"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription							
0.	0.348 69 Pasture/grassland/range, Fair, HSG B									
0.	348	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
4.2	100	0.1950	0.39		Sheet Flow,					
					Grass: Short n= 0.150 P2= 2.77"					
0.2	36	0.2500	3.50		Shallow Concentrated Flow,					
0.4	162	0.0200	6.74	80.87	Short Grass Pasture Kv= 7.0 fps  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
4.8	298	Total								

## **Summary for Subcatchment Area 30: Area 30**

Runoff 0.86 cfs @ 12.07 hrs, Volume= 0.039 af, Depth= 3.18"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription							
	0.149 69 Pasture/grassland/range, Fair, HSG B										
	0.149 100.00% Pervious Area										
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	0.7	12	0.2500	0.28		Sheet Flow,					
	0.5	199	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding					
_	1 2	211	Total								

1.2 211 Total

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#### **Summary for Subcatchment Area 31: Area 31**

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 0.0

0.033 af, Depth= 3.18"

Routed to Link F10: Flume 10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Desc	cription							
	0.126 69 Pasture/grassland/range, Fair, HSG B										
	0.	126	100.	00% Pervi	ous Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	2.5	59	0.2500	0.39		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"			

#### **Summary for Subcatchment Area 32: Area 32**

Runoff = 2.55 cfs @ 12.10 hrs, Volume= 0.125 af, Depth= 3.28"

Routed to Reach S3.2: Swale S3.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription					
	0.	435 6	39 Past	ure/grassl	and/range,	Fair, HSG B			
	0.	022	96 Grav	el surface/	, HSG A				
Area (ac) CN Description  0.435 69 Pasture/grassland/range, Fair, HSG B  0.022 96 Gravel surface, HSG A  0.457 70 Weighted Average 0.457 100.00% Pervious Area  Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)  3.8 100 0.2500 0.43 Sheet Flow, Grass: Short n= 0.150 P2= 2.77"									
0.457 100.00% Pervious Area									
	Тс	Length	Slope	,	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	22	0.2500	3.50		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	3.9	122	Total						

#### **Summary for Subcatchment Area 33: Area 33**

Runoff = 5.48 cfs @ 12.11 hrs, Volume= 0.280 af, Depth= 3.18"

Routed to Link F7C : Flume 7C

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	Area	(ac) C	N Des	cription			
1.056 69 Pasture/grassland/range, Fair, HSG B							
•	1.	056		00% Pervi		,	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
•	3.8	100	0.2500	0.43		Sheet Flow,	
	0.3	57	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	0.6	262	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
•	4.7	419	Total			<del>-</del>	

#### **Summary for Subcatchment Area 34: Area 34**

Runoff = 2.28 cfs @ 12.11 hrs, Volume= 0.115 af, Depth= 3.18"

Routed to Link F8B: Flume 8B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription			
0.434 69 Pasture/grassland/range, Fair, HSG B							
_	0.	434	100.	00% Pervi	ous Area		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	3.8	100	0.2500	0.43		Sheet Flow,	
	0.1	15	0.2500	3.50		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
	0.6	259	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
_	4.5	374	Total				

# **Summary for Subcatchment Area 35: Area 35**

Runoff = 7.33 cfs @ 12.11 hrs, Volume= 0.374 af, Depth= 3.69" Routed to Reach S5.1 : Swale S5.1

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	Area	(ac) C	N Desc	cription		
	_					Fair, HSG B
_	0.	232 9	<u>6 Grav</u>	<u>el surface</u>	<u>, HSG A</u>	
	1.	218 7	'4 Weig	hted Aver	age	
	1.	218	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.3	70	0.1736	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.6	20	0.0050	0.53		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.77"
	0.7	10	0.1766	0.24		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.0	4	0.1766	2.94		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	4.6	104	Total			

#### **Summary for Subcatchment Area 36: Area 36**

Runoff = 5.57 cfs @ 12.11 hrs, Volume= 0.275 af, Depth= 2.79"

Routed to Reach S1.1: Swale S1.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription			
0.322 39 Pasture/grassland/range, Good, HSG A							
0.696 69 Pasture/grassland/range, Fair, HSG B							
0.167 96 Gravel surface, HSG A							
1.185 65 Weighted Average							
	1.	185	100.	00% Pervi	ous Area		
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.4	72	0.1736	0.35		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.6	20	0.0050	0.53		Sheet Flow,	
						Smooth surfaces n= 0.011 P2= 2.77"	
	0.1	14	0.1766	2.94		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	4.1	106	Total				

#### **Summary for Subcatchment Area 37: Area 37**

Runoff = 5.48 cfs @ 12.11 hrs, Volume= 0.269 af, Depth= 2.50"

Routed to Reach S1.2 : Swale S1.2

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 Area (ac) CN Description								
0.	415	39 Pas	ture/grassl	and/range,	Good, HSG A			
0.743 69 Pasture/grassland/range, Fair, HSG B								
0.133 96 Gravel surface, HSG A								
1.291 62 Weighted Average								
1.	291	100.	.00% Pervi	ous Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.8	100	0.2500	0.43		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
0.1	20	0.2500	3.50		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
3.9	120	Total			•			

#### **Summary for Subcatchment Area 38: Area 38**

Runoff = 3.43 cfs @ 12.11 hrs, Volume= 0.172 af, Depth= 2.60"

Routed to Reach S1.3: Swale S1.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription					
_	0.	Good, HSG A							
	0.	Fair, HSG B							
0.123 96 Gravel surface, HSG A									
	0.795 63 Weighted Average								
	0.	795	100.	00% Pervi	ous Area				
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	14	0.2500	3.50		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	17	0.0050	1.44		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.2	24	0.0833	2.02		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	4.3	155	Total						

## **Summary for Subcatchment Area 39: Area 39**

Runoff = 1.57 cfs @ 12.18 hrs, Volume= 0.101 af, Depth= 1.96"

Routed to Reach S1.3: Swale S1.3

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Area	(ac) C	N Desc	cription					
0.	.317 3	39 Past	ure/grassl	and/range,	Good, HSG A			
0.	.243 6	9 Past	ure/grassl	and/range,	Fair, HSG B			
0.060 96 Gravel surface, HSG A								
0.	0.620 56 Weighted Average							
0.	.620	100.	00% Pervi	ous Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.2	81	0.0245	0.16		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
1.0	19	0.2500	0.31		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
0.1	29	0.2500	3.50		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	20	0.0050	1.44		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
0.1	19	0.1053	2.27		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
9.6	168	Total						

#### **Summary for Subcatchment Area 4: Area 4**

Runoff = 1.55 cfs @ 12.10 hrs, Volume= 0.

0.076 af, Depth= 3.18"

Routed to Link F2A: Flume 2A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Desc	cription				
0.288 69 Pasture/grassland/range, Fair, HSG B							
0.	.288	100.	00% Pervi	ous Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.4	49	0.0820	0.24	,	Sheet Flow,		
0.6	247	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
4.0	296	Total	·	·			

# Summary for Subcatchment Area 40: Area 40

Runoff = 2.11 cfs @ 12.15 hrs, Volume= 0.120 af, Depth= 1.96"

Routed to Reach S1.4: Swale S1.4

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Area	(ac) C	N Desc	cription		
0.	389 3	39 Past	ure/grassl	and/range,	Good, HSG A
0.	270	39 Past	ure/grassl	and/range,	Fair, HSG B
0.	080 9	96 Grav	el surface	, HSG A	
0.	739		ghted Aver		
0.	739	100.	00% Pervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.5	49	0.0408	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
2.1	47	0.2500	0.37		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.2	4	0.0050	0.39		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 2.77"
0.2	19	0.0050	1.44		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.2	22	0.1136	2.36		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	141	Total			

## Summary for Subcatchment Area 41a: Area 41a

3.58 cfs @ 12.15 hrs, Volume= 0.202 af, Depth= 2.79" Runoff

Routed to Reach S1.6: Swale S1.6

_	Area (ac) CN Description						
0.249 39 Pasture/grassland/range, 0					and/range,	Good, HSG A	
0.489 69 Pasture/grassland/range, Fa						Fair, HSG B	
0.133 96 Gravel surface, HSG A					, HSG A		
	0.871 65 Weighted Average						
	0.	871	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.3	100	0.0500	0.23		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.5	44	0.0500	1.57		Shallow Concentrated Flow,	
_						Short Grass Pasture Kv= 7.0 fps	
	7.8	144	Total				

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#### **Summary for Subcatchment Area 41b: Area 41b**

Runoff = 3.63 cfs @ 12.15 hrs, Volume= 0.201 af, Depth= 3.38"

Routed to Reach S1.7: Swale S1.7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription			
	_			ure/grassl el surface		Fair, HSG B	
_					•		
0.712 71 Weighted Average							
	0.	712	100.	00% Pervi	ous Area		
	Тс	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.3	100	0.0500	0.23		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.77"	
	0.0	2	0.0500	1.57		Shallow Concentrated Flow,	
	3.0	_	212000			Short Grass Pasture Kv= 7.0 fps	
_	7.3	102	Total			·	

#### **Summary for Subcatchment Area 42: Area 42**

Runoff = 3.63 cfs @ 12.15 hrs, Volume= 0.204 af, Depth= 3.18"

Routed to Reach S1.5: Swale S1.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
	0.	769 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.	769	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	7.3	100	0.0500	0.23	, ,	Sheet Flow,
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
_	7.7	139	Total			

#### **Summary for Subcatchment Area 43: Area 43**

Runoff = 7.53 cfs @ 12.24 hrs, Volume= 0.561 af, Depth= 2.41"

Routed to Reach S1.8: Swale S1.8

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			N Desc	cription		
_	Area					
	0.	797 3	89 Past	ure/grassl	and/range,	Good, HSG A
	1.	938 6				Fair, HSG B
				el surface	<b>O</b> 7	,
-				hted Aver	<i>'</i>	
		792 792		00% Pervi		
	۷.	192	100.	00% Pervi	ous Area	
	То	Longth	Clana	Volocity	Canacity	Description
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.3	100	0.0500	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.3	119	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	22	0.1905	3.06		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	6.0	178	0.0050	0.49		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	14.7	419	Total			•

## **Summary for Subcatchment Area 44: Area 44**

Runoff 0.43 cfs @ 12.47 hrs, Volume= 0.074 af, Depth= 0.63"

Routed to Reach S2.1 : Swale S2.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Desc	cription			
1.416 39 Pasture/grassland/range, Good, HSG A							
	1.	416	100.	00% Pervi	ous Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.5	100	0.0260	0.18		Sheet Flow,	
	2.9	194	0.0260	1.13		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Parture, Ky= 7.0 fps	
	9.6	647	0.0260	1.13		Short Grass Pasture Kv= 7.0 fps  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps	
	22.0	941	Total				

#### **Summary for Subcatchment Area 45: Area 45**

11.02 cfs @ 12.10 hrs, Volume= 0.542 af, Depth= 3.18"

Routed to Reach S3.1: Swale S3.1

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_	Area	(ac) C	N Desc	cription					
	2.044 69 Pasture/grassland/range, Fair, HSG B								
_	2.	044	100.00% Pervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	3.8	100	0.2500	0.43	, ,	Sheet Flow,			
	0.2	38	0.2632	3.59		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
_	4.0	138	Total			·			

#### **Summary for Subcatchment Area 46: Area 46**

Runoff = 3.63 cfs @ 12.15 hrs, Volume=

0.204 af, Depth= 3.18"

Routed to Reach S3.4: Swale S3.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area (ac) CN Description							
0.769 69 Pasture/grassland/range, Fair, HSG B								
	0.	769	100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
•	7.3	100	0.0500	0.23	, ,	Sheet Flow,		
	0.4	39	0.0500	1.57		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	7.7	139	Total					

## **Summary for Subcatchment Area 47: Area 47**

Runoff = 0.44 cfs @ 12.12 hrs, Volume= 0.023 af, Depth= 3.48"

Routed to Link C6: Culvert C6

_	Area (ac)	CN	Description					
	0.070	69	Pasture/grassland/range, Fair, HSG B					
_	0.009	96	Gravel surface, HSG A					
	0.079	72	Weighted Average					
	0.079		100.00% Pervious Area					

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.5	90	0.2500	0.43	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.2	10	0.0500	0.14		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.1	10	0.0500	1.57		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	15	0.0050	1.44		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.1	18	0.1390	2.61		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	5.1	143	Total			

#### **Summary for Subcatchment Area 48: Area 48**

Runoff = 16.26 cfs @ 12.19 hrs, Volume= 1.050 af, Depth= 3.38"

Routed to Reach S3.5: Swale S3.5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Desc	cription					
				Pasture/grassland/range, Fair, HSG B					
	0.	256 g	<u>6 Grav</u>	∕el surface	, HSG A				
	3.726 71 Weighted Average								
	3.	726	100.	00% Pervi	ous Area				
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
•	7.3	100	0.0500	0.23	, ,	Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.9	83	0.0500	1.57		Shallow Concentrated Flow,			
	0.0		0.000			Short Grass Pasture Kv= 7.0 fps			
	2.9	208	0.0289	1.19		Shallow Concentrated Flow,			
	2.0	200	0.0200	1.10		Short Grass Pasture Kv= 7.0 fps			
•	11 1	201	Total			Chart Grade Factors 1tt 1.0 ips			
	11.1	391	Total						

#### **Summary for Subcatchment Area 49: Area 49**

Runoff = 0.32 cfs @ 12.19 hrs, Volume= 0.036 af, Depth= 0.63"

Routed to Reach S2.1: Swale S2.1

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 Area	(ac) C	N Des	cription						
0.698 39 Pasture/grassland/range, Good, HSG A									
0.698 100.00% Pervious Area									
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.8	100	0.0600	0.25		Sheet Flow, Grass: Short	n= 0.150	P2= 2.77"		

#### **Summary for Subcatchment Area 5: Area 5**

Runoff = 2.81 cfs @ 12.10 hrs, Volume=

0.134 af, Depth= 3.18"

Routed to Link F6A: Flume 6A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription			
0.504 69 Pasture/grassland/range, Fair, HSG B							
	0.504		100.	00% Pervi	ous Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	2.9	72	0.2500	0.41		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"	
	0.5	186	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding	
	3.4	258	Total				

# **Summary for Subcatchment Area 50: Area 50**

Runoff = 1.22 cfs @ 12.10 hrs, Volume=

0.059 af, Depth= 3.18"

Routed to Link F4A: Flume 4A

_	Area (ac)	CN	Description
	0.223	69	Pasture/grassland/range, Fair, HSG B
	0.223		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	77	0.2500	0.41		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.77"
0.5	157	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow,
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
					n= 0.030 Earth, grassed & winding
0.1	62	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'
					n= 0.078 Riprap, 12-inch
3.7	296	Total			

# **Summary for Subcatchment Area 51: Area 51**

Runoff = 3.35 cfs @ 12.12 hrs, Volume= 0.17

0.174 af, Depth= 3.18"

Routed to Link F4B: Flume 4B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription		
	0.	655 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	0.655 100.00% Pervious Are					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.8	100	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.1	11	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	8.0	314	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	0.3	217	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch
_	5.0	642	Total		•	

#### **Summary for Subcatchment Area 52: Area 52**

Runoff = 1.28 cfs @ 12.10 hrs, Volume=

0.063 af, Depth= 3.18"

Routed to Link F5A: Flume 5A

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_	Area	(ac) C	N Des	cription			
0.237 69 Pasture/grassland/range, Fair, HSG B							
	0.	237	100.	00% Pervi	ous Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	3.3	83	0.2500	0.42		Sheet Flow,	
	0.5	138	0.0200	4.80	23.38	Grass: Short n= 0.150 P2= 2.77" <b>Trap/Vee/Rect Channel Flow, Existing Diversion Berm</b> Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26' n= 0.030 Earth, grassed & winding	
	0.2	127	0.2500	12.26	441.43		
_	4.0	348	Total				

#### **Summary for Subcatchment Area 53: Area 53**

2.50 cfs @ 12.11 hrs, Volume= Runoff

0.126 af, Depth= 3.18"

Routed to Link F5B: Flume 5B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription				
0.475 69 Pasture/grassland/range, Fair, HSG B								
	0.	475	100.	00% Pervi	ous Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
•	3.5	90	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"		
	8.0	219	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'		
	0.2	116	0.2500	12.26	441.43	n= 0.030 Earth, grassed & winding  Trap/Vee/Rect Channel Flow, Riprap Flume  Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.078 Riprap, 12-inch		
•	4.5	425	Total					

# **Summary for Subcatchment Area 54: Area 54**

5.93 cfs @ 12.11 hrs, Volume= 0.300 af, Depth= 2.23" Runoff Routed to Reach S2.3: Swale S2.3

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	Area	(ac) C	N Desc	Description		
						Fair, HSG B
_	0.	<u>539 3</u>	9 Past	ure/grassl	<u>and/range,</u>	Good, HSG A
	1.	618 5				
	1.	618	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	3.1	77	0.2500	0.41		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	8.0	237	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm
						Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
						n= 0.030 Earth, grassed & winding
	0.3	70	0.2500	3.50		Shallow Concentrated Flow,
	3.0	, ,	0.2000	3.00		Short Grass Pasture Kv= 7.0 fps
_	4.2	384	Total			enon ender addition the though

#### **Summary for Subcatchment Area 55: Area 55**

Runoff = 2.95 cfs @ 12.11 hrs, Volume= 0.147 af, Depth= 2.13"

Routed to Reach S2.2 : Swale S2.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

_	Area	(ac) C	N Des	cription					
	0.	520	69 Past	Pasture/grassland/range, Fair, HSG B					
0.306 39			39 Past	Pasture/grassland/range, Good, HSG A					
0.826 58 Weighted Average									
	0.	826	100.	00% Pervi	ous Area				
	Tc	Length	•	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	0.1	26	0.2500	3.50		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	3.9	126	Total						

## **Summary for Subcatchment Area 56: Area 56**

Runoff = 6.17 cfs @ 12.13 hrs, Volume= 0.325 af, Depth= 3.18"

Routed to Link F4B: Flume 4B

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		<i>,</i> , ,				
_	Area	(ac) C	N Des	cription		
	1.	228 6	9 Past	ure/grassl	and/range,	Fair, HSG B
	1.	228	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
•	3.8	100	0.2500	0.43		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.0	6	0.2500	3.50		Shallow Concentrated Flow,
	1.9	541	0.0200	4.80	23.38	Short Grass Pasture Kv= 7.0 fps <b>Trap/Vee/Rect Channel Flow, Existing Diversion Berm</b> Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'
-						n= 0.030 Earth, grassed & winding
	5.7	647	Total			

# **Summary for Subcatchment Area 57: Area 57**

Runoff = 3.79 cfs @ 12.11 hrs, Volume= 0.194 af, Depth= 2.13"

Routed to Reach S4.1: Swale S4.1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Des	cription						
0.	526 3	39 Past	Pasture/grassland/range, Good, HSG A						
0.									
_			Gravel surface, HSG A						
			ghted Aver	<i>'</i>					
	089	,	00% Pervi	•					
	000	100.	00 70 1 01 1	00371100					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.5	80	0.2000	0.38	,	Sheet Flow,				
					Grass: Short n= 0.150 P2= 2.77"				
0.2	20	0.0500	1.34		Sheet Flow,				
					Smooth surfaces n= 0.011 P2= 2.77"				
0.1	10	0.0050	1.44		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
0.5	48	0.0625	1.75		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
4.3	158	Total			•				

#### **Summary for Subcatchment Area 58: Area 58**

Runoff = 4.33 cfs @ 12.11 hrs, Volume= 0.221 af, Depth= 2.23"

Routed to Reach S4.2: Swale S4.2

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	Area	(ac)	CN D	escri	ption				
0.580 39				Pasture/grassland/range, Good, HSG A					
	0.	433		Pasture/grassland/range, Fair, HSG B					
_	0.	181	96 G	Gravel surface, HSG B					
	1.	194	59 W	/eigh	ted Aver	age			
	1.	194	1	00.00	)% Pervi	ous Area			
	Tc (min)	Lengtl (feet			/elocity (ft/sec)	Capacity (cfs)	Description		
	3.8	100	0.250	00	0.43		Sheet Flow,		
	0.6	12 ⁻	l 0.23	14	3.37		Grass: Short n= 0.150 P2= 2.77"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	4 4	22.	L Total			•			

#### **Summary for Subcatchment Area 59: Area 59**

Runoff = 6.17 cfs @ 12.12 hrs, Volume= 0.330 af, Depth= 1.78"

Routed to Reach S4.3: Swale S4.3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) (	CN Des	cription					
	1.	134	39 Pas	Pasture/grassland/range, Good, HSG A					
1.086 69 Pasture/grassla					and/range,	Fair, HSG B			
	2.	220	54 Wei	ghted Aver	age				
	2.	220	100.	.00% Pervi	ous Area				
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	100	0.2500	0.43		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.77"			
	8.0	140	0.1857	3.02		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	4.6	240	Total						

# Summary for Subcatchment Area 6: Area 6

Runoff = 4.72 cfs @ 12.12 hrs, Volume= 0.248 af, Depth= 3.18"

Routed to Link F6A: Flume 6A

 Area (ac)	CN	Description
0.936	69	Pasture/grassland/range, Fair, HSG B
0.936		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.1	100	0.2070	0.40		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	46	0.2500	3.50		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.0	395	0.0200	6.74	80.87	Trap/Vee/Rect Channel Flow, Diversion Berm
						Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00'
						n= 0.030 Earth, grassed & winding
	5.3	541	Total			

#### **Summary for Subcatchment Area 60: Area 60**

Runoff = 8.43 cfs @ 12.12 hrs, Volume= 0.441 af, Depth= 2.13"

Routed to Reach S4.4 : Swale S4.4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) (	<u>CN Des</u>	cription		
	0.	939	39 Pas	ture/grassl	and/range,	Good, HSG A
1.537			69 Pas	ture/grassl	and/range,	Fair, HSG B
2.476 58 Weighted Average						
	2.	476		.00% Pervi		
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.9	163	0.1718	2.90		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	4.7	263	Total			<u> </u>

# **Summary for Subcatchment Area 61: Area 61**

Runoff = 4.54 cfs @ 12.11 hrs, Volume= 0.238 af, Depth= 1.70"

Routed to Reach S4.5 : Swale S4.5

 Area (ac)	CN	Description
0.898	39	Pasture/grassland/range, Good, HSG A
 0.785	69	Pasture/grassland/range, Fair, HSG B
1.683	53	Weighted Average
1.683		100.00% Pervious Area

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	Тс	Length	Slope	,	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.9	100	0.2345	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	46	0.2345	3.39		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	4.1	146	Total			

# **Summary for Subcatchment Area 62: Area 62**

Runoff = 1.18 cfs @ 12.39 hrs, Volume=

0.150 af, Depth= 0.90"

Routed to Reach S4.6: Swale S4.6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) C	N Des	cription							
					Good, HSG A					
0.	000	96 Gra∖	Gravel surface, HSG A							
0.130 96 Gravel surface, HSG A										
2.	2.001 43 Weighted Average									
2.	001	100.	00% Pervi	ous Area						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·					
15.7	100	0.0074	0.11		Sheet Flow,	_				
					Grass: Short n= 0.150 P2= 2.77"					
4.7	169	0.0074	0.60		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.4	49	0.0800	1.98		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
20.8	318	Total			· •	_				

## **Summary for Subcatchment Area 63: Area 63**

Runoff = 0.97 cfs @ 12.21 hrs, Volume= 0.114 af, Depth= 0.63"

Routed to Reach S2.3 : Swale S2.3

 Area (ac)	CN	Description
2.177	39	Pasture/grassland/range, Good, HSG A
2.177		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	6.4	66	0.0303	0.17		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.6	34	0.2500	0.35		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	49	0.2500	3.50		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	8.2	149	Total			

#### **Summary for Subcatchment Area 64: Area 64**

Runoff = 1.14 cfs @ 12.16 hrs, Volume= 0.071 af, Depth= 1.44"

Routed to Pond Sed Pond : Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	Area (ac) CN Description							
0.	.479	39 Pas	ture/grassl	and/range,	Good, HSG A			
0.115 96 Gravel surface, HSG A								
0.594 50 Weighted Average								
0.	.594	100.	00% Pervi	ous Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.1	100	0.0544	0.24		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.77"			
0.5	47	0.0544	1.63		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
7.6	147	Total						

# **Summary for Subcatchment Area 65: Area 65**

Runoff = 14.43 cfs @ 12.04 hrs, Volume= 0.799 af, Depth= 6.35"

Routed to Pond Sed Pond : Sedimentation Basin

_	Area	(ac)	CN	Desc	cription		
	1.	509	98	Wate	er Surface,	, HSG A	
	1.509 100.00% Impervious Area						1
	Tc	Lengt	h s	Slope	Velocity	Canacity	Description
_	(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	Boodipaon
	0.0						Direct Entry,

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#### **Summary for Subcatchment Area 66: Area 66**

Runoff = 2.36 cfs @ 12.21 hrs, Volume= 0.273 af, Depth= 0.63"

Routed to Pond Sed Pond: Sedimentation Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac) C	N Des	cription		
_	5.	227 3	9 Past	ture/grassl	and/range,	Good, HSG A
	5.	227	100.	00% Pervi	ous Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.5	75	0.0933	0.28		Sheet Flow,
	4.0	0.5	0.0500	0.00		Grass: Short n= 0.150 P2= 2.77"
	1.3	25	0.2500	0.33		Sheet Flow, Grass: Short n= 0.150 P2= 2.77"
	0.0	10	0.2500	3.50		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	0.8	381	0.0265	7.85	109.92	Trap/Vee/Rect Channel Flow, Bot.W=0.00' D=2.00' Z= 4.0 & 3.0 '/' Top.W=14.00'
	0.8	162	0.2500	3.50		n= 0.030 Earth, grassed & winding  Shallow Concentrated Flow,  Short Grass Pasture Kv= 7.0 fps
_	0.5	48	0.0500	1.57		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
_	7.9	701	Total			

# **Summary for Subcatchment Area 67: Area 67**

Runoff = 1.99 cfs @ 12.64 hrs, Volume= 0.304 af, Depth= 1.20"

Routed to Reach RD1: Roadside Ditch 1

 Area (ac)	CN	Description
2.616	39	Pasture/grassland/range, Good, HSG A
0.039	69	Pasture/grassland/range, Fair, HSG B
 0.380	96	Gravel surface, HSG A
 3.035	Weighted Average	
3.035		100.00% Pervious Area

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	Tc	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	(min)	(leet)	(11/11)	(11/560)	(615)	
	16.2	100	0.0068	0.10		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	22.7	786	0.0068	0.58		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
-	38.9	886	Total			•

# **Summary for Subcatchment Area 68: Area 68**

Runoff = 0.26 cfs @ 12.11 hrs, Volume= 0.017 af, Depth= 0.83"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	(ac) (	CN De	Description							
0	.227	39 Pa	Pasture/grassland/range, Good, HSG A							
0	0.024 69 Pasture/grassland/range, Fair, HSG B									
0.	0.251 42 Weighted Average									
0.	.251	10	0.00% Perv	ious Area						
Tc	Length		,	Capacity	Description					
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)						
3.1	52	0.1154	4 0.28		Sheet Flow,					
					Grass: Short	n= 0.150	P2= 2.77"			

#### **Summary for Subcatchment Area 69: Area 69**

Runoff = 1.50 cfs @ 12.11 hrs, Volume= 0.086 af, Depth= 1.13"

Routed to Reach RD2: Roadside Ditch 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Area	(ac)	CN	Desc	Description						
	0.	695	39	Past	ure/grassla	and/range,	Good, HSG A				
0.218 69 Pasture/grassland/range, Fair, HSG B											
_	0.913 46 Weighted Average										
	0.	913		100.	00% Pervi	ous Area					
	Tc	Lengt	h S	Slope	Velocity	Capacity	Description				
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)					
	3.5	8	6 0.	.2326	0.41		Sheet Flow,				
								0 450 DO 0 77"			

Grass: Short n= 0.150 P2= 2.77"

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#### **Summary for Subcatchment Area 7: Area 7**

Runoff = 4.85 cfs @ 12.14 hrs, Volume= 0.261 af, Depth= 3.18"

Routed to Link F4A: Flume 4A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area	Area (ac) CN Description										
0.	986 6	9 Past	ure/grassl	and/range,	Fair, HSG B						
0.	986	100.	00% Pervi	ous Area							
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
2.2	31	0.1000	0.24		Sheet Flow,						
					Grass: Short n= 0.150 P2= 2.77"						
2.8	69	0.2500	0.40		Sheet Flow,						
					Grass: Short n= 0.150 P2= 2.77"						
0.0	9	0.2500	3.50		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
1.5	419	0.0200	4.80	23.38	Trap/Vee/Rect Channel Flow, Existing Diversion Berm						
					Bot.W=0.00' D=1.18' Z= 4.0 & 3.0 '/' Top.W=8.26'						
					n= 0.030 Earth, grassed & winding						
0.1	80	0.2500	12.26	441.43	Trap/Vee/Rect Channel Flow, Riprap Flume						
					Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00'						
					n= 0.078 Riprap, 12-inch						
6.6	608	Total									

## **Summary for Subcatchment Area 70: Area 70**

Runoff = 3.98 cfs @ 12.11 hrs, Volume= 0.213 af, Depth= 1.53"

Routed to Reach RD4: Roadside Ditch 4

	Area	(ac) C	N Desc	cription		
1.016 39 Pasture/grassland/range, Good, HSG A						
0.620 69 Pasture/grassland/range, Fair, HSG B						
	0.	035	96 Grav	el surface/	, HSG A	
_	1.671 51 Weighted Average					
	1.	671	•	00% Pervi	•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
_	3.8	100	0.2500	0.43		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	0.2	26	0.1538	2.75		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	4.0	126	Total			·

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# **Summary for Subcatchment Area 71: Area 71**

Runoff = 5.65 cfs @ 12.32 hrs, Volume= 0.684 af, Depth= 0.83"

Routed to Pond N: North Infiltration Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

Area (ac) CN Description						
9.360 39 Pasture/grassland/range, G					and/range,	Good, HSG A
0.515 96 Gravel surface, HSG A					, HSG A	
9.875 42 Weighted Average 9.875 100.00% Pervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'
	10.5	100	0.0200	0.16		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.77"
	1.7	100	0.0200	0.99		Shallow Concentrated Flow,
	4.4	1,137	0.0193	4.31	32.30	Short Grass Pasture Kv= 7.0 fps <b>Trap/Vee/Rect Channel Flow, Roadside Ditch</b> Bot.W=0.00' D=1.00' Z= 5.0 & 10.0 '/' Top.W=15.00' n= 0.030
_	16.6	1.337	Total			11 0.000

#### **Summary for Subcatchment Area 8: Area 8**

Runoff = 4.50 cfs @ 12.10 hrs, Volume= 0.216 af, Depth= 3.18"

Routed to Link F7A: Flume 7A

	Α	rea (sf)	CN D	escription		
		35,545	69 P	asture/gra	ssland/ran	ge, Fair, HSG B
		35,545	1	00.00% Pe	ervious Are	ea
	Tc in)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.9	59	0.1695	0.33		Sheet Flow,
(	0.7	289	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding
	3.6	348	Total			<u> </u>

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#### **Summary for Subcatchment Area 9: Area 9**

Runoff = 1.44 cfs @ 12.09 hrs, Volume= 0.065 af, Depth= 3.18"

Routed to Link F7A: Flume 7A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

	Α	rea (sf)	CN D	escription				
		10,716 69 Pasture/grassland/range, Fair, HSG B						
10,716 100.00% Pervious Area					ervious Are	a		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	2.1	48	0.2500	0.38		Sheet Flow,		
	0.2	96	0.0200	6.74	80.87	Grass: Short n= 0.150 P2= 2.77"  Trap/Vee/Rect Channel Flow, Diversion Berm  Bot.W=0.00' D=2.00' Z= 4.0 & 2.0 '/' Top.W=12.00' n= 0.030 Earth, grassed & winding		
	2.3	144	Total					

#### Summary for Reach RD1: Roadside Ditch 1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 1.20" for 100-yr, 24-hr event

Inflow = 1.99 cfs @ 12.64 hrs, Volume= 0.304 af

Outflow = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af, Atten= 1%, Lag= 6.5 min

Routed to Link C1: Culvert C1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.00 fps, Min. Travel Time= 3.7 min Avg. Velocity = 0.80 fps, Avg. Travel Time= 9.2 min

Peak Storage= 434 cf @ 12.68 hrs

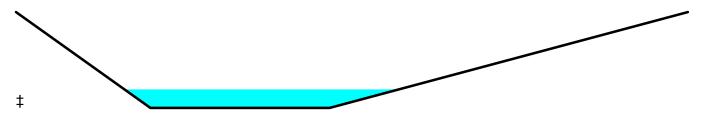
Average Depth at Peak Storage= 0.19', Surface Width= 6.14' Bank-Full Depth= 1.00' Flow Area= 9.5 sf, Capacity= 47.16 cfs

4.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 3.0 8.0 '/' Top Width= 15.00'

Length= 440.6' Slope= 0.0188 '/'

Inlet Invert= 824.00', Outlet Invert= 815.70'



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#### Summary for Reach RD2: Roadside Ditch 2

Inflow Area = 7.576 ac, 0.00% Impervious, Inflow Depth = 2.29" for 100-yr, 24-hr event

Inflow = 19.31 cfs @ 12.19 hrs, Volume= 1.445 af

Outflow = 17.96 cfs @ 12.25 hrs, Volume= 1.445 af, Atten= 7%, Lag= 3.9 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.14 fps, Min. Travel Time= 2.3 min

Avg. Velocity = 0.92 fps, Avg. Travel Time= 7.9 min

Peak Storage= 2,519 cf @ 12.21 hrs

Average Depth at Peak Storage= 0.52', Surface Width= 16.40' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 72.77 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 433.0' Slope= 0.0162 '/'

Inlet Invert= 814.55', Outlet Invert= 807.54'



## Summary for Reach RD3: Roadside Ditch 3

Inflow Area = 2.159 ac, 0.00% Impervious, Inflow Depth = 3.69" for 100-yr, 24-hr event

Inflow = 13.48 cfs @ 12.10 hrs, Volume= 0.663 af

Outflow = 11.77 cfs @ 12.19 hrs, Volume= 0.663 af, Atten= 13%, Lag= 5.2 min

Routed to Link C2: Culvert C2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.50 fps, Min. Travel Time= 3.0 min

Avg. Velocity = 1.47 fps, Avg. Travel Time= 9.3 min

Peak Storage= 2,151 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.81', Surface Width= 6.48'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 20.76 cfs

0.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 8.00'

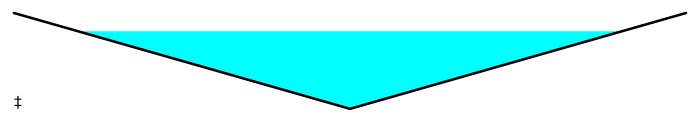
Length= 821.0' Slope= 0.0288 '/'

Inlet Invert= 841.47', Outlet Invert= 817.83'

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# Summary for Reach RD4: Roadside Ditch 4

Inflow Area = 1.671 ac, 0.00% Impervious, Inflow Depth = 1.53" for 100-yr, 24-hr event

Inflow = 3.98 cfs @ 12.11 hrs, Volume= 0.213 af

Outflow = 2.99 cfs @ 12.26 hrs, Volume= 0.213 af, Atten= 25%, Lag= 9.0 min

Routed to Link C9: Culvert C9

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.52 fps, Min. Travel Time= 5.4 min

Avg. Velocity = 0.44 fps, Avg. Travel Time= 18.6 min

Peak Storage= 991 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.24', Surface Width= 10.77' Bank-Full Depth= 1.00' Flow Area= 16.0 sf, Capacity= 54.26 cfs

6.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 10.0 '/' Top Width= 26.00'

Length= 495.6' Slope= 0.0090 '/'

Inlet Invert= 812.00', Outlet Invert= 807.54'



# Summary for Reach RD5: Roadside Ditch 5

Inflow Area = 0.616 ac, 0.00% Impervious, Inflow Depth = 3.48" for 100-yr, 24-hr event

Inflow = 3.67 cfs @ 12.10 hrs, Volume= 0.179 af

Outflow = 3.30 cfs @ 12.14 hrs, Volume= 0.179 af, Atten= 10%, Lag= 2.2 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.14 fps, Min. Travel Time= 1.2 min

Avg. Velocity = 1.52 fps, Avg. Travel Time= 3.2 min

Peak Storage= 238 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.46', Surface Width= 3.64'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 28.18 cfs

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59" Printed 8/28/2023

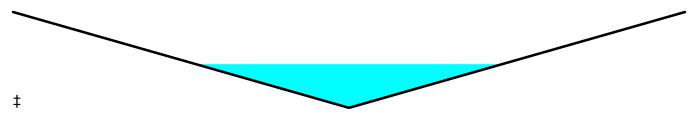
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0.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 8.00' Length= 288.0' Slope= 0.0531 '/' Inlet Invert= 841.47', Outlet Invert= 826.18'



# **Summary for Reach S1.1: Swale S1.1**

Inflow Area = 1.185 ac, 0.00% Impervious, Inflow Depth = 2.79" for 100-yr, 24-hr event

Inflow = 5.57 cfs @ 12.11 hrs, Volume= 0.275 af

Outflow = 4.95 cfs @ 12.16 hrs, Volume= 0.275 af, Atten= 11%, Lag= 3.2 min

Routed to Link C10: Culvert C10

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.87 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 0.80 fps, Avg. Travel Time= 6.7 min

Peak Storage= 581 cf @ 12.13 hrs

Average Depth at Peak Storage= 0.21', Surface Width= 9.64' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 338.34 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00' Length= 321.0' Slope= 0.0319 '/' Inlet Invert= 825.20', Outlet Invert= 814.96'



# Summary for Reach S1.2: Swale S1.2

Inflow Area = 11.287 ac, 0.00% Impervious, Inflow Depth = 3.06" for 100-yr, 24-hr event

Inflow = 52.19 cfs @ 12.14 hrs, Volume= 2.880 af

Outflow = 49.91 cfs @ 12.16 hrs, Volume= 2.880 af, Atten= 4%, Lag= 1.2 min

Routed to Link C3: Culvert C3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.24 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 1.15 fps, Avg. Travel Time= 2.9 min

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Peak Storage= 2,484 cf @ 12.15 hrs

Average Depth at Peak Storage= 1.02', Surface Width= 16.13' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 196.86 cfs

8.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 202.8' Slope= 0.0108 '/'

Inlet Invert= 813.36', Outlet Invert= 811.17'



# **Summary for Reach S1.3: Swale S1.3**

Inflow Area = 14.199 ac, 0.00% Impervious, Inflow Depth = 3.00" for 100-yr, 24-hr event

Inflow = 61.05 cfs @ 12.15 hrs, Volume= 3.550 af

Outflow = 60.33 cfs @ 12.15 hrs, Volume= 3.550 af, Atten= 1%, Lag= 0.4 min

Routed to Link C4: Culvert C4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.92 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 1.36 fps, Avg. Travel Time= 0.9 min

Peak Storage= 895 cf @ 12.15 hrs

Average Depth at Peak Storage= 1.03', Surface Width= 16.20'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 227.36 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 72.2' Slope= 0.0144 '/'

Inlet Invert= 810.90', Outlet Invert= 809.86'



# Summary for Reach S1.4: Swale S1.4

Inflow Area = 14.938 ac, 0.00% Impervious, Inflow Depth = 2.95" for 100-yr, 24-hr event

Inflow = 62.44 cfs @ 12.15 hrs, Volume= 3.671 af

Outflow = 59.56 cfs @ 12.18 hrs, Volume= 3.671 af, Atten= 5%, Lag= 1.4 min

Routed to Reach S1.6: Swale S1.6

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59"

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.24 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 0.92 fps, Avg. Travel Time= 2.7 min

Peak Storage= 2,814 cf @ 12.16 hrs

Average Depth at Peak Storage= 1.40', Surface Width= 19.19'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 126.50 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 148.0' Slope= 0.0045 '/'

Inlet Invert= 809.60', Outlet Invert= 808.94'



## **Summary for Reach S1.5: Swale S1.5**

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.63 cfs @ 12.15 hrs, Volume= 0.204 af

Outflow = 3.33 cfs @ 12.21 hrs, Volume= 0.204 af, Atten= 8%, Lag= 3.8 min

Routed to Reach S1.6: Swale S1.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.35 fps, Min. Travel Time= 2.2 min

Avg. Velocity = 0.36 fps, Avg. Travel Time= 8.2 min

Peak Storage= 448 cf @ 12.18 hrs

Average Depth at Peak Storage= 0.27', Surface Width= 10.19'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 134.06 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 179.7' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.95'



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# Summary for Reach S1.6: Swale S1.6

Inflow Area = 17.290 ac, 0.00% Impervious, Inflow Depth = 2.97" for 100-yr, 24-hr event

Inflow = 69.21 cfs @ 12.18 hrs, Volume= 4.277 af

Outflow = 66.73 cfs @ 12.21 hrs, Volume= 4.277 af, Atten= 4%, Lag= 2.1 min

Routed to Link C5: Culvert C5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.60 fps, Min. Travel Time= 1.2 min

Avg. Velocity = 0.98 fps, Avg. Travel Time= 4.3 min

Peak Storage= 4,825 cf @ 12.20 hrs

Average Depth at Peak Storage= 1.41', Surface Width= 19.25' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 139.68 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 252.0' Slope= 0.0054 '/'

Inlet Invert= 808.94', Outlet Invert= 807.57'

‡

# Summary for Reach S1.7: Swale S1.7

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 3.38" for 100-yr, 24-hr event

Inflow = 3.63 cfs @ 12.15 hrs, Volume= 0.201 af

Outflow = 3.32 cfs @ 12.20 hrs, Volume= 0.201 af, Atten= 9%, Lag= 3.3 min

Routed to Reach S1.6: Swale S1.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.20 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 0.63 fps, Avg. Travel Time= 6.5 min

Peak Storage= 380 cf @ 12.17 hrs

Average Depth at Peak Storage= 0.41', Surface Width= 4.98'

Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 90.14 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

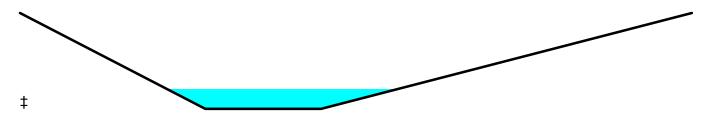
Length= 245.8' Slope= 0.0099 '/'

Inlet Invert= 810.00', Outlet Invert= 807.57'

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## Summary for Reach S1.8: Swale S1.8

Inflow Area = 20.082 ac, 0.00% Impervious, Inflow Depth = 2.89" for 100-yr, 24-hr event

Inflow = 74.10 cfs @ 12.22 hrs, Volume= 4.838 af

Outflow = 70.85 cfs @ 12.27 hrs, Volume= 4.838 af, Atten= 4%, Lag= 3.2 min

Routed to Reach S2.1: Swale S2.1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.09 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 1.09 fps, Avg. Travel Time= 6.5 min

Peak Storage= 7,485 cf @ 12.24 hrs

Average Depth at Peak Storage= 1.33', Surface Width= 18.65' Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 163.67 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 422.0' Slope= 0.0075 '/'

Inlet Invert= 807.15', Outlet Invert= 804.00'



# Summary for Reach S2.1: Swale S2.1

Inflow Area = 22.196 ac, 0.00% Impervious, Inflow Depth = 2.68" for 100-yr, 24-hr event

Inflow = 71.34 cfs @ 12.27 hrs, Volume= 4.949 af

Outflow = 68.62 cfs @ 12.32 hrs, Volume= 4.949 af, Atten= 4%, Lag= 3.2 min

Routed to Reach S2.2: Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.72 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 1.10 fps, Avg. Travel Time= 5.9 min

Peak Storage= 7,331 cf @ 12.29 hrs

Average Depth at Peak Storage= 1.73', Surface Width= 17.82'

Bank-Full Depth= 2.00' Flow Area= 24.0 sf, Capacity= 97.05 cfs

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59" Printed 8/28/2023

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4.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 20.00' Length= 389.0' Slope= 0.0054 '/' Inlet Invert= 806.10', Outlet Invert= 804.00'



# Summary for Reach S2.2: Swale S2.2

Inflow Area = 39.450 ac, 0.00% Impervious, Inflow Depth = 2.90" for 100-yr, 24-hr event

Inflow = 120.78 cfs @ 12.27 hrs, Volume= 9.518 af

Outflow = 118.35 cfs @ 12.32 hrs, Volume= 9.518 af, Atten= 2%, Lag= 3.1 min

Routed to Reach S2.3: Swale S2.3

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 3.96 fps, Min. Travel Time= 1.7 min

Avg. Velocity = 1.03 fps, Avg. Travel Time= 6.7 min

Peak Storage= 12,407 cf @ 12.29 hrs

Average Depth at Peak Storage= 1.77', Surface Width= 24.15' Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 152.61 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 26.00' Length= 411.0' Slope= 0.0049 '/' Inlet Invert= 804.00', Outlet Invert= 802.00'



# Summary for Reach S2.3: Swale S2.3

Inflow Area = 43.245 ac, 0.00% Impervious, Inflow Depth = 2.76" for 100-yr, 24-hr event

Inflow = 120.84 cfs @ 12.32 hrs, Volume= 9.932 af

Outflow = 119.37 cfs @ 12.35 hrs, Volume= 9.932 af, Atten= 1%, Lag= 1.7 min

Routed to Link C7: Culvert C7

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 5.64 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.45 fps, Avg. Travel Time= 3.5 min

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Peak Storage= 6,544 cf @ 12.33 hrs

Average Depth at Peak Storage= 1.38', Surface Width= 21.00'

Bank-Full Depth= 2.00' Flow Area= 36.0 sf, Capacity= 249.72 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 26.00'

Length= 307.0' Slope= 0.0130 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



# Summary for Reach S3.1: Swale S3.1

Inflow Area = 2.044 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 11.02 cfs @ 12.10 hrs, Volume= 0.542 af

Outflow = 9.75 cfs @ 12.19 hrs, Volume= 0.542 af, Atten= 12%, Lag= 5.2 min

Routed to Reach S3.2: Swale S3.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.94 fps, Min. Travel Time= 3.1 min

Avg. Velocity = 0.48 fps, Avg. Travel Time= 12.5 min

Peak Storage= 1,797 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.50', Surface Width= 12.02'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.76 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 357.0' Slope= 0.0050 '/'

Inlet Invert= 809.85', Outlet Invert= 808.07'



# Summary for Reach S3.2: Swale S3.2

Inflow Area = 8.141 ac, 0.00% Impervious, Inflow Depth = 3.19" for 100-yr, 24-hr event

Inflow = 38.61 cfs @ 12.12 hrs, Volume= 2.162 af

Outflow = 37.80 cfs @ 12.13 hrs, Volume= 2.162 af, Atten= 2%, Lag= 0.5 min

Routed to Reach S3.3: Swale S3.3

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.93 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.75 fps, Avg. Travel Time= 0.8 min

Peak Storage= 442 cf @ 12.12 hrs

Average Depth at Peak Storage= 1.06', Surface Width= 16.50'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 133.95 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 34.0' Slope= 0.0050 '/'

Inlet Invert= 798.00', Outlet Invert= 797.83'



# Summary for Reach S3.3: Swale S3.3

Inflow Area = 8.141 ac, 0.00% Impervious, Inflow Depth = 3.19" for 100-yr, 24-hr event

Inflow = 37.80 cfs @ 12.13 hrs, Volume= 2.162 af

Outflow = 36.67 cfs @ 12.15 hrs, Volume= 2.162 af, Atten= 3%, Lag= 1.5 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.12 fps, Min. Travel Time= 0.8 min

Avg. Velocity = 1.04 fps, Avg. Travel Time= 3.2 min

Peak Storage= 1,839 cf @ 12.14 hrs

Average Depth at Peak Storage= 0.82', Surface Width= 14.53'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 215.99 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 200.0' Slope= 0.0130 '/'

Inlet Invert= 808.00', Outlet Invert= 805.40'



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# Summary for Reach S3.4: Swale S3.4

Inflow Area = 0.769 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.63 cfs @ 12.15 hrs, Volume= 0.204 af

Outflow = 3.29 cfs @ 12.22 hrs, Volume= 0.204 af, Atten= 9%, Lag= 4.3 min

Routed to Link C6: Culvert C6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.95 fps, Min. Travel Time= 2.4 min

Avg. Velocity = 0.57 fps, Avg. Travel Time= 8.3 min

Peak Storage= 487 cf @ 12.18 hrs

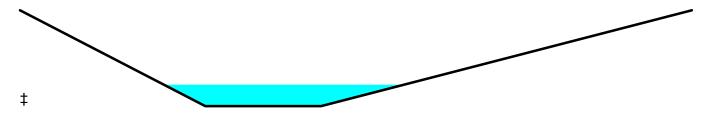
Average Depth at Peak Storage= 0.45', Surface Width= 5.19' Bank-Full Depth= 2.00' Flow Area= 17.0 sf, Capacity= 76.21 cfs

2.50' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 2.0 4.0 '/' Top Width= 14.50'

Length= 283.0' Slope= 0.0071 '/'

Inlet Invert= 810.00', Outlet Invert= 808.00'



# Summary for Reach S3.5: Swale S3.5

Inflow Area = 15.716 ac, 0.00% Impervious, Inflow Depth = 3.23" for 100-yr, 24-hr event

Inflow = 68.29 cfs @ 12.15 hrs, Volume= 4.234 af

Outflow = 64.19 cfs @ 12.21 hrs, Volume= 4.234 af, Atten= 6%, Lag= 3.7 min

Routed to Reach S2.2 : Swale S2.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.63 fps, Min. Travel Time= 2.0 min

Avg. Velocity = 0.68 fps, Avg. Travel Time= 7.8 min

Peak Storage= 7,904 cf @ 12.17 hrs

Average Depth at Peak Storage= 1.68', Surface Width= 21.48'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 93.14 cfs

8.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 318.5' Slope= 0.0024 '/'

Inlet Invert= 804.76', Outlet Invert= 803.99'

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# Summary for Reach S4.1: Swale S4.1

Inflow Area = 1.364 ac, 0.00% Impervious, Inflow Depth = 2.35" for 100-yr, 24-hr event

Inflow = 5.28 cfs @ 12.10 hrs, Volume= 0.267 af

Outflow = 4.68 cfs @ 12.16 hrs, Volume= 0.267 af, Atten= 11%, Lag= 3.3 min

Routed to Reach S4.2: Swale S4.2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.07 fps, Min. Travel Time= 1.9 min

Avg. Velocity = 0.50 fps, Avg. Travel Time= 8.0 min

Peak Storage= 556 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.22', Surface Width= 11.51' Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 70.22 cfs

10.00' x 1.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00'

Length= 240.0' Slope= 0.0153 '/'

Inlet Invert= 811.94', Outlet Invert= 808.26'

# ‡

# Summary for Reach S4.2: Swale S4.2

Inflow Area = 3.174 ac, 0.00% Impervious, Inflow Depth = 2.52" for 100-yr, 24-hr event

Inflow = 11.92 cfs @ 12.14 hrs, Volume= 0.667 af

Outflow = 11.06 cfs @ 12.18 hrs, Volume= 0.667 af, Atten= 7%, Lag= 2.8 min

Routed to Reach S4.3: Swale S4.3

Routing by Stor-Ind+Trans method. Time Span= 0.00-72.00 hrs. dt= 0.05 hrs

Max. Velocity= 2.70 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 0.62 fps, Avg. Travel Time= 7.0 min

Peak Storage= 1,118 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.38', Surface Width= 12.66'

Bank-Full Depth= 1.00' Flow Area= 13.5 sf, Capacity= 63.88 cfs

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10.00' x 1.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 17.00' Length= 259.3' Slope= 0.0127 '/' Inlet Invert= 808.26', Outlet Invert= 804.97'



# **Summary for Reach S4.3: Swale S4.3**

Inflow Area = 9.483 ac, 0.00% Impervious, Inflow Depth = 2.63" for 100-yr, 24-hr event

Inflow = 36.14 cfs @ 12.14 hrs, Volume= 2.080 af

Outflow = 33.37 cfs @ 12.21 hrs, Volume= 2.080 af, Atten= 8%, Lag= 4.5 min

Routed to Reach S4.4: Swale S4.4

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.26 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.54 fps, Avg. Travel Time= 11.3 min

Peak Storage= 5,406 cf @ 12.17 hrs Average Depth at Peak Storage= 1.08', Surface Width= 17.57' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 108.12 cfs

10.00' x 2.00' deep channel, n= 0.030 Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00' Length= 362.9' Slope= 0.0027 '/' Inlet Invert= 804.97', Outlet Invert= 804.00'



# Summary for Reach S4.4: Swale S4.4

Inflow Area = 14.158 ac, 0.00% Impervious, Inflow Depth = 2.63" for 100-yr, 24-hr event

Inflow = 45.34 cfs @ 12.17 hrs, Volume= 3.104 af

Outflow = 43.47 cfs @ 12.26 hrs, Volume= 3.104 af, Atten= 4%, Lag= 5.4 min

Routed to Reach S4.5: Swale S4.5

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 2.82 fps, Min. Travel Time= 2.9 min Avg. Velocity = 0.67 fps, Avg. Travel Time= 12.3 min

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Peak Storage= 7,633 cf @ 12.21 hrs

Average Depth at Peak Storage= 1.11', Surface Width= 17.77' Bank-Full Depth= 2.00' Flow Area= 34.0 sf, Capacity= 132.85 cfs

10.00' x 2.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 24.00'

Length= 495.6' Slope= 0.0040 '/'

Inlet Invert= 804.00', Outlet Invert= 802.00'



# Summary for Reach S4.5: Swale S4.5

Inflow Area = 16.268 ac, 0.00% Impervious, Inflow Depth = 2.55" for 100-yr, 24-hr event

Inflow = 45.91 cfs @ 12.26 hrs, Volume= 3.454 af

Outflow = 44.58 cfs @ 12.31 hrs, Volume= 3.454 af, Atten= 3%, Lag= 3.1 min

Routed to Reach S4.6: Swale S4.6

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 3.87 fps, Min. Travel Time= 1.8 min

Avg. Velocity = 0.98 fps, Avg. Travel Time= 7.0 min

Peak Storage= 4,792 cf @ 12.28 hrs

Average Depth at Peak Storage= 0.89', Surface Width= 16.22'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf, Capacity= 465.89 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 411.1' Slope= 0.0097 '/'

Inlet Invert= 802.00', Outlet Invert= 798.00'



# Summary for Reach S4.6: Swale S4.6

Inflow Area = 64.606 ac, 0.00% Impervious, Inflow Depth = 2.67" for 100-yr, 24-hr event

Inflow = 168.73 cfs @ 12.33 hrs, Volume= 14.356 af

Outflow = 165.72 cfs @ 12.38 hrs, Volume= 14.356 af, Atten= 2%, Lag= 2.6 min

Routed to Pond Sed Pond : Sedimentation Basin

MSE 24-hr 4 100-yr, 24-hr Rainfall=6.59" Printed 8/28/2023

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Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.02 fps, Min. Travel Time= 1.5 min

Avg. Velocity = 1.58 fps, Avg. Travel Time= 5.7 min

Peak Storage= 14,984 cf @ 12.35 hrs

Average Depth at Peak Storage= 1.74', Surface Width= 22.15'

Bank-Full Depth= 3.00' Flow Area= 61.5 sf, Capacity= 499.25 cfs

10.00' x 3.00' deep channel, n= 0.030

Side Slope Z-value= 4.0 3.0 '/' Top Width= 31.00'

Length= 537.0' Slope= 0.0112 '/'

Inlet Invert= 798.00', Outlet Invert= 792.00'



## Summary for Reach S5.1: Swale S5.1

Inflow Area = 1.218 ac, 0.00% Impervious, Inflow Depth = 3.69" for 100-yr, 24-hr event

Inflow = 7.33 cfs @ 12.11 hrs, Volume= 0.374 af

Outflow = 6.57 cfs @ 12.20 hrs, Volume= 0.374 af, Atten= 10%, Lag= 5.2 min

Routed to Link C2 : Culvert C2

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.49 fps, Min. Travel Time= 3.2 min

Avg. Velocity = 0.63 fps, Avg. Travel Time= 12.7 min

Peak Storage= 1,277 cf @ 12.15 hrs

Average Depth at Peak Storage= 0.29', Surface Width= 10.33'

Bank-Full Depth= 2.00' Flow Area= 32.0 sf, Capacity= 235.22 cfs

 $8.00' \times 2.00'$  deep channel, n= 0.030

Side Slope Z-value= 4.0 '/' Top Width= 24.00'

Length= 478.0' Slope= 0.0154 '/'

Inlet Invert= 825.20', Outlet Invert= 817.83'



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# **Summary for Pond C8: Culvert C8**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Outflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.1 min

Primary = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Routed to Reach S4.1: Swale S4.1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 811.38' @ 12.08 hrs Surf.Area= 0.000 ac Storage= 0.000 af Flood Elev= 819.00' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= 0.2 min calculated for 0.073 af (100% of inflow) Center-of-Mass det. time= 0.2 min (817.4 - 817.3)

Volume Invert Avail.Storage Storage Description #1 810.70' 0.001 af 3.00'D x 7.00'H Vertical Cone/Cylinder Device Routing Invert Outlet Devices #1 Primary 810.70' 12.0" Round Culvert L= 85.6' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 810.70' / 808.60' S= 0.0245 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.49 cfs @ 12.08 hrs HW=811.35' (Free Discharge) 1=Culvert (Inlet Controls 1.49 cfs @ 2.75 fps)

# **Summary for Pond N: North Infiltration Area**

Inflow Area = 19.122 ac, 0.00% Impervious, Inflow Depth = 1.47" for 100-yr, 24-hr event

Inflow = 26.07 cfs @ 12.26 hrs, Volume= 2.341 af

Outflow = 2.21 cfs @ 14.51 hrs, Volume= 2.341 af, Atten= 92%, Lag= 134.8 min

Primary = 2.21 cfs @ 14.51 hrs, Volume= 2.341 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 805.23' @ 14.51 hrs Surf.Area= 26,475 sf Storage= 49,470 cf

Plug-Flow detention time= 286.7 min calculated for 2.340 af (100% of inflow)

Center-of-Mass det. time= 286.7 min (1,155.0 - 868.3)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	802.00'	256,569 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
802.00	5,140	0	0
804.00	17,424	22,564	22,564
806.00	32,191	49,615	72,179
808.00	46,130	78,321	150,500
810.00	59.939	106.069	256.569

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Device	Routing	Invert	Outlet Devices
#1	Primary	802.00'	3.600 in/hr Exfiltration over Surface area

Primary OutFlow Max=2.21 cfs @ 14.51 hrs HW=805.23' (Free Discharge) 1=Exfiltration (Exfiltration Controls 2.21 cfs)

# **Summary for Pond Sed Pond: Sedimentation Basin**

Inflow Area = 71.936 ac, 2.10% Impervious, Inflow Depth = 2.59" for 100-yr, 24-hr event Inflow 169.96 cfs @ 12.38 hrs, Volume= 15.500 af 83.78 cfs @ 12.65 hrs, Volume= 15.500 af, Atten= 51%, Lag= 16.6 min Outflow 5.91 cfs @ 12.65 hrs, Volume= Discarded = 7.233 af Primary 11.51 cfs @ 12.65 hrs, Volume= 5.165 af Routed to Link Wetland: Wetland 31.49 cfs @ 12.65 hrs, Volume= 2.330 af Secondary = Routed to Link Wetland: Wetland 34.88 cfs @ 12.65 hrs, Volume= Tertiarv = 0.772 af Routed to Link Wetland: Wetland

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 793.20' @ 12.65 hrs Surf.Area= 70,892 sf Storage= 245,676 cf Flood Elev= 794.00' Surf.Area= 75,797 sf Storage= 304,443 cf

Plug-Flow detention time= 158.5 min calculated for 15.489 af (100% of inflow) Center-of-Mass det. time= 158.5 min (1,005.3 - 846.8)

Volume	Invert	Avail.Sto	rage	Storage D	escription	
#1	789.00'	304,4	43 cf	Custom S	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)	Sur	f.Area (sq-ft)	Inc.	Store -feet)	Cum.Store (cubic-feet)	
789.00	2	27,325		0	0	
790.00	5	55,972	4	1,649	41,649	
791.00	6	31,532	58	8,752	100,401	
792.00	6	5,703	63	3,618	164,018	
793.00	6	9,675	6	7,689	231,707	
794.00	7	75,797	72	2,736	304,443	
	Routing Primary	Invert 787.70'		t Devices  Round C	- Lulvort	
#I F	ııınaıy	101.10	L= 40	).0' RCP,	mitered to co	nform to fill, Ke= 0.700 787.50' S= 0.0050 '/' Cc= 0.900

			L= 40.0' RCP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 787.70' / 787.50' S= 0.0050 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf
#2	Device 1	791.00'	30.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	790.50'	<b>0.8" Vert. Orifice/Grate X 4.00</b> C= 0.600
			Limited to weir flow at low heads
#4	Device 1	790.00'	<b>0.8" Vert. Orifice/Grate X 4.00</b> C= 0.600
			Limited to weir flow at low heads
#5	Device 1	789.00'	0.5" Vert. Orifice/Grate X 14.00 columns
			X 6 rows with 6.0" cc spacing C= 0.600

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			Limited to weir flow at low heads
#6	Secondary	792.50'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#7	Tertiary	793.00'	158.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

**Discarded OutFlow** Max=5.91 cfs @ 12.65 hrs HW=793.20' (Free Discharge) **8=Exfiltration** (Exfiltration Controls 5.91 cfs)

Primary OutFlow Max=11.51 cfs @ 12.65 hrs HW=793.20' (Free Discharge) 1=Culvert (Inlet Controls 11.51 cfs @ 9.38 fps)

2=Orifice/Grate (Passes < 35.04 cfs potential flow)

**─3=Orifice/Grate** (Passes < 0.11 cfs potential flow)

**—4=Orifice/Grate** (Passes < 0.12 cfs potential flow)

-5=Orifice/Grate (Passes < 0.93 cfs potential flow)

Secondary OutFlow Max=31.42 cfs @ 12.65 hrs HW=793.20' (Free Discharge) 6=Broad-Crested Rectangular Weir (Weir Controls 31.42 cfs @ 2.25 fps)

Tertiary OutFlow Max=34.62 cfs @ 12.65 hrs HW=793.20' (Free Discharge) 7=Broad-Crested Rectangular Weir (Weir Controls 34.62 cfs @ 1.11 fps)

# Summary for Link C1: Culvert C1

Inflow Area = 3.035 ac, 0.00% Impervious, Inflow Depth = 1.20" for 100-yr, 24-hr event

Inflow = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af

Primary = 1.96 cfs @ 12.74 hrs, Volume= 0.304 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C10: Culvert C10

Inflow Area = 1.185 ac, 0.00% Impervious, Inflow Depth = 2.79" for 100-yr, 24-hr event

Inflow = 4.95 cfs @ 12.16 hrs, Volume= 0.275 af

Primary = 4.95 cfs @ 12.16 hrs, Volume= 0.275 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.2: Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C2: Culvert C2

Inflow Area = 3.377 ac, 0.00% Impervious, Inflow Depth = 3.69" for 100-yr, 24-hr event

Inflow = 18.32 cfs @ 12.19 hrs, Volume= 1.038 af

Primary = 18.32 cfs @ 12.19 hrs, Volume= 1.038 af, Atten= 0%, Lag= 0.0 min

Routed to Reach RD2: Roadside Ditch 2

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# Summary for Link C3: Culvert C3

Inflow Area = 11.287 ac, 0.00% Impervious, Inflow Depth = 3.06" for 100-yr, 24-hr event

Inflow = 49.91 cfs @ 12.16 hrs, Volume= 2.880 af

Primary = 49.91 cfs @ 12.16 hrs, Volume= 2.880 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C4: Culvert C4

Inflow Area = 14.199 ac, 0.00% Impervious, Inflow Depth = 3.00" for 100-yr, 24-hr event

Inflow = 60.33 cfs @ 12.15 hrs, Volume= 3.550 af

Primary = 60.33 cfs @ 12.15 hrs, Volume= 3.550 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.4: Swale S1.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C5: Culvert C5

Inflow Area = 17.290 ac, 0.00% Impervious, Inflow Depth = 2.97" for 100-yr, 24-hr event

Inflow = 66.73 cfs @ 12.21 hrs, Volume= 4.277 af

Primary = 66.73 cfs @ 12.21 hrs, Volume= 4.277 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.8: Swale S1.8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link C6: Culvert C6**

Inflow Area = 11.990 ac, 0.00% Impervious, Inflow Depth = 3.19" for 100-yr, 24-hr event

Inflow = 53.57 cfs @ 12.14 hrs, Volume= 3.184 af

Primary = 53.57 cfs @ 12.14 hrs, Volume= 3.184 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.5: Swale S3.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link C7: Culvert C7

Inflow Area = 46.337 ac, 0.00% Impervious, Inflow Depth = 2.78" for 100-yr, 24-hr event

Inflow = 123.92 cfs @ 12.34 hrs, Volume= 10.752 af

Primary = 123.92 cfs @ 12.34 hrs, Volume= 10.752 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.6: Swale S4.6

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# **Summary for Link C9: Culvert C9**

Inflow Area = 9.247 ac, 0.00% Impervious, Inflow Depth = 2.15" for 100-yr, 24-hr event

Inflow = 20.94 cfs @ 12.26 hrs, Volume= 1.658 af

Primary = 20.94 cfs @ 12.26 hrs, Volume= 1.658 af, Atten= 0%, Lag= 0.0 min

Routed to Pond N: North Infiltration Area

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F10: Flume 10**

Inflow Area = 0.275 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af

Primary = 1.60 cfs @ 12.08 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.0 min

Routed to Pond C8: Culvert C8

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F1A: Flume 1A**

Inflow Area = 1.853 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 9.08 cfs @ 12.13 hrs, Volume= 0.491 af

Primary = 9.08 cfs @ 12.13 hrs, Volume= 0.491 af, Atten= 0%, Lag= 0.0 min

Routed to Link F1B: Flume 1B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F1B: Flume 1B

Inflow Area = 4.089 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 20.35 cfs @ 12.13 hrs, Volume= 1.084 af

Primary = 20.35 cfs @ 12.13 hrs, Volume= 1.084 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.3: Swale S4.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F2A: Flume 2A

Inflow Area = 0.636 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.34 cfs @ 12.11 hrs, Volume= 0.169 af

Primary = 3.34 cfs @ 12.11 hrs, Volume= 0.169 af, Atten= 0%, Lag= 0.0 min

Routed to Link F2B: Flume 2B

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# Summary for Link F2B: Flume 2B

Inflow Area = 2.199 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 11.13 cfs @ 12.12 hrs, Volume= 0.583 af

Primary = 11.13 cfs @ 12.12 hrs, Volume= 0.583 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.4: Swale S4.4

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F3: Flume 3**

Inflow Area = 0.427 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 2.33 cfs @ 12.10 hrs, Volume= 0.113 af

Primary = 2.33 cfs @ 12.10 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S4.5: Swale S4.5

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F4A: Flume 4A**

Inflow Area = 1.209 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 5.87 cfs @ 12.13 hrs, Volume= 0.320 af

Primary = 5.87 cfs @ 12.13 hrs, Volume= 0.320 af, Atten= 0%, Lag= 0.0 min

Routed to Link F4B: Flume 4B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F4B: Flume 4B

Inflow Area = 3.092 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 15.35 cfs @ 12.13 hrs, Volume= 0.820 af

Primary = 15.35 cfs @ 12.13 hrs, Volume= 0.820 af, Atten= 0%, Lag= 0.0 min

Routed to Link C7: Culvert C7

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F5A: Flume 5A

Inflow Area = 0.237 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 1.28 cfs @ 12.10 hrs, Volume= 0.063 af

Primary = 1.28 cfs @ 12.10 hrs, Volume= 0.063 af, Atten= 0%, Lag= 0.0 min

Routed to Link F5B: Flume 5B

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# Summary for Link F5B: Flume 5B

Inflow Area = 0.712 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 3.77 cfs @ 12.11 hrs, Volume= 0.189 af

Primary = 3.77 cfs @ 12.11 hrs, Volume= 0.189 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S2.2: Swale S2.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F6A: Flume 6A

Inflow Area = 1.440 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 7.42 cfs @ 12.11 hrs, Volume= 0.382 af

Primary = 7.42 cfs @ 12.11 hrs, Volume= 0.382 af, Atten= 0%, Lag= 0.0 min

Routed to Link F6B: Flume 6B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F6B: Flume 6B

Inflow Area = 3.001 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 15.57 cfs @ 12.11 hrs, Volume= 0.795 af

Primary = 15.57 cfs @ 12.11 hrs, Volume= 0.795 af, Atten= 0%, Lag= 0.0 min

Routed to Link C6: Culvert C6

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F7A: Flume 7A

Inflow Area = 1.062 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 5.92 cfs @ 12.10 hrs, Volume= 0.281 af

Primary = 5.92 cfs @ 12.10 hrs, Volume= 0.281 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7B: Flume 7B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F7B: Flume 7B

Inflow Area = 3.407 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 18.00 cfs @ 12.11 hrs, Volume= 0.903 af

Primary = 18.00 cfs @ 12.11 hrs, Volume= 0.903 af, Atten= 0%, Lag= 0.0 min

Routed to Link F7C: Flume 7C

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# **Summary for Link F7C: Flume 7C**

Inflow Area = 5.640 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 29.56 cfs @ 12.11 hrs, Volume= 1.495 af

Primary = 29.56 cfs @ 12.11 hrs, Volume= 1.495 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S3.2: Swale S3.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link F8A: Flume 8A**

Inflow Area = 0.381 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 2.02 cfs @ 12.11 hrs, Volume= 0.101 af

Primary = 2.02 cfs @ 12.11 hrs, Volume= 0.101 af, Atten= 0%, Lag= 0.0 min

Routed to Link F8B: Flume 8B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F8B: Flume 8B

Inflow Area = 1.497 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 7.88 cfs @ 12.11 hrs, Volume= 0.397 af

Primary = 7.88 cfs @ 12.11 hrs, Volume= 0.397 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.3: Swale S1.3

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F9A: Flume 9A

Inflow Area = 2.445 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 11.65 cfs @ 12.15 hrs, Volume= 0.648 af

Primary = 11.65 cfs @ 12.15 hrs, Volume= 0.648 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9B: Flume 9B

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# Summary for Link F9B: Flume 9B

Inflow Area = 5.540 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 26.78 cfs @ 12.14 hrs, Volume= 1.468 af

Primary = 26.78 cfs @ 12.14 hrs, Volume= 1.468 af, Atten= 0%, Lag= 0.0 min

Routed to Link F9C: Flume 9C

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# Summary for Link F9C: Flume 9C

Inflow Area = 8.811 ac, 0.00% Impervious, Inflow Depth = 3.18" for 100-yr, 24-hr event

Inflow = 42.66 cfs @ 12.14 hrs, Volume= 2.335 af

Primary = 42.66 cfs @ 12.14 hrs, Volume= 2.335 af, Atten= 0%, Lag= 0.0 min

Routed to Reach S1.2: Swale S1.2

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

# **Summary for Link Wetland: Wetland**

Inflow Area = 71.936 ac, 2.10% Impervious, Inflow Depth = 1.38" for 100-yr, 24-hr event

Inflow = 77.88 cfs @ 12.65 hrs, Volume= 8.267 af

Primary = 77.88 cfs @ 12.65 hrs, Volume= 8.267 af, Atten= 0%, Lag= 0.0 min



SCS ENGINE	ERS	Sheet No:	1 of 3
2 5 5 5 5 5 5 5 5 5		Calc. No.	
		Rev. No.	2
Job No. 25222260.00	Project: Columbia Energy Center MOD 12-13	By: SJL	Date: 8/28/23
Client: WPL	Subject: Swale Sizing	Chk'd: RJG	Date: 8/28/23

#### Purpose:

To size the proposed swales f to accommodate the 25-year, 24-hour storm event and determine required erosion matting.

- 1. WisDOT Facilities Development Manual Chapter 13, Section 30-15 Grass Lined Channels.
- 2. Design of Roadside Channels with Flexible Linings, HEC-15, USDOT FHWA.
- 3. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 4. Wisconsin Department of Natural Resources Conservation Practice Standard 1053 Channel Erosion Mat.

#### Approach:

Use the HydroCAD Model results to obtain the peak flow during a 25-year, 24-hour storm event.

Use Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2 (from Reference #1) to size the swale for each design swale cross section. The WisDOT spreadsheet incorporates the design guidelines and equations described in "Design of Roadside Channels with Flexible Linings", HEC-15, USDOT FHWA (Reference #2).

Confirm the swale is stable and has enough capacity for the design flow rate.

Use Standard 1053 (see Reference #4) to select appropriate erosion control mat based on shear stress and application.

#### **Assumptions:**

- 1. Swales geometry shown on the drawing set.
- 2. Assume the following parameters per Section 15.2 Grass Lining Properties from Reference #1:

Vegetation Retardance Class = C for Swales

Vegetation Condition = Good

Vegetation Growth Form = Turf

3. Assume cohesive soil type with ASTM Soil Class SC and a Plasticity Index (PI) of 16.

#### Calculations:

From the HydroCAD Report, the 25-year, 24-hour peak discharge rates in the swales are

Swales:	25-year		25-year		25-year
Swale \$1.1 =	3.1 cfs	Swale \$2.1 =	39.6 cfs	Swale \$3.5 =	39.6 cfs
Swale \$1.2 =	30.3 cfs	Swale \$2.2 =	65.6 cfs	Swale \$4.1 =	2.7 cfs
Swale \$1.3 =	34.7 cfs	Swale \$2.3 =	65.5 cfs	Swale \$4.2 =	6.3 cfs
Swale \$1.4 =	35.1 cfs	Swale \$3.1 =	6.5 cfs	Swale \$4.3 =	19.2 cfs
Swale \$1.5 =	2.1 cfs	Swale \$3.2 =	22.3 cfs	Swale \$4.4 =	22.5 cfs
Swale \$1.6 =	39.2 cfs	Swale \$3.3 =	21.8 cfs	Swale \$4.5 =	22.6 cfs
Swale \$1.7 =	2.2 cfs	Swale \$3.4 =	2.1 cfs	Swale \$4.6 =	89.3 cfs
Swale \$1.8 =	41.3 cfs			Swale \$5.1 =	4.6 cfs
Roadside Ditch 1 =	0.6 cfs	Roadside Ditch 2 =	11.2 cfs	Roadside Ditch 3 =	8.4 cfs
Roadside Ditch 4 =	1.5 cfs	Roadside Ditch 5 =	2.3 cfs		

Use the WisDOT Grass Swale Design Spreadsheet (Page 2) to determine the flow depth, velocity and shear stress in the swales.

#### Results:

The swales are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

The swales are stable at the design flow rates.

Use Class I, Type B erosion mat for all swales except Roadside Ditch 3 and 5 should be Class II, Type B if regraded.

0.006993007

Channel/Ditch Geometry	Swale S1.1	Swale S1.2	Swale S1.3	Swale S1.4	Swale S1.5	Swale S1.6	Swale S1.7	Swale S1.8	Swale S2.1	Swale S2.2	Swale S2.3	Swale S3.1	Swale S3.2	Swale S3.3	Swale S3.4 =	Swale S3.5 =	Swale S4.1 =	Swale \$4.2 =	Swale S4.3 =	Swale S4.4 =	Swale S4.5 =	Swale S4.6 =	Swale S5.1 =	Roadside Ditch 1 =	Roadside Ditch 2 =	Roadside Ditch 3 =	Roadside Ditch 4 =	Roadside Ditch 5 =
Channel Slope, S _c (ft/ft)	0.0319	0.0108	0.0144	0.0045	0.0054	0.005	0.005	0.005	0.0054	0.0049	0.0130	0.0050	0.0050	0.0130	0.0071	0.0090	0.0153	0.0127	0.0027	0.0040	0.0097	0.0112	0.0154	0.0188	0.0162	0.0288	0.0090	0.0531
Channel Bottom Width, B (ft)	8	8	8	8	8	8	2.5	8	4	10	10	8	8	8	2.5	8	10	10	10	10	10	10	8	4	6	0	6	0
Channel Side Slope, z _i	4	4	4	4	4	4	2	4	4	4	4	4	4	4	2	4	4	4	4	4	4	4	4	3	10	4	10	4
Channel Side Slope, z ₂	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	3	3	3	3	3	3	4	8	10	4	10	4
Flow Depth, d (ft) Solve iteratively	0.25	1.14	1.08	1.73	0.45	1.73	0.75	1.77	2.00	2.02	1.37	0.78	1.36	0.91	0.65	1.38	0.29	0.47	1.52	1.38	0.97	1.70	0.41	0.20	0.63	0.90	0.33	0.49
Safety Factor, SF	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Vegetation/Soil Parameters																												
Vegetation Retardance Class	U	С	c	С	С	С	С	С	С	С	C	С	c	С	С	С	С	С	С	С	С	С	С	С	С	С	С	С
Vegetation Condition	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good	good
Vegetation Growth Form	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf	turf
Soil Type	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive	cohesive
D ₇₅ (in) (Set at 0.00 for cohesive soils)		- 00								00	00			00			00		- 00				00		00	00		
ASTM Soil Class Plasticity Index. Pl	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16	SC 16
	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16	16
Results Summary	2.4	30.3	34.7	35.1	2.1	39.2		41.3	39.6	65.6	65.5		22.3	21.8	0.4	39.6	2.7		19.2	22.5	22.6	89.3	4.6		44.0	8.4	1.5	
Design Q (ft ³ /s)  1. Swales geometry shown on the drawing se	3.1	30.3	35.0	35.6	2.1	39.2	2.2	41.3	39.6	65.3	65.3	6.5 6.6	22.3	21.8	2.1	39.6	2.8	6.3 6.3	19.2	22.2	23.0	90.0	4.6	0.6	11.2	8.4	1.5	2.3
Difference Between Design & Calc. Flow (%)	3.2	0.4%	0.8%	1.3%	1.4%	0.7%	-1.4%	41.3	J9.5	00.3	-0.3%	0.8%	0.9%	0.2%	-0.4%	0.0%	0.4%	0.4%	1 5%	-1 1%	1.8%		4.0	-1.5%		-0.4 -0.2%	-1.7%	0.4%
	YES	YES	YES	YES	YES	YES	-1.4% YES	YES	YES	+0.6% YES	+0.3% YES	YES	YES	YES	+0.4% YES	YES	YES	YES	YES	-1.176 YES	YES	0.8% YES	+0.2% YES	-1.5% YES	1.2% YES	+0.2% YES	-1.7% YES	YES
Stable (Yes or No)	169	TES	TES	169	169	159	159	169	TES	159	169	159	169	TES	TES	TES	169	TES	TES	TES	169	169	TES	TES	169	169	169	TES
Channel Parameters																												
Vegetation Height, h (ft)	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Grass Roughness Coefficient, C,	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238	0.238
Cover Factor, C ₁	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Noncohesive Soil Soil Grain Roughness, n.	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016
Permissible Soil Shear Stress, t, (lb/ft²)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Cohesive Soil																												
Porosity, e	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Soil Coefficient 1, c ₁	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700
Soil Coefficient 2, c ₂	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30	14.30
Soil Coefficient 3, c ₃	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700	47.700
Soil Coefficient 4, c _q	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42	1.42
Soil Coefficient 5, o ₅	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61	-0.61
Soil Coefficient 6, c ₆	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010	0.00010
Permissible Soil Shear Stress, τ _o (lb/ft²) Total Permissible Shear Stress, τ _o (lb/ft²)	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080	0.080
Cross Sectional Area, A (ff²)	2.250	14,318	13.306	25.812	4.410	25.812	3.563	26.692	24.000	36.522	21,208	8.674	18,278	10.592	2.893	18.658	3.194	5.473	23.286	20.465	12.993	27.115	3.952	0.089	7.749	3.240	3.069	0.080
Wetted Perimeter, P (ft)	10.06	17.40	16.91	22.27	11.71	22.27	7.27	22.60	20.49	26.66	21.30	14.43	19.21	15.50	6.63	19.38	12.11	13.42	21.07	20.05	17.07	22.39	11.38	6.19	18.66	7.42	12.63	4.04
Hydraulic Radius, R (ft)	0.224	0.823	0.787	1.159	0.377	1,159	0.490	1.181	1.171	1.370	0.996	0.601	0.951	0.683	0.436	0.963	0.264	0.408	1.105	1.021	0.761	1.211	0.347	0.160	0.415	0.437	0.243	0.238
Top Width, T (ft)	10.00	17.12	16.64	21.84	11.60	21.84	7.00	22.16	20.00	26.16	20.96	14.24	18.88	15.28	6.40	19.04	12.03	13.29	20.64	19.66	16.79	21.90	11.28	6.15	18.60	7.20	12.60	3.92
Hydraulic Depth, D (ft)	0.225	0.836	0.800	1.182	0.380	1.182	0.509	1.204	1.200	1.396	1.012	0.609	0.968	0.693	0.452	0.980	0.266	0.412	1.128	1.041	0.774	1.238	0.350	0.161	0.417	0.450	0.244	0.245
Froude Number (Q design)	0.520	0.409	0.518	0.224	0.141	0.248	0.150	0.249	0.264	0.267	0.540	0.171	0.220	0.436	0.193	0.377	0.295	0.317	0.139	0.187	0.355	0.526	0.344	0.267	0.400	0.683	0.174	0.838
Channel Shear Stress, τ _o (lb/ft²)	0.45	0.55	0.71	0.33	0.13	0.36	0.15	0.37	0.39	0.42	0.81	0.19	0.30	0.55	0.19	0.54	0.25	0.32	0.19	0.25	0.46	0.85	0.33	0.19	0.42	0.78	0.14	0.79
Actual Sheer Stress, τ _d (lb/ft²)	0.50	0.77	0.97	0.49	0.15	0.54	0.23	0.55	0.67	0.62	1.11	0.24	0.42	0.74	0.29	0.78	0.28	0.37	0.26	0.34	0.59	1.19	0.39	0.23	0.64	1.62	0.19	1.62
Mannings n	0.070	0.064	0.058	0.080	0.116	0.076	0.108	0.076	0.074	0.072	0.055	0.099	0.083	0.064	0.098	0.065	0.088	0.080	0.099	0.088	0.069	0.054	0.079	0.099	0.072	0.056	0.113	0.056
Average Velocity, V (ft/s)	1.39	2.12	2.61	1.36	0.49	1.52	0.61	1.55	1.65	1.80	3.09	0.75	1.22	2.05	0.74	2.12	0.86	1.15	0.82	1.10	1.74	3.29	1.16	0.62	1.45	2.60	0.50	2.34
Calculated Flow, Q (ft ² /s)	3.2	30.4	35.0	35.6	2.2	39.5	2.2	41.3	39.5	65.3	65.3	6.6	22.4	21.8	2.1	39.6	2.8	6.3	19.5	22.2	23.0	90.0	4.6	0.6	11.4	8.4	1.5	2.3
Difference Between Design & Calc. Flow (%)	1.0%	0.4%	0.8%	1.3%	1.4%	0.7%	-1.4%	0.2%	-0.3%	-0.6%	-0.3%	0.8%	0.9%	0.2%	-0.4%	0.0%	0.4%	0.4%	1.5%	-1.1%	1.8%	0.8%	-0.2%	-1.5%	1.2%	-0.2%	-1.7%	0.4%
Effective Shear on Soil Surface,τ _e (lb/ft²)	0.003	0.005	0.007	0.002	0.000 42.11	0.002 18.08	0.001 36.51	0.002 18.08	0.003	0.003 16.22	0.009	0.001 30.67	0.002 21.56	0.005	0.001 30.06	0.005	0.001	0.001 20.03	0.001 30.67	0.001	0.003	0.010 9.13	0.002 19.53	0.001 30.67	0.003	0.013 9.81	0.000 39.96	0.013 9.81
Total Permissible Shear on Veg., τ _{evec} (lb/ft ² ) Stable (Y or N)	15.34 YES	12.82 YES	10.53 YES	20.03 YES	42.11 YES	18.08 YES	36.51 YES	18.08 YES	17.14 YES	16:22 YES	9.47 YES	30.67 YES	21.56 YES	12.82 YES	30.06 YES	13.22 YES	24.24 YES	20.03 YES	30.67 YES	24.24 YES	14.90 YES	9.13 YES	19.53 YES	30.67 YES	16.22 YES	9.81 YES	39.96 YES	9.81 YES
Stable (1 OF N)	iES	IES	163	IES	IES	IES	IES	169	169	169	iES	iES	163	IES	169	169	153	IES	IES	169	169	165	IES	169	169	165	169	169

Source: Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2

## SCS ENGINEERS

Sheet No:	3 of 3
Calc. No.	
Rev. No.	2
By: SJL	Date: 8/28/23
Chk'd: RIG	Date: 8/28/23

Job No. 25220183.00 Project: Job: Columbia Energy Center
Client: WPL Subject: Subject: Swale Sizing

## **Channel Erosion Mat**

(1053)

Wisconsin Department of Natural Resources Conservation Practice Standard

To differentiate applications WisDOT organizes erosion mats into three classes of mats, which are further broken down into various Types.

- A. Class I: A short-term duration (minimum of 6 months), light duty, organic ECRM with plastic or biodegradable netting.
  - Type A Only suitable for slope applications, not channel applications.
  - Type B Double netted product for use in channels where the calculated (design) shear stress is 1.5 lbs/ft² or less.
- B. Class II: A long-term duration (three years or greater), organic ECRM.
  - Type A Jute fiber only for use in channels to reinforce sod.
  - Type B For use in channels where the calculated (design) shear stress is 2.0 lbs/ft² or less. Made with plastic or biodegradable mat.
  - Type C A woven mat of 100% organic material for use in channels where the calculated (design) shear stress is 2.0 lbs/ft² or less. Applicable

for use in environmentally sensitive areas where plastic netting is inappropriate.

- C. Class III: A permanent 100% synthetic ECRM or TRM. Class I, Type B erosion mat or Class II, Type B or C erosion mat must be placed over a soil filled TRM.
  - Type A An ECRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft² or less.
  - Type B A TRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft² or less.
  - Type C A TRM for use in channels where the calculated (design) shear stress of 3.5 lbs/ft² or less.
  - Type D A TRM for use in channels where the calculated (design) shear stress of 5.0 lbs/ft² or less.



SCS ENG	Sheet No: 1 of 21					
2 2 2 2 2 2		Calc. No.				
		Rev. No.	1			
lob No. 25222260.00	Job: Columbia Energy Center MOD 12-13	By: SJL	Date: 8/28/23			
Client: WPL	Subject: Culvert Sizing	Chk'd: RJG	Date: 8/28/23			

#### Purpose:

To size the post closure culverts to accommodate the 25-year, 24-hour storm event.

#### References:

- 1. HY-8 7.40 Computer Model
- 2. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 3. Figure 1 Final Grades (Module 13)

#### Approach:

- 1. Create culvert crossing in HY-8 and input data from Reference #2 and #3.
- 2. Adjust diameter size and number of culverts in model until design flow does not over top berm/road crossing.

#### **Assumptions:**

- 1. Assume the tailwater channel data is a based on discharge swale or rock chute geometry (Reference #2).
- 2. Culverts are circular, PE Pipe with smooth interior, and with square edge with headwall.
- 3. Culvert elevatons, lengths, and slopes based on Figure 1 (Reference #3).
- 4. Roadway data for crossing based on Figure 1 (Reference #3).
- 5. Discharge flows from HydroCAD report (Refence #2).

#### Calculations:

See attached HY-8 Model output reports for C1 through C10.

#### Results:

The culverts are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

Culvert	Dia. (ft)	# of Barrels	Upstream Invert (ft)	Downstream Invert (ft)	Slope (%)	Length (ft)
C1	2	1	815.70	814.55	2,22	52
C2	2	2	817.83	814.00	5.14	75
C3	2.5	2	811.1 <i>7</i>	810.90	0.54	50
C4	2.5	2	809.86	809.60	0.52	50
C5	2.5	2	807.57	807.15	0.87	49
C6	2	2	805.40	804.76	0.61	105
C7	3.5	2	796.64	796.34	0.50	60
C8	1	1	810.70	808.60	2.45	86
C9	2	1	807.54	806.81	0.73	100
C10	2	2	814.96	813.36	3.20	50

# **Culvert Data: Culvert C1**

#### Site Data - Culvert C1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 815.70 ft Outlet Station: 51.88 ft Outlet Elevation: 814.55 ft Number of Barrels: 1

## **Culvert Data Summary - Culvert C1**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 2 - Culvert Summary Table: Culvert C1** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.60 cfs	0.60 cfs	816.05	0.35	0.0*	1-JS1t	0.18	0.27	1.06	0.29	0.36	1.23
0.74 cfs	0.74 cfs	816.09	0.39	0.0*	1-S2n	0.20	0.29	0.20	0.31	4.64	1.29
0.87 cfs	0.87 cfs	816.13	0.43	0.0*	1-JS1t	0.21	0.32	1.10	0.33	0.49	1.35
1.01 cfs	1.01 cfs	816.16	0.46	0.0*	1-JS1t	0.23	0.35	1.12	0.35	0.56	1.40
1.14 cfs	1.14 cfs	816.19	0.49	0.0*	1-JS1t	0.24	0.37	1.13	0.36	0.62	1.44
1.28 cfs	1.28 cfs	816.22	0.52	0.005	1-JS1t	0.26	0.39	1.15	0.38	0.69	1.48
1.42 cfs	1.42 cfs	816.25	0.55	0.021	1-JS1t	0.27	0.41	1.16	0.39	0.75	1.52
1.55 cfs	1.55 cfs	816.28	0.58	0.036	1-JS1t	0.28	0.43	1.18	0.41	0.81	1.55
1.69 cfs	1.69 cfs	816.30	0.60	0.050	1-JS1t	0.29	0.45	1.19	0.42	0.87	1.59
1.82 cfs	1.82 cfs	816.33	0.63	0.064	1-JS1t	0.30	0.47	1.20	0.43	0.92	1.62
1.96 cfs	1.96 cfs	816.35	0.65	0.078	1-JS1t	0.31	0.49	1.22	0.45	0.98	1.65

^{*} Full Flow Headwater elevation is below inlet invert.

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 815.70 ft, Outlet Elevation (invert): 814.55 ft

Culvert Length: 51.89 ft, Culvert Slope: 0.0222

**Tailwater Data for Crossing: Culvert C1** 

Table 3 - Downstream Channel Rating Curve (Crossing: Culvert C1)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
0.60	815.61	0.29	1.23	0.34	0.57
0.74	815.63	0.31	1.29	0.36	0.58
0.87	815.65	0.33	1.35	0.39	0.59

1.01	815.67	0.35	1.40	0.41	0.59
1.14	815.68	0.36	1.44	0.43	0.60
1.28	815.70	0.38	1.48	0.45	0.60
1.42	815.71	0.39	1.52	0.46	0.60
1.55	815.73	0.41	1.55	0.48	0.61
1.69	815.74	0.42	1.59	0.49	0.61
1.82	815.75	0.43	1.62	0.51	0.61
1.96	815.77	0.45	1.65	0.52	0.62

## **Tailwater Channel Data - Culvert C1**

Tailwater Channel Option: Triangular Channel

Side Slope (H:V): 6.00 (_:1) Channel Slope: 0.0188 Channel Manning's n: 0.0450 Channel Invert Elevation: 815.32 ft

**Roadway Data for Crossing: Culvert C1** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 819.06 ft Roadway Surface: Gravel Roadway Top Width: 30.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0.60 cfs Design Flow: 0.60 cfs Maximum Flow: 1.96 cfs

Table 4 - Summary of Culvert Flows at Crossing: Culvert C1

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert C1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
816.05	0.60	0.60	0.00	1
816.09	0.74	0.74	0.00	1
816.13	0.87	0.87	0.00	1
816.16	1.01	1.01	0.00	1
816.19	1.14	1.14	0.00	1
816.22	1.28	1.28	0.00	1
816.25	1.42	1.42	0.00	1
816.28	1.55	1.55	0.00	1
816.30	1.69	1.69	0.00	1
816.33	1.82	1.82	0.00	1
816.35	1.96	1.96	0.00	1
819.06	22.61	22.61	0.00	Overtopping

# **Culvert Data: C2**

#### Site Data - C2

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 817.83 ft Outlet Station: 74.54 ft Outlet Elevation: 814.00 ft Number of Barrels: 2

# **Culvert Data Summary - C2**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 3 - Culvert Summary Table: C2** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
10.95 cfs	10.95 cfs	818.96	1.13	0.593	1-JS1f	0.42	0.83	2.00	0.49	1.74	2.24
11.69 cfs	11.62 cfs	819.00	1.17	0.625	1-JS1f	0.44	0.85	2.00	0.51	1.85	2.29
12.42 cfs	11.95 cfs	819.02	1.19	0.649	1-JS1f	0.44	0.86	2.00	0.53	1.90	2.34
13.16 cfs	12.19 cfs	819.04	1.21	0.672	1-JS1f	0.45	0.87	2.00	0.54	1.94	2.38
13.90 cfs	12.40 cfs	819.05	1.22	0.693	1-JS1f	0.45	0.88	2.00	0.56	1.97	2.42
14.63 cfs	12.59 cfs	819.06	1.23	0.714	1-JS1f	0.45	0.89	2.00	0.58	2.00	2.46
15.37 cfs	12.77 cfs	819.07	1.24	0.734	1-JS1f	0.46	0.89	2.00	0.59	2.03	2.50
16.11 cfs	12.94 cfs	819.09	1.26	0.754	1-JS1f	0.46	0.90	2.00	0.61	2.06	2.54
16.85 cfs	13.10 cfs	819.10	1.27	0.773	1-JS1f	0.46	0.91	2.00	0.62	2.09	2.57
17.58 cfs	13.26 cfs	819.10	1.27	0.791	1-JS1f	0.47	0.91	2.00	0.64	2.11	2.61
18.32 cfs	13.41 cfs	819.11	1.28	0.810	1-JS1f	0.47	0.92	2.00	0.65	2.13	2.64

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 817.83 ft, Outlet Elevation (invert): 814.00 ft

Culvert Length: 74.64 ft, Culvert Slope: 0.0514

# **Tailwater Data for Crossing: Culvert C2**

Table 5 - Downstream Channel Rating Curve (Crossing: Culvert C2)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
10.95	818.32	0.49	2.24	0.47	0.62
11.69	818.34	0.51	2.29	0.49	0.62
12.42	818.36	0.53	2.34	0.51	0.62

13.16	818.37	0.54	2.38	0.52	0.63
13.90	818.39	0.56	2.42	0.54	0.63
14.63	818.41	0.58	2.46	0.55	0.63
15.37	818.42	0.59	2.50	0.57	0.63
16.11	818.44	0.61	2.54	0.58	0.64
16.85	818.45	0.62	2.57	0.60	0.64
17.58	818.47	0.64	2.61	0.61	0.64
18.32	818.48	0.65	2.64	0.63	0.64

# **Tailwater Channel Data - Culvert C2**

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0154 Channel Manning's n: 0.0450 Channel Invert Elevation: 817.83 ft

## **Roadway Data for Crossing: Culvert C2**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 819.00 ft Roadway Surface: Gravel Roadway Top Width: 20.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 10.95 cfs Design Flow: 10.95 cfs Maximum Flow: 18.32 cfs

Table 6 - Summary of Culvert Flows at Crossing: Culvert C2

Headwater Elevation (ft)	Total Discharge (cfs)	C2 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
818.96	10.95	10.95	0.00	1
819.00	11.69	11.62	0.01	13
819.02	12.42	11.95	0.43	6
819.04	13.16	12.19	0.93	5
819.05	13.90	12.40	1.48	5
819.06	14.63	12.59	2.01	4
819.07	15.37	12.77	2.58	4
819.09	16.11	12.94	3.15	4
819.10	16.85	13.10	3.73	4
819.10	17.58	13.26	4.32	4
819.11	18.32	13.41	4.91	4
819.00	11.61	11.61	0.00	Overtopping

# Culvert Data: C3

#### Site Data - C3

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 811.17 ft Outlet Station: 50.00 ft Outlet Elevation: 810.90 ft Number of Barrels: 2

## **Culvert Data Summary - C3**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 4 - Culvert Summary Table: C3** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
28.58 cfs	28.58 cfs	813.05	1.88	1.252	1-S2n	1.16	1.27	1.16	0.91	6.38	2.71
30.71 cfs	30.71 cfs	813.13	1.96	1.339	1-S2n	1.20	1.32	1.21	0.94	6.50	2.77
32.85 cfs	32.85 cfs	813.22	2.05	1.428	1-S2n	1.25	1.37	1.26	0.98	6.61	2.82
34.98 cfs	34.98 cfs	813.30	2.13	1.518	1-S2n	1.30	1.42	1.31	1.01	6.72	2.88
37.11 cfs	37.11 cfs	813.38	2.21	1.609	1-S2n	1.35	1.46	1.36	1.04	6.82	2.93
39.24 cfs	39.24 cfs	813.46	2.29	1.702	1-S2n	1.40	1.50	1.40	1.07	6.92	2.98
41.38 cfs	41.38 cfs	813.54	2.37	1.796	1-S2n	1.44	1.54	1.45	1.10	7.01	3.02
43.51 cfs	43.51 cfs	813.62	2.45	1.892	1-S2n	1.49	1.59	1.50	1.13	7.10	3.07
45.64 cfs	45.64 cfs	813.71	2.54	1.990	5-S2n	1.54	1.63	1.54	1.16	7.17	3.11
47.78 cfs	47.78 cfs	813.79	2.62	2.089	5-S2n	1.59	1.66	1.59	1.19	7.25	3.15
49.91 cfs	49.09 cfs	813.85	2.68	2.152	5-S2n	1.62	1.69	1.62	1.22	7.29	3.19

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 811.17 ft, Outlet Elevation (invert): 810.90 ft

Culvert Length: 50.00 ft, Culvert Slope: 0.0054

## **Tailwater Data for Crossing: Culvert C3**

**Table 7 - Downstream Channel Rating Curve (Crossing: Culvert C3)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
28.58	812.08	0.91	2.71	0.28	0.57
30.71	812.12	0.94	2.77	0.29	0.58
32.85	812.15	0.98	2.82	0.30	0.58

34.98	812.18	1.01	2.88	0.32	0.58
37.11	812.22	1.04	2.93	0.33	0.59
39.24	812.25	1.07	2.98	0.33	0.59
41.38	812.28	1.10	3.02	0.34	0.59
43.51	812.31	1.13	3.07	0.35	0.59
45.64	812.34	1.16	3.11	0.36	0.59
47.78	812.36	1.19	3.15	0.37	0.60
49.91	812.39	1.22	3.19	0.38	0.60

## **Tailwater Channel Data - Culvert C3**

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0300 Channel Invert Elevation: 811.17 ft

**Roadway Data for Crossing: Culvert C3** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.00 ft Crest Elevation: 813.80 ft Roadway Surface: Gravel Roadway Top Width: 30.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 28.58 cfs Design Flow: 28.58 cfs Maximum Flow: 49.91 cfs

Table 8 - Summary of Culvert Flows at Crossing: Culvert C3

Headwater Elevation (ft)	Total Discharge (cfs)	C3 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
813.05	28.58	28.58	0.00	1
813.13	30.71	30.71	0.00	1
813.22	32.85	32.85	0.00	1
813.30	34.98	34.98	0.00	1
813.38	37.11	37.11	0.00	1
813.46	39.24	39.24	0.00	1
813.54	41.38	41.38	0.00	1
813.62	43.51	43.51	0.00	1
813.71	45.64	45.64	0.00	1
813.79	47.78	47.78	0.00	1
813.85	49.91	49.09	0.77	7
813.80	47.90	47.90	0.00	Overtopping

# **Culvert Data: C4**

#### Site Data - C4

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 809.86 ft Outlet Station: 50.00 ft Outlet Elevation: 809.60 ft Number of Barrels: 2

## **Culvert Data Summary - C4**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 7 - Culvert Summary Table: C4** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
34.14 cfs	34.14 cfs	811.96	2.10	1.492	1-S2n	1.30	1.40	1.30	0.91	6.60	3.22
36.76 cfs	36.76 cfs	812.06	2.20	1.604	1-S2n	1.36	1.45	1.36	0.95	6.72	3.29
39.38 cfs	39.38 cfs	812.16	2.30	1.718	1-S2n	1.41	1.51	1.42	0.98	6.84	3.36
42.00 cfs	42.00 cfs	812.26	2.40	1.834	1-S2n	1.47	1.56	1.48	1.02	6.94	3.42
44.62 cfs	44.62 cfs	812.36	2.50	1.953	1-S2n	1.53	1.61	1.54	1.05	7.04	3.48
47.23 cfs	47.23 cfs	812.46	2.60	2.074	5-S2n	1.59	1.65	1.60	1.08	7.13	3.54
49.85 cfs	49.85 cfs	812.57	2.71	2.198	5-S2n	1.66	1.70	1.66	1.11	7.23	3.59
52.47 cfs	52.47 cfs	812.68	2.82	2.325	5-S2n	1.72	1.75	1.72	1.14	7.28	3.65
55.09 cfs	55.09 cfs	812.79	2.93	2.454	5-S2n	1.78	1.79	1.78	1.17	7.35	3.70
57.71 cfs	57.71 cfs	812.99	3.05	3.129	7-M2c	1.85	1.83	1.83	1.20	7.49	3.75
60.33 cfs	60.33 cfs	813.07	3.18	3.214	7-M2c	1.93	1.87	1.87	1.23	7.65	3.79

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 809.86 ft, Outlet Elevation (invert): 809.60 ft

Culvert Length: 50.00 ft, Culvert Slope: 0.0052

## **Tailwater Data for Crossing: Culvert C4**

**Table 13 - Downstream Channel Rating Curve (Crossing: Culvert C4)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
34.14	810.78	0.91	3.22	0.40	0.68
36.76	810.82	0.95	3.29	0.41	0.68
39.38	810.85	0.98	3.36	0.43	0.69

42.00	810.88	1.02	3.42	0.44	0.69
44.62	810.92	1.05	3.48	0.46	0.69
47.23	810.95	1.08	3.54	0.47	0.70
49.85	810.98	1.11	3.59	0.49	0.70
<b>52.47</b>	811.01	1.14	3.65	0.50	0.70
55.09	811.04	1.17	3.70	0.51	0.70
57.71	811.07	1.20	3.75	0.53	0.71
60.33	811.10	1.23	3.79	0.54	0.71

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0070 Channel Manning's n: 0.0300 Channel Invert Elevation: 809.87 ft

## **Roadway Data for Crossing: Culvert C4**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 29.00 ft Crest Elevation: 813.14 ft Roadway Surface: Gravel Roadway Top Width: 30.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 34.14 cfs Design Flow: 34.14 cfs Maximum Flow: 60.33 cfs

Table 14 - Summary of Culvert Flows at Crossing: Culvert C4

Headwater Elevation (ft)	Total Discharge (cfs)	C4 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
811.96	34.14	34.14	0.00	1
812.06	36.76	36.76	0.00	1
812.16	39.38	39.38	0.00	1
812.26	42.00	42.00	0.00	1
812.36	44.62	44.62	0.00	1
812.46	47.23	47.23	0.00	1
812.57	49.85	49.85	0.00	1
812.68	52.47	52.47	0.00	1
812.79	55.09	55.09	0.00	1
812.99	57.71	57.71	0.00	1
813.07	60.33	60.33	0.00	1
813.14	62.28	62.28	0.00	Overtopping

# **Culvert Data: C5**

### Site Data - C5

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 807.57 ft Outlet Station: 48.50 ft Outlet Elevation: 807.15 ft Number of Barrels: 2

## **Culvert Data Summary - C5**

Barrel Shape: Circular Barrel Diameter: 2.50 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 8 - Culvert Summary Table: C5** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
37.41 cfs	37.41 cfs	809.79	2.22	1.490	1-S2n	1.18	1.47	1.23	1.05	7.81	2.93
40.34 cfs	40.34 cfs	809.90	2.33	1.601	1-S2n	1.23	1.52	1.28	1.09	7.96	3.00
43.27 cfs	43.27 cfs	810.01	2.44	1.728	1-S2n	1.28	1.58	1.34	1.13	8.10	3.06
46.21 cfs	46.21 cfs	810.13	2.56	1.862	5-S2n	1.33	1.64	1.39	1.17	8.24	3.12
49.14 cfs	49.14 cfs	810.25	2.68	1.999	5-S2n	1.39	1.69	1.44	1.21	8.37	3.17
52.07 cfs	52.07 cfs	810.37	2.80	2.140	5-S2n	1.44	1.74	1.50	1.24	8.50	3.23
55.00 cfs	55.00 cfs	810.50	2.93	2.284	5-S2n	1.49	1.79	1.55	1.28	8.62	3.28
57.93 cfs	57.93 cfs	810.63	3.06	2.431	5-S2n	1.54	1.83	1.60	1.31	8.74	3.32
60.87 cfs	60.87 cfs	810.77	3.20	2.892	5-S2n	1.59	1.88	1.65	1.35	8.86	3.37
63.80 cfs	63.26 cfs	810.89	3.32	2.999	5-S2n	1.64	1.92	1.69	1.38	8.95	3.42
66.73 cfs	64.47 cfs	810.95	3.38	3.055	5-S2n	1.66	1.93	1.71	1.41	8.99	3.46

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 807.57 ft, Outlet Elevation (invert): 807.15 ft

Culvert Length: 48.50 ft, Culvert Slope: 0.0087

## **Tailwater Data for Crossing: Culvert C5**

Table 15 - Downstream Channel Rating Curve (Crossing: Culvert C5)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
37.41	808.64	1.05	2.93	0.33	0.59
40.34	808.68	1.09	3.00	0.34	0.59
43.27	808.72	1.13	3.06	0.35	0.59

46.21	808.76	1.17	3.12	0.36	0.59
49.14	808.80	1.21	3.17	0.38	0.60
52.07	808.83	1.24	3.23	0.39	0.60
55.00	808.87	1.28	3.28	0.40	0.60
57.93	808.90	1.31	3.32	0.41	0.60
60.87	808.94	1.35	3.37	0.42	0.61
63.80	808.97	1.38	3.42	0.43	0.61
66.73	809.00	1.41	3.46	0.44	0.61

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0050 Channel Manning's n: 0.0300 Channel Invert Elevation: 807.59 ft

**Roadway Data for Crossing: Culvert C5** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 28.00 ft Crest Elevation: 810.85 ft Roadway Surface: Gravel Roadway Top Width: 20.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 37.41 cfs Design Flow: 37.41 cfs Maximum Flow: 66.73 cfs

Table 16 - Summary of Culvert Flows at Crossing: Culvert C5

Headwater Elevation (ft)	Total Discharge (cfs)	C5 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
809.79	37.41	37.41	0.00	1
809.90	40.34	40.34	0.00	1
810.01	43.27	43.27	0.00	1
810.13	46.21	46.21	0.00	1
810.25	49.14	49.14	0.00	1
810.37	52.07	52.07	0.00	1
810.50	55.00	55.00	0.00	1
810.63	57.93	57.93	0.00	1
810.77	60.87	60.87	0.00	1
810.89	63.80	63.26	0.50	11
810.95	66.73	64.47	2.22	7
810.85	62.53	62.53	0.00	Overtopping

## **Culvert Data: C6**

### Site Data - C6

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 805.40 ft
Outlet Station: 104.56 ft
Outlet Elevation: 804.76 ft
Number of Barrels: 2

## **Culvert Data Summary - C6**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: None

Table 1 - Culvert Summary Table: C6

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
30.67 cfs	30.67 cfs	807.82	2.42	1.808	5-S2n	1.35	1.41	1.35	0.81	6.79	3.25
32.96 cfs	32.96 cfs	808.01	2.61	2.020	5-S2n	1.43	1.46	1.43	0.85	6.87	3.33
35.25 cfs	35.25 cfs	808.21	2.81	2.485	5-S2n	1.51	1.51	1.51	0.88	6.94	3.40
37.54 cfs	37.54 cfs	808.43	3.03	2.882	7-M2c	1.60	1.56	1.56	0.91	7.14	3.47
39.83 cfs	39.00 cfs	808.58	3.18	2.950	7-M2c	1.67	1.59	1.59	0.95	7.29	3.54
42.12 cfs	39.76 cfs	808.65	3.25	2.987	7-M2c	1.71	1.60	1.60	0.98	7.37	3.61
44.41 cfs	40.38 cfs	808.72	3.32	3.019	7-M2c	1.75	1.61	1.61	1.01	7.44	3.67
46.70 cfs	40.94 cfs	808.78	3.38	3.049	7-M2c	2.00	1.62	1.62	1.04	7.50	3.73
48.99 cfs	41.44 cfs	808.83	3.43	3.079	7-M2c	2.00	1.63	1.63	1.07	7.55	3.79
51.28 cfs	41.89 cfs	808.88	3.48	3.107	7-M2c	2.00	1.64	1.64	1.09	7.60	3.85
53.57 cfs	42.33 cfs	808.93	3.53	3.135	7-M2c	2.00	1.65	1.65	1.12	7.65	3.90

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 805.40 ft, Outlet Elevation (invert): 804.76 ft

Culvert Length: 104.56 ft, Culvert Slope: 0.0061

## **Tailwater Data for Crossing: Culvert C6**

**Table 1 - Downstream Channel Rating Curve (Crossing: Culvert C6)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
30.67	805.37	0.81	3.25	0.36	0.68
32.96	805.41	0.85	3.33	0.37	0.68
35.25	805.44	0.88	3.40	0.38	0.68

37.54	805.47	0.91	3.47	0.40	0.69
39.83	805.51	0.95	3.54	0.41	0.69
42.12	805.54	0.98	3.61	0.43	0.69
44.41	805.57	1.01	3.67	0.44	0.70
46.70	805.60	1.04	3.73	0.45	0.70
48.99	805.63	1.07	3.79	0.47	0.70
51.28	805.65	1.09	3.85	0.48	0.70
53.57	805.68	1.12	3.90	0.49	0.71

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 10.00 ft Side Slope (H:V): 2.00 (_:1) Channel Slope: 0.0070 Channel Manning's n: 0.0300 Channel Invert Elevation: 804.56 ft

**Roadway Data for Crossing: Culvert C6** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 15.00 ft Crest Elevation: 808.50 ft Roadway Surface: Gravel Roadway Top Width: 100.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 30.67 cfs Design Flow: 30.67 cfs Maximum Flow: 53.57 cfs

Table 2 - Summary of Culvert Flows at Crossing: Culvert C6

	•			
Headwater	Total	C6 Discharge	Roadway	<b>Iterations</b>
Elevation (ft)	Discharge	(cfs)	Discharge	
	(cfs)		(cfs)	
807.82	30.67	30.67	0.00	1
808.01	32.96	32.96	0.00	1
808.21	35.25	35.25	0.00	1
808.43	37.54	37.54	0.00	1
808.58	39.83	39.00	0.81	10
808.65	42.12	39.76	2.34	7
808.72	44.41	40.38	4.01	6
808.78	46.70	40.94	5.75	6
808.83	48.99	41.44	7.55	6
808.808	51.28	41.89	9.37	5
808.93	53.57	42.33	11.23	5
808.50	38.24	38.24	0.00	Overtopping

# **Culvert Data: C7**

### Site Data - C7

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 796.64 ft Outlet Station: 60.20 ft Outlet Elevation: 796.34 ft Number of Barrels: 2

## **Culvert Data Summary - C7**

Barrel Shape: Circular Barrel Diameter: 3.50 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in Barrel Manning's n: 0.0240 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 5 - Culvert Summary Table: C7** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
67.17 cfs	67.17 cfs	799.52	2.60	2.880	2-M2c	2.53	1.79	1.79	1.08	6.76	4.34
72.84 cfs	72.84 cfs	799.66	2.73	3.024	2-M2c	2.71	1.87	1.87	1.13	6.95	4.44
78.52 cfs	78.52 cfs	799.81	2.86	3.167	2-M2c	2.93	1.95	1.95	1.18	7.14	4.54
84.19 cfs	84.19 cfs	799.95	3.00	3.310	2-M2c	3.50	2.02	2.02	1.22	7.32	4.64
89.87 cfs	89.87 cfs	800.09	3.13	3.452	2-M2c	3.50	2.09	2.09	1.26	7.50	4.72
95.55 cfs	95.55 cfs	800.23	3.26	3.595	7-M2c	3.50	2.16	2.16	1.31	7.67	4.81
101.22 cfs	101.22 cfs	800.38	3.40	3.739	7-M2c	3.50	2.22	2.22	1.35	7.85	4.89
106.90 cfs	106.90 cfs	800.53	3.54	3.886	7-M2c	3.50	2.29	2.29	1.38	8.02	4.97
112.57 cfs	112.57 cfs	800.68	3.68	4.037	7-M2c	3.50	2.35	2.35	1.42	8.20	5.04
118.25 cfs	118.25 cfs	800.83	3.83	4.194	7-M2c	3.50	2.41	2.41	1.46	8.37	5.11
123.92 cfs	123.92 cfs	801.00	3.98	4.360	7-M2c	3.50	2.47	2.47	1.50	8.55	5.18

### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 796.64 ft, Outlet Elevation (invert): 796.34 ft

Culvert Length: 60.20 ft, Culvert Slope: 0.0050

## **Tailwater Data for Crossing: Culvert C7**

**Table 9 - Downstream Channel Rating Curve (Crossing: Culvert C7)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
67.17	796.68	1.08	4.34	0.67	0.84

72.84	796.73	1.13	4.44	0.70	0.84
78.52	796.78	1.18	4.54	0.73	0.85
84.19	796.82	1.22	4.64	0.76	0.85
89.87	796.86	1.26	4.72	0.79	0.86
95.55	796.91	1.31	4.81	0.81	0.86
101.22	796.95	1.35	4.89	0.84	0.86
106.90	796.98	1.38	4.97	0.86	0.87
112.57	797.02	1.42	5.04	0.89	0.87
118.25	797.06	1.46	5.11	0.91	0.87
123.92	797.10	1.50	5.18	0.93	0.88

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 10.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0100 Channel Manning's n: 0.0300 Channel Invert Elevation: 795.60 ft

## **Roadway Data for Crossing: Culvert C7**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 802.50 ft Roadway Surface: Gravel Roadway Top Width: 60.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 67.17 cfs Design Flow: 67.17 cfs Maximum Flow: 123.92 cfs

Table 10 - Summary of Culvert Flows at Crossing: Culvert C7

Headwater Elevation (ft)	Total Discharge (cfs)	C7 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
799.52	67.17	67.17	0.00	1
799.66	72.84	72.84	0.00	1
799.81	78.52	78.52	0.00	1
799.95	84.19	84.19	0.00	1
800.09	89.87	89.87	0.00	1
800.23	95.55	95.55	0.00	1
800.38	101.22	101.22	0.00	1
800.53	106.90	106.90	0.00	1
800.68	112.57	112.57	0.00	1
800.83	118.25	118.25	0.00	1
801.00	123.92	123.92	0.00	1
802.50	162.83	162.83	0.00	Overtopping

# **Culvert Data: C8**

### Site Data - C8

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 810.70 ft Outlet Station: 85.60 ft Outlet Elevation: 808.60 ft Number of Barrels: 1

## **Culvert Data Summary - C8**

Barrel Shape: Circular Barrel Diameter: 1.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 9 - Culvert Summary Table: C8** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.96 cfs	0.96 cfs	811.27	0.57	0.534	1-JS1f	0.27	0.41	1.00	0.45	1.22	2.40
1.02 cfs	1.02 cfs	811.30	0.60	0.557	1-S2n	0.28	0.43	0.28	0.46	5.74	2.44
1.09 cfs	1.09 cfs	811.32	0.62	0.581	1-JS1f	0.29	0.44	1.00	0.47	1.39	2.47
1.15 cfs	1.15 cfs	811.34	0.64	0.605	1-JS1f	0.30	0.45	1.00	0.48	1.47	2.51
1.22 cfs	1.22 cfs	811.37	0.67	0.629	1-JS1f	0.30	0.47	1.00	0.49	1.55	2.54
1.28 cfs	1.28 cfs	811.39	0.69	0.653	1-JS1f	0.31	0.48	1.00	0.50	1.63	2.58
1.34 cfs	1.34 cfs	811.41	0.71	0.679	1-JS1f	0.32	0.49	1.00	0.51	1.71	2.61
1.41 cfs	1.41 cfs	811.43	0.73	0.704	1-JS1f	0.33	0.50	1.00	0.52	1.79	2.64
1.47 cfs	1.47 cfs	811.45	0.75	0.730	1-JS1f	0.34	0.51	1.00	0.53	1.87	2.67
1.54 cfs	1.54 cfs	811.54	0.77	0.840	1-S1f	0.34	0.53	1.00	0.53	1.96	2.70
1.60 cfs	1.60 cfs	811.55	0.79	0.849	1-S1f	0.35	0.54	1.00	0.54	2.04	2.72

### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 810.70 ft, Outlet Elevation (invert): 808.60 ft

Culvert Length: 85.63 ft, Culvert Slope: 0.0245

## **Tailwater Data for Crossing: Culvert C8**

**Table 17 - Downstream Channel Rating Curve (Crossing: Culvert C8)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
0.96	811.15	0.45	2.40	0.56	0.89
1.02	811.16	0.46	2.44	0.57	0.90

1.09	811.17	0.47	2.47	0.59	0.90
1.15	811.18	0.48	2.51	0.60	0.90
1.22	811.19	0.49	2.54	0.61	0.91
1.28	811.20	0.50	2.58	0.62	0.91
1.34	811.21	0.51	2.61	0.63	0.91
1.41	811.22	0.52	2.64	0.64	0.91
1.47	811.23	0.53	2.67	0.66	0.92
1.54	811.23	0.53	2.70	0.67	0.92
1.60	811.24	0.54	2.72	0.68	0.92

Tailwater Channel Option: Triangular Channel

Side Slope (H:V): 2.00 (_:1) Channel Slope: 0.0200 Channel Manning's n: 0.0300 Channel Invert Elevation: 822.00 ft

## **Roadway Data for Crossing: Culvert C8**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 10.00 ft Crest Elevation: 822.00 ft Roadway Surface: Gravel Roadway Top Width: 25.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0.96 cfs Design Flow: 0.96 cfs Maximum Flow: 1.60 cfs

Table 18 - Summary of Culvert Flows at Crossing: Culvert C8b

Headwater Elevation (ft)	Total Discharge (cfs)	C8 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
811.27	0.96	0.96	0.00	1
811.30	1.02	1.02	0.00	1
811.32	1.09	1.09	0.00	1
811.34	1.15	1.15	0.00	1
811.37	1.22	1.22	0.00	1
811.39	1.28	1.28	0.00	1
811.41	1.34	1.34	0.00	1
811.43	1.41	1.41	0.00	1
811.45	1.47	1.47	0.00	1
811.54	1.54	1.54	0.00	1
811.55	1.60	1.60	0.00	1
822.00	10.37	10.37	0.00	Overtopping

## **Culvert Data: C9**

### Site Data - C9

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft Inlet Elevation: 807.54 ft Outlet Station: 99.86 ft Outlet Elevation: 806.81 ft Number of Barrels: 1

## **Culvert Data Summary - C9**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Corrugated Steel

Embedment: 0.00 in Barrel Manning's n: 0.0240 Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 6 - Culvert Summary Table: C9** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
11.16 cfs	11.16 cfs	809.67	1.98	2.134	7-M2c	1.79	1.20	1.20	0.27	5.68	3.11
12.14 cfs	12.14 cfs	809.83	2.11	2.287	7-M2c	2.00	1.25	1.25	0.29	5.87	3.21
13.12 cfs	13.12 cfs	810.01	2.24	2.473	7-M2c	2.00	1.30	1.30	0.30	6.05	3.30
14.09 cfs	14.09 cfs	810.31	2.39	2.772	7-M2c	2.00	1.35	1.35	0.31	6.24	3.39
15.07 cfs	15.07 cfs	810.63	2.54	3.090	7-M2c	2.00	1.40	1.40	0.33	6.42	3.47
16.05 cfs	16.05 cfs	810.97	2.70	3.425	7-M2c	2.00	1.44	1.44	0.34	6.61	3.55
17.03 cfs	17.03 cfs	811.32	2.87	3.777	7-M2c	2.00	1.49	1.49	0.35	6.80	3.62
18.01 cfs	18.01 cfs	811.69	3.05	4.147	7-M2c	2.00	1.53	1.53	0.36	6.99	3.69
18.98 cfs	18.98 cfs	812.04	3.24	4.503	7-M2c	2.00	1.57	1.57	0.37	7.19	3.76
19.96 cfs	19.96 cfs	812.44	3.44	4.896	7-M2c	2.00	1.60	1.60	0.38	7.39	3.83
20.94 cfs	20.94 cfs	812.85	3.65	5.310	7-M2c	2.00	1.64	1.64	0.40	7.60	3.90

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 807.54 ft, Outlet Elevation (invert): 806.81 ft

Culvert Length: 99.86 ft, Culvert Slope: 0.0073

**Tailwater Data for Crossing: Culvert C9** 

Table 11 - Downstream Channel Rating Curve (Crossing: Culvert C9)

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
11.16	807.71	0.27	3.11	0.96	1.09
12.14	807.73	0.29	3.21	1.01	1.10
13.12	807.74	0.30	3.30	1.05	1.11
14.09	807.75	0.31	3.39	1.10	1.11

15.07	807.77	0.33	3.47	1.14	1.12
16.05	807.78	0.34	3.55	1.18	1.13
17.03	807.79	0.35	3.62	1.23	1.13
18.01	807.80	0.36	3.69	1.27	1.14
18.98	807.81	0.37	3.76	1.31	1.14
19.96	807.82	0.38	3.83	1.34	1.15
20.94	807.84	0.40	3.90	1.38	1.15

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 12.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0560 Channel Manning's n: 0.0450 Channel Invert Elevation: 807.44 ft

**Roadway Data for Crossing: Culvert C9** 

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft Crest Elevation: 812.87 ft Roadway Surface: Paved Roadway Top Width: 24.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 11.16 cfs Design Flow: 11.16 cfs Maximum Flow: 20.94 cfs

Table 12 - Summary of Culvert Flows at Crossing: Culvert C9

Headwater Elevation (ft)	Total Discharge (cfs)	C9 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
809.67	11.16	11.16	0.00	1
809.83	12.14	12.14	0.00	1
810.01	13.12	13.12	0.00	1
810.31	14.09	14.09	0.00	1
810.63	15.07	15.07	0.00	1
810.97	16.05	16.05	0.00	1
811.32	17.03	17.03	0.00	1
811.69	18.01	18.01	0.00	1
812.04	18.98	18.98	0.00	1
812.44	19.96	19.96	0.00	1
812.85	20.94	20.94	0.00	1
812.87	21.00	21.00	0.00	Overtopping

# **Culvert Data: C10**

### Site Data - C10

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft
Inlet Elevation: 814.96 ft
Outlet Station: 50.00 ft
Outlet Elevation: 813.36 ft
Number of Barrels: 2

## **Culvert Data Summary - C10**

Barrel Shape: Circular Barrel Diameter: 2.00 ft

Barrel Material: Smooth HDPE

Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: None

**Table 10 - Culvert Summary Table: C10** 

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
2.76 cfs	2.76 cfs	815.50	0.54	0.147	1-JS1t	0.24	0.41	1.74	0.14	0.48	2.29
2.98 cfs	2.98 cfs	815.52	0.56	0.154	1-JS1t	0.25	0.42	1.75	0.15	0.51	2.36
3.20 cfs	3.20 cfs	815.54	0.58	0.162	1-JS1t	0.26	0.44	1.75	0.15	0.55	2.42
3.42 cfs	3.42 cfs	815.56	0.60	0.169	1-JS1t	0.27	0.45	1.76	0.16	0.58	2.48
3.64 cfs	3.64 cfs	815.58	0.62	0.176	1-JS1t	0.28	0.47	1.77	0.17	0.62	2.54
3.85 cfs	3.85 cfs	815.60	0.64	0.183	1-JS1t	0.28	0.48	1.77	0.17	0.66	2.59
4.07 cfs	4.07 cfs	815.62	0.66	0.190	1-JS1t	0.29	0.50	1.78	0.18	0.69	2.64
4.29 cfs	4.29 cfs	815.64	0.68	0.197	1-JS1t	0.30	0.51	1.78	0.18	0.73	2.69
4.51 cfs	4.51 cfs	815.66	0.70	0.204	1-JS1t	0.31	0.52	1.79	0.19	0.76	2.74
4.73 cfs	4.73 cfs	815.68	0.72	0.211	1-JS1t	0.31	0.53	1.79	0.19	0.80	2.79
4.95 cfs	4.95 cfs	815.69	0.73	0.218	1-JS1t	0.32	0.55	1.80	0.20	0.83	2.84

## **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert Inlet Elevation (invert): 814.96 ft, Outlet Elevation (invert): 813.36 ft

Culvert Length: 50.03 ft, Culvert Slope: 0.0320

## **Tailwater Data for Crossing: Culvert C10**

**Table 19 - Downstream Channel Rating Curve (Crossing: Culvert C10)** 

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
2.76	815.10	0.14	2.29	0.28	1.11
2.98	815.11	0.15	2.36	0.29	1.12
3.20	815.11	0.15	2.42	0.31	1.13

3.42	815.12	0.16	2.48	0.32	1.13
3.64	815.13	0.17	2.54	0.33	1.14
3.85	815.13	0.17	2.59	0.34	1.15
4.07	815.14	0.18	2.64	0.35	1.15
4.29	815.14	0.18	2.69	0.36	1.16
4.51	815.15	0.19	2.74	0.38	1.16
4.73	815.15	0.19	2.79	0.39	1.17
4.95	815.16	0.20	2.84	0.40	1.17

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft Side Slope (H:V): 4.00 (_:1) Channel Slope: 0.0320 Channel Manning's n: 0.0300

Channel Invert Elevation: 814.96 ft

## **Roadway Data for Crossing: Culvert C10**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 10.00 ft Crest Elevation: 818.00 ft Roadway Surface: Gravel Roadway Top Width: 25.00 ft Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 2.76 cfs Design Flow: 2.76 cfs Maximum Flow: 4.95 cfs

Table 20 - Summary of Culvert Flows at Crossing: Culvert C10

Headwater Elevation (ft)	Total Discharge (cfs)	C10 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
815.50	2.76	2.76	0.00	1
815.52	2.98	2.98	0.00	1
815.54	3.20	3.20	0.00	1
815.56	3.42	3.42	0.00	1
815.58	3.64	3.64	0.00	1
815.60	3.85	3.85	0.00	1
815.62	4.07	4.07	0.00	1
815.64	4.29	4.29	0.00	1
815.66	4.51	4.51	0.00	1
815.68	4.73	4.73	0.00	1
815.69	4.95	4.95	0.00	1
818.00	41.60	41.60	0.00	Overtopping



SCS ENGI	NEERS	Sheet No:	1 of 3
2 2 2 2 3 3 3 3		Calc. No.	
		Rev. No.	1
Job No. 25222260.00	Project: Columbia Energy Center MOD 12-13	By: RJG	Date: 8/11/23
Client: WPL	Subject: Diversion Berm Sizing	Chk'd: SJL	Date: 8/24/23

#### Purpose:

To size the post closure diversion berms on the final cover to accommodate the 25-year, 24-hour storm event.

### References:

- 1. WisDOT Facilities Development Manual Chapter 13, Section 30-15 Grass Lined Channels.
- 2. Design of Roadside Channels with Flexible Linings, HEC-15, USDOT FHWA.
- 3. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 4. Wisconsin Department of Natural Resources Conservation Practice Standard 1053 Channel Erosion Mat.

### Approach:

Use the Post Closure HydroCAD Model results to obtain the peak flow during a 25-year, 24-hour storm event along the diversion berms.

Use Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2 (from Reference #1) to size the swale for each design swale cross section. The WisDOT spreadsheet incorporates the design guidelines and equations described in "Design of Roadside Channels with Flexible Linings", HEC-15, USDOT FHWA (Reference #2).

Confirm the swale is stable and has enough capacity for the design flow rate.

### **Assumptions:**

- 1. Assume the channel geometry is a v-notch swale with one sideslope at 4:1 and one sideslope at 2:1 and a depth of 2.0 ft.
- 2. Assume 2.0% slope along the flowpath of the diversion swale.
- 3. Assume the following parameters per Section 15.2 Grass Lining Properties from Reference #1:

Vegetation Retardance Class = C for Swales

Vegetation Condition = Good

Vegetation Growth Form = Turf

4. Assume cohesive soil type with ASTM Soil Class SC and a Plasticity Index (PI) of 16.

## Calculations:

From the HydroCAD Report, the peak flow rate along the diversion berms are as follows:

<u>Areas</u>			<u>Areas</u>			<u>Areas</u>		<u>Areas</u>		
1	3.75	cfs	10	1.68	cfs	18	5.06 cfs	26	2.10	cfs
2	1.71	cfs	11	5.22	cfs	19	2.16 cfs	27	7.46	cfs
3	1.06	cfs	12	4.83	cfs	20	1.19 cfs	30	1.34	cfs
4	0.92	cfs	13	1.94	cfs	21	1.62 cfs	33	3.24	cfs
5	1.66	cfs	14	1.93	cfs	22	7.54 cfs	34	1.35	cfs
6	2.81	cfs	15	2.80	cfs	23	1.38 cfs			
8	2.66	cfs	16	1.78	cfs	24	3.61 cfs			
9	0.86	cfs	1 <i>7</i>	3.04	cfs	25	2.12 cfs			

Use highest flow to confirm diversion berm functions.

Use the Grass Swale Design Spreadsheet (Page 2) to determine the flow depth, velocity and shear stress in the swales.

### Results:

The diversion berms are adequately designed to accommodate the flows from the 25-year, 24-hour storm event. The diversion berms are stable at the design flow rates. The design flow depth of 2.0 feet maintains at least 0.5 ft of freeboard during the 25-year, 24-hour storm event. Based on shear stress, use erosion mat Class I, Type B along the flow path of the diversion berms.

## SCS ENGINEERS SCS ENGINEERS

| Sheet No: 2 of 3 | Calc. No. | Rev. No. | By: RJG | Date: 8/11/23 | Chi'd. S.II | Date: 8/24/23

		10.	
Job No. 25222260.00	Project: Columbia Energy Center MOD 12-13	By: RJG	Date: 8/11/23
Client: WPL	Subject: Diversion Berm Sizing	Chk'd: SJL	Date: 8/24/23
<del>`</del>			

Channel/Ditch Coometry	A == = 22
Channel/Ditch Geometry	Area 22
Channel Slope, S _o (ft/ft)	0.02
Channel Bottom Width, B (ft)	0
Channel Side Slope, z ₁	4
Channel Side Slope, z ₂	2
Flow Depth, d (ft) Solve iteratively	1.07
Safety Factor, SF	1.0
Vegetation/Soil Parameters	
Vegetation Retardance Class	С
Vegetation Condition	good
Vegetation Growth Form	turf
Soil Type	cohesive
D ₇₅ (in) (Set at 0.00 for cohesive soils)	
ASTM Soil Class	SC
Plasticity Index, PI	16
Results Summary	
Design Q (ft ³ /s)	7.5
Calculated Q (ft³/s)	7.5
Difference Between Design & Calc. Flow (%)	-0.2%
Stable (Yes or No)	YES
Channel Parameters	
Vegetation Height, h (ft)	0.67
Grass Roughness Coefficient, C _n	0.238
Cover Factor, C _f	0.90
Noncohesive Soil	0.00
Soil Grain Roughness, n _s	0.016
Permissible Soil Shear Stress, τ _p (lb/ft²)	N/A
Cohesive Soil	,
Porosity, e	0.35
Soil Coefficient 1, c ₁	1.0700
Soil Coefficient 2, c ₂	14.30
Soil Coefficient 3, c ₃	47.700
Soil Coefficient 4, c ₄	1.42
Soil Coefficient 5, c ₅	-0.61
Soil Coefficient 6, c ₆	0.00010
Permissible Soil Shear Stress, τ _p (lb/ft²)	0.080
Total Permissible Shear Stress, τ _p (lb/ft²)	0.080
Cross Sectional Area, A (ft ² )	3.435
Wetted Perimeter, P (ft)	6.80
Hydraulic Radius, R (ft)	0.505
Top Width, T (ft)	6.42
Hydraulic Depth, D (ft)	0.535
Froude Number (Q design)	0.528
Channel Shear Stress, τ _o (lb/ft²)	0.63
Actual Sheer Stress, τ _d (lb/ft²)	1.34
Mannings n	0.061
Average Velocity, V (ft/s)	2.20
Calculated Flow, Q (ft³/s)	7.5
Difference Between Design & Calc. Flow (%)	-0.2%
Effective Shear on Soil Surface, τ _e (lb/ft²)	0.009
Total Permissible Shear on Veg., τ _{p,veg} (lb/ft²)	11.65
Stable (Y or N)	YES

Source: Grass Lined Channel Design WisDOT Spreadsheet, FDM 13-30 Attachment 15.2

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Client: WPL	Subject:   Subject: Swale Sizing	Chk'd: SJL	Date: 8/24/23

## **Channel Erosion Mat**

(1053)

Wisconsin Department of Natural Resources Conservation Practice Standard

To differentiate applications WisDOT organizes erosion mats into three classes of mats, which are further broken down into various Types.

- A. Class I: A short-term duration (minimum of 6 months), light duty, organic ECRM with plastic or biodegradable netting.
  - 1. Type A Only suitable for slope applications, not channel applications.
  - 2. Type B Double netted product for use in channels where the calculated (design) shear stress is 1.5 lbs/ft2 or
- B. Class II: A long-term duration (three years or greater), organic ECRM.
  - 1. Type A Jute fiber only for use in channels to reinforce sod.
  - 2. Type B For use in channels where the calculated (design) shear stress is 2.0 lbs/ft2 or less. Made with plastic or biodegradable mat.
  - 3. Type C A woven mat of 100% organic material for use in channels where the calculated (design) shear stress is 2.0 lbs/ft2 or less. Applicable

for use in environmentally sensitive areas where plastic netting is inappropriate.

- C. Class III: A permanent 100% synthetic ECRM or TRM. Class I, Type B erosion mat or Class II, Type B or C erosion mat must be placed over a soil filled TRM.
  - Type A An ECRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft2 or less.
  - 2. Type B A TRM for use in channels where the calculated (design) shear stress of 2.0 lbs/ft2 or less.
  - 3. Type C A TRM for use in channels where the calculated (design) shear stress of 3.5 lbs/ft2 or less.
  - 4. Type D A TRM for use in channels where the calculated (design) shear stress of 5.0 lbs/ft² or less.

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Client: WPL	Subject: Diversion Berm Spacing Calculation	Chk'd: SJL	Date: 8/18/23

### Purpose:

Determine the spacing between diversion berms on the landfill final cover, with the goal of maintaining  $\leq 3$  ton/acre of soil loss along the final cover.

#### References

- 1. "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.
  - (Figure 1 on Sheet 2 and Tables 10 and 13 on Sheet 4).
- 2. Erosion and Sediment Control Handbook," Goldman, Jackson, & Bursztynsky, 1986.

(Table 5.5 on Sheet 5).

- 3. Rainfed retention probabilities computed for different cropping tillage systems. Agricultural Water Management, A.W. Mills & G.W. Thomas, 1985. Table 5.10 on Sheet 3)
- 4. Colombia Energy Center POO Update Drawings

### Approach:

Use the Universal Soil Loss Equation (USLE) to determine diversion berm spacing. Longest flow length is 555 feet.

USLE Equation: 
$$A = R * K * LS * C * P$$

where: A = Average annual soil loss, tons/acre

R = Rainfall and runoff erosivity index

K = Soil erodibility factor, tons/acre

LS = Slope length and steepness factor

C = Cover management factor

P = Practice factor

or 
$$LS = A$$
 $R \times K \times C \times P$ 

### **Assumptions:**

$$A = 3$$
 tons/acre

R =145 see Figure 1 on Sheet 2 (Reference #1)

0.38 see Table 5.10 on Sheet 3 for Loamy Very Fine Sand (Reference #3)

C = 0.0064 see Table 10 on Sheet 4, assuming 90% cover (Reference #1)

assume no support practice used

### Calculation:

$$LS = A = 3 = 8.51$$

$$R \times K \times C \times P = 145 \times 0.38 \times 0.0064 \times 1.0$$

From the LS Values Table (Sheet 5), based on the 4:1 final cover slope, the slope distance is between 200 and 250 feet.

Use linear interpolation between the LS values for 200 and 250 feet to determine the slope length value for the 4:1 slope.

Slope Length @ 200 ft LS= 8.33 Slope Length @ 250 ft LS= 9.31

Slope length for the calculate LS factor = 209

### Results:

The maximum distance between diversion berms along the final cover to maintain less than 3 tons/acre soil loss is

209 ft.

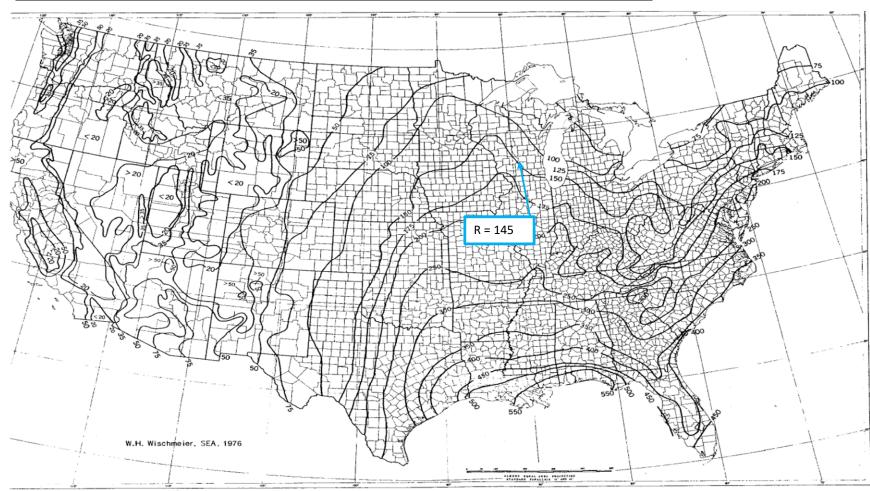
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IGURE 1.—Average annual values of the rainfall erosion index.

Source: "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.

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Table 5.10. Soil Erodibility Factor K_{fact} (after Stewart et al. 1975)^(a)

	P _{om} (%)		
Textural Class	<0.5	2	4
Sand	0.05	0.03	0.02
Fine sand	0.16	0.14	0.10
Very finesand	0.42	0.36	0.28
Loamy sand	0.12	0.10	0.08
Loamy finesand	0.24	0.20	0.16
Loamy veryfine sand	0.44	0.38	0.30
Sandy loam	0.27	0.24	0.19
Fine sandyloam	0.35	0.30	0.24
Very fine sandy loam	0.47	0.41	0.33
Loam	0.38	0.34	0.29
Silt loam	0.48	0.42	0.33
Silt	0.60	0.52	0.42
Sandy clayloam	0.27	0.25	0.21
Clay loam	0.28	0.25	0.21
Silty clayloam	0.37	0.32	0.26
Sandy clay	0.14	0.13	0.12
Silty clay	0.25	0.23	0.19
Clay		0.13- 0.2	

⁽a) The values shown are estimated averages of broad ranges of specific soil values. When a texture is near the border line of two texture classes, use the average of the two K_{fact} values. In addition, the values shown are commensurate with the English units used in the cited reference (and as used in the source-term module input files). To obtain analagous values in the metric units used in this report, the above values should be multiplied by 1.292.

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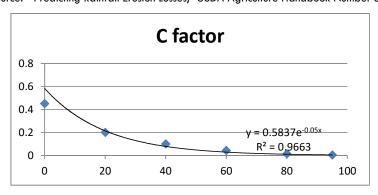
Subject: Diversion Berm Spacing Calculation

TABLE 10.—Factor C for permanent pasture, range, and idle land¹

		747.0						
Vegetative cano	ру	Cc	ver th	at co	ntacts	the so	il surfa	ce
	Percent		Percent ground cover					
height ²	cover3	Type	0	20	40	60	80	95+
No appreciable		G	0.45	0.20	0.10	0.042	0.013	0.003
canopy		w	.45	.24	.15	.091	.043	.011
Tall weeds or	25	G	.36	.17	.09	.038	.013	.003
short brush with average		W	.36	.20	.13	.083	.041	.011
drop fall height	50	G	.26	.13	.07	.035	.012	.003
of 20 in		W	.26	.16	.11	.076	.039	.011
	75	G	.17	.10	.06	.032	.011	.003
		W	.17	.12	.09	860.	.038	.011
Appreciable brush	25	G	.40	.18	.09	.040	.013	.003
or bushes, with average drop fa	II	w	.40	.22	.14	.087	.042	.011
height of 61/2 ft	50	G	.34	.16	.08	.038	.012	.003
		w	.34	.19	.13	.082	.041	.011
	75	G	.28	.14	.08	.036	.012	.003
		W	.28	.17	.12	.078	.040	.011
Trees, but no	25	G	.42	.19	.10	.041	.013	.003
appreciable low brush. Average		W	.42	.23	.14	.089	.042	.011
drop fall height	50	G	.39	.18	.09	.040	.013	.003
of 13 ft		w	.39	.21	.14	.087	.042	.011
	75	G	.36	.17	.09	.039	.012	.003
		w	.36	.20	.13	.084	.041	.011

Interpolated value C = 0.0064

Source: "Predicting Rainfall Erosion Losses," USDA Agriculture Handbook Number 537, 1978.



90 % cover = 0.0064

¹ The listed C values assume that the vegetation and mulch are randomly distributed over the entire area.

² Canopy height is measured as the average fall height of water drops falling from the canopy to the ground. Canopy effect is inversely proportional to drop fall height and is negligible if fall height exceeds 33 ft.

³Portion of total-area surface that would be hidden from view by canopy in a vertical projection (a bird's-eye view).

⁴G: cover at surface is grass, grasslike plants, decaying compacted duff, or litter at least 2 in deep.

W: cover at surface is mostly broadleaf herbaceous plants (as weeds with little lateral-root network near the surface) or undecayed residues or both.

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TABLE 5.5 LS Values* (10)

SlopeLS values for following slope lengths l, ft (m)						LS values for following slope lengths $l$ , ft (m)																		
Slope	gradient	10	20	30	40	50	60	70	80	90	100	150	200	250	300	350	400	450	500	600	700	800	900	1000
ratio	8, %	(3.0)	(6.1)	(9.1)	(12.2)	(15.2)	(18.3)	(21.3)	(24.4)	(27.4)			(61)	(76)			(122)			(183)	(213)	(244)	(274)	(305)
	0.5	0.06		0.07	0.08	0.08	0.09	0.09	0.09	0.09	0.10	0.10	0.11	0.11	0.12	0.12	0.13	0.13	0.13	0.14	0.14	0.14	0.15	0.16
100:1	1	0.08	0.09	0.10	0.10	0.11	0.11	0.12	0.12	0.12	0.12	0.14	0.14	0.15	0.16	0.16	0.16	0.17	0.17	0.18	0.18	0.19	0.19	
	2	0.10			0.15	0.16	0.17	0.18	0.19	0.19	0.20	0.23	0.25	0.26	0.28	0.29	0.30	0.32	0.33	0.34	0.36	0.37	0.39	0.40
	3	0.14	0.18		0.22	0.23	0.25	0.26	0.27	0.28	0.29	0.32	0.35	0.38	0.40	0.42	0.43	0.45	0.46	0.49	0.51	0.54	0.55	0.57
	4	0.16	0.21	0.25	0.28	0.30	0.33	0.35	0.37	0.38	0.40	0.47	0.53	0.58	0.62	0.66	0.70	0.73	0.76	0.82	0.87	0.92	0.96	1.00
20:1	5	0.17		0.29	0.34	0.38	0.41	0.45	0.48	0.51	0.53	0.66	0.76	0.85	0.93	1.00	1.07	1.13	1.20	1.31	1.42	1.51	1.60	1.69
	6	0.21	0.30	0.37	0.43	0.48	0.52	0.56	0.60	0.64	0.67	0.82	0.95	1.06	1.16	1.26	1.34	1.43	1.50	1.65	1.78	1.90	2.02	2.13
	7	0.26	0.37	0.45	0.52	0.58	0.64	0.69	0.74	0.78	0.82	1.01	1.17	1.30	1.43	1.54	1.65	1.75	1.84	2.02	2.18	2.33	2.47	2.61
12%:1	8	0.31	0.44	0.54	0.63	0.70	0.77	0.83	0.89	0.94	0.99	1.21	1.40	1.57	1.72	1.85	1.98	2.10	2.22	2.43	2.62	2.80	2.97	3.13
	9	0.37	0.52	0.64	0.74	0.83	0.91	0.98	1.05	1.11	1.17	1.44	1.66	1.85	2.03		2.35		2.62	2.87	3.10	3.32	3.52	3.71
10:1	10	0.43	0.61	0.75	0.87	0.97	1.06	1.15	1.22	1.30	1.37	1.68	1.94	2.16	2.37	' 2.56	2.74	2.90	3.06	3.35	3.62	3.87	4.11	4.33
	11	0.50	0.71	0.86	1.00	1.12	1.22	1.32	1.41	1.50	1.58	1.93	2.23	2.50	2.74	2.95	3.16	3.35	3.53	3.87	4.18	4.47	4.74	4.99
8:1	12.5	0.61	0.86	1.05	1.22	1.36	1.49	1.61	1.72	1.82	1.92	2.35	2.72	3.04	3.33	3.59	3.84	4.08	4.30	4.71	5.08	5.43	5.76	6.08
	15	0.81	1.14	1.40	1.62	1.81	1.98	2.14	2.29	2.43	2.56	3.13	3.62	4.05	4.43	4.79	5.12	5.43	5.72	6.27	6.77	7.24	7.68	8.09
6:1	16.7	0.96	1.36	1.67	1.92	2.15	2.36	2.54	2.72	2.88	3.04	3.72	4.30	4.81	5.27	5.69	6.08	6.45	6.80	7.45	8.04	8.60	9.12	9.62
5:1	20	1.29	1.82	2.23	2.58	2.88	3.16	3.41	3.65	3.87	4.08	5.00	5.77	6.45	7.06	7.63	8.16	8.65	9.12	9.99	10.79	11.54	12.24	12.90
4%:1	22	1.51	2.13	2.61	3.02	3.37	3.69	3.99	4.27	4.53	4.77	5.84		7.54				10.12		11.68	12.62	13.49	14.31	15.08
~ 4:1	25	1.86	2.63	3.23	3.73	4.16	4.56	4.93	5.27	5.59	5.89	7.21			10.20					14.43	15.58	16.66	17.67	18.63
	30	2.51	3.56	4.36	5.03	5.62	6.16	6.65	7.11	7.54	7.95	9.74			13.77					19.48	21.04	22,49	23,86	25.15
3:1	33.3	2.98	4.22	5.17	5.96	6.67	7.30	7.89	8.43	8.95	9.43	11.55								23.10	24.95	26.67	28.29	29.82
	35	3.23	4.57	5.60	6.46	7.23	7.92	8.55	9.14	9.70	10.22	12.52	14.46	16.16	17.70	19.12	20.44	21.68	22.86	25.04	27.04	28.91	30.67	32.32
2%:1	40	4.00	5.66	6.93	8.00	8.95	9.80	10.59	11.32	12.00	12.65				21.91					30.99	33.48	35.79	37.96	40.01
_	45	4.81	6.80	8.33	9.61	10.75	11.77	12.72	13.60	14.42	15.20	18.62	21.50	24.03	26.33	28.44	30.40	32.24	33.99	37.23	40.22	42.99	45.60	48.07
2:1	50	5.64	7.97	9.76	11.27	12.60		14.91	15.94	16.91	17.82	21.83	25.21	28.18	30.87	33.34	35.65	37.81	39.85	43.66	47.16	50.41	53.47	56.36
	55	6.48	9.16	11.22	12.96	14.48	15.87	17.14	18.32	19.43	20.48	25.09								50.18	54.20	57.94	61.45	
1%:1	57	6.82	9.64	11.80	13.63	15.24	16.69	18.03	19.28	20.45	21.55	26.40	30.48	34.08	37.33	40.32	43.10	45.72	48.19	52.79	57.02	60.96	64.66	68.15
	60		10.35	12.68		16.37	17.93	19.37	20.71	21.96	23.15				40.10					56.71	61.25	65.48	69.45	73.21
1%:1	66.7			14.61		18.87		22.32	23.87	25.31	26.68				46.22					65.36	70.60	75.47	80.05	84.38
	70			15.55		20.08			25.39	26.93	28.39				49.17					69.54	75.12	80.30	85.17	89.78
	75	9.78	13.83	16.94	19.56	21.87	23.95	25.87	27.66	29.34	30.92	37.87	43.73	48.89	53.56	<b>57.85</b>	61.85	65.60	69.15	75.75	81.82	87.46	92.77	
1%:1			14.93		21.11	23.60	25.85	27.93	29.85	31.66	33.38	40.88	47.20	52.77	57.81	62.44	66.75	70.80	74.63	81.76	88.31	94.41	100.13	105.55
			15.98		22.61	25.27	27.69	29.90			35.74				61.91					87.55		101.09		
	90	12.02	17.00	20.82	24.04	26.88	29.44	31.80			38.01				65.84							107.51		
		12.71	17.97	22.01	25.41	28.41	31.12			38.12					69.59							113.64		
1:1	100	13.36	18.89	23.14	26.72	29.87	32.72	35.34		40.08												119.48		

*Calculated from

LS = 
$$\left(\frac{65.41 \times s^2}{s^2 + 10,000} + \frac{4.56 \times s}{\sqrt{s^2 + 10,000}} + 0.065\right) \left(\frac{t}{72.5}\right)$$

FROM "EROSION ÉSEDMENT COUTROL HANDBOOK", Goldman, Jackson, + Borsztynsky, 1986

LS = topographic factor

l = slope length, ft (m × 0.3048)

s = slope steepness,

m = exponent dependent upon slope steep
(0.2 for slopes < 1%, 0.3 for slopes 1:
0.4 for slopes 3.5 to 4.5%, and
0.5 for slopes > 5%)



## CC ENCINEEDS

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Client: WPL	Subject: Downslope Pipe and Inlet Sizing	Chk'd: RJG	Date: 8/28/23

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1 of 4

### Purpose:

To size the downslope pipe and inlet to accommodate the 25-year, 24-hour storm event.

### References:

1. HydroCAD Report: COL_Mod12-13_HydroCAD Report

Use the orifice equation to size the downslope pipe inlet. Size the inlet for the largest diversion berm flow rate and apply that inlet size to all downslope pipe inlets. Confirm the head (h) acting on the orifice will not overtop the diversion berm depth of 2.0 ft.

Use Manning's equation to size the downslope pipe based on the largest diversion berm flow rate. Confirm the pipe has capacity for the design flow under open channel flow conditions.

### Assumptions:

- 1. Orifice coefficient =
- 0.63 2. Assume the orifice head (h) acts on the centerline of the inlet pipe.
- 3. Manning's n =0.012 (For smooth walled HDPE pipe: http://www.engineeringtoolbox.com/mannings-roughness-d_799.html)
- 4. Size flumes under the vegetated cover condition.

From the HydroCAD Report (Reference 1), the peak discharge to each downslope flume resulting from a 25-year, 24-hour storm is as follows*:

Flume 1	F	lume 2		Flume 3 Existing	3)	Flume 4 (Exist	ling)	Flume 5 (Existi	ng)
Area 1	3.75	Area 3	1.06						
Area 2	1.71	Area 4	0.92						
Area 12	4.83	Area 14	1.93						
Area 13	1.94	Area 15	2.80			_		_	
Total =	12.23		6.71		0		0		0
Flume 6	F	lume 7		Flume 8		Flume 9		Flume 10	
Area 5	1.66	Area 8	2.66	Area 20	1.19	Area 10	1.68	Area 27	0.52
Area 6	2.81	Area 9	0.86	Area 25	2.12	Area 11	5.22	Area 28	0.44
Area 16	1.78	Area 18	5.06	Area 34	1.35	Area 21	1.62		
Area 17	3.04	Area 19	2.16			Area 22	7.54		
		Area 24	3.61			Area 26	2.10		
		Area 33	3.24			Area 27	7.46		
Total =	9.29 T	otal =	17.59	Total =	4.66	Total =	25.62	Total =	0.96

^{*} Please note that the total flow rate at each flume calculated above may not reflect the flow rate shown in the HydroCAD Model due to the inflow to the flume occuring at different times during the storm event. The calculation above reflects the peak flow rate.

Based on the inlet sizing calculation, an 18" diameter inlet will convey the stormwater runoff from the largest flow rate to a inlet (Area 22).

Based on the Manning's calculation for flow within the pipe, the 12" diameter downslope pipe will accommodate the design flow for Flumes 1 through 8 and Flume 10 under open channel flow conditions. Although the flow for the downslope pipes can be handled by 12" dia. pipes, for ease of construction, all downslope pipes will be 18" dia with the exception of Flume 10. Flume 10 will be constructed with a 12" dia pipe based on the drainage area and anticipated flow rate.

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### Calculations:

### Size the downslope pipe inlet:

From the HydroCAD report (Reference #1), the maximum 25-year, 24-hour flow along a diversion berm is in HydroCAD model).

7.54 cfs Area 27 Inlet

Orifice Equation:  $Q = C * A * (2 * g * h)^{0.5}$ 

Q = flow rate (cfs) = where: 7.5 (From above)

C = orifice coefficient = 0.63 (See assumption #1)

A = orifice area (sf) = 1.77 (area of 18" diameter pipe)

Actual Pipe Diameter =

18 inches

 $g = gravity (ft/sec^2)=$ h = orifice head acting on centerline (ft)

 $h = (Q/(C * A))^2/(2 * g) = 0.7 ft$ 

Given Assumption #2, depth of flow along diversion berm = h + D/2/12 =

1.46 ft

The diversion swale depth of 2 ft is sufficient to prevent overtopping at the downslope pipe inlet locations.

The depth of the diversion berm increases at the entrance of the down slope pipes due to mounding of the soil over the pipe.

### Size the downslope flume pipe:

Use Manning's equation to size the downslope pipe.

Manning's Equation:  $Q = (1.49/n) \times A \times R^{(2/3)} \times S^{(1/2)}$ 

where: Q = Flow Rate, cfs

n = Manning's Roughness Coefficient

A = Flow Area, sf

R = Hydraulic Radius, ft (= A/P)

S = Channel Slope, ft/ft

For flow rates  $\leq$  20 cfs, assume a 12" diameter downslope flume:

17.59 cfs to Flume 7 to check sizing (max flow to a flume that is < 20 cfs) Use

Design Criteria

Pipe Diameter (in) = D =

Pipe Slope (ft/ft) = S =0.25

Manning's Roughness Coefficient = n =0.012

See Downslope Flume 7 pipe flow calculator on Sheet 3

For flow rates >20 cfs, try 18" diameter downslope flume:

25.62 cfs to Flume 9 to check sizing (max flow to a flume that is < 20 cfs)

Design Criteria

Pipe Diameter (in) = D =18

Pipe Slope (ft/ft) = S =

Manning's Roughness Coefficient = n = 0.012

See Downslope Flume 9 pipe flow calculator on Sheet 3

 Sheet No:
 3 of 4

 Calc. No.
 Rev. No.

 By: SJL
 Date: 8/28/23

Job No. 25222260.00 Project: Columbia Energy Center MOD 12-13 By: SJL Date: 8/28/23 Client: WPL Subject: Downslope Pipe and Inlet Sizing Chk'd: RJG Date: 8/28/23

Calculations (Continued): Flume 7 17.59 0

## Manning Formula Uniform Pipe Flow at Given Slope and Depth

## Inputs:

Pipe Diameter, d _o	12.00	in
Manning Roughness,		
<u>n</u>	0.0120	
Pressure slope		
(possibly equal to		
pipe slope), So	0.2500	slope
Percent of (or ratio		
to) full depth (100%		
or 1 if flowing full)	0.7500	fraction

### **Results:**

Flow, Q	17.5969	ft^3/s
Velocity, v	27.8498	ft/s
Velocity head, hv	12.0541	ft
Flow Area, A	0.6319	ft^2
Wetted Perimeter, P	2.0944	ft
Hydraulic Radius	0.3017	ft
Top Width, T	0.8660	ft
Froude Number, F	5.84	
Shear Stress (tractive		
force), τ	11.7045	psf

Version 2.0 (20 June 2017)

**HawsEDC Calculators** 

 Sheet No:
 4 of 4

 Calc. No.
 Rev. No.

 By: SJL
 Date: 8/28/23

Job No. 25222260.00Project:Columbia Energy Center MOD 12-13By: SJLDate: 8/28/23Client: WPLSubject:Downslope Pipe and Inlet SizingChk'd: RJGDate: 8/28/23

Calculations (Continued): Flume 9 25.62

## Manning Formula Uniform Pipe Flow at Given Slope and Depth

## Inputs:

Pipe Diameter, d _o	18.00	in
Manning Roughness, n	0.0120	
Pressure slope (possibly equal to pipe slope), $S_0$	0.2500	slope
Percent of (or ratio to) full depth (100% or 1 if flowing full)	0.4705	fraction

### **Results:**

Flow, Q	25.6240	ft^3/s
Velocity, v	31.3544	ft/s
Velocity head, hv	15.2789	ft
Flow Area, A	0.8172	ft^2
Wetted Perimeter, P	2.2676	ft
Hydraulic Radius	0.3604	ft
Top Width, T	1.4974	ft
Froude Number, F	7.60	
Shear Stress (tractive		
force), τ	11.0139	psf

Version 2.0 (20 June 2017)

**HawsEDC Calculators** 

SCS ENGINE	ERS	Sheet No.	1	of	7
		Calc. No.			
		Rev. No.			
Job No. 25222260.00	Project: Columbia Energy Center MOD 12-13	By: SJL		Date:	8/28/23
Client: WPL	Subject: Energy Dissipator Sizing	Chk'd: RJG		Date:	8/30/23

### Purpose:

To size an energy dissipator structure and riprap apron at the outlet of the downslope flume pipes.

### References:

- 1. "Hydraulic Design of Energy Dissipators for Culverts and Channels," HEC-14, Third Edition, July 2006, USDOT FHWA.
- 2. Downslope Pipe and Inlet Sizing calculation (for pipe size, flow rate, and pipe velocity).
- 3. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 4. Facilities Development Manual Chapter 13, Section 13-30 Rock Riprap Lined Chutes.
- 5. WisDOT FDM Table 25.1

### Approach:

Use the downslope pipe outlet velocity to size an energy dissipator structure (USBR Type VI Impact Basin) following the design approach outlined in Section 9.4 of Reference #1.

Use Rock Chute Data Spreadsheet, FDM 13-30-30 Attachment 30.1 (from Reference #5) to design the rock chute. For construction purposes use the maximum flow to size all dissipators and riprap apron.

### **Assumptions:**

- 1. Riprap specific gravity = 2.65
- 2. From the HydroCAD Report, the 25-year, 24-hour peak discharge to each downslope flume is as follows*:

Flume 1 Flume 2			Flume 3 Existing)	Flume 4 (Ex	isting)	Flume 5 (Existing	ng)	
Area 1	3.75	Area 3	1.06					
Area 2	1.71	Area 4	0.92					
Area 12	4.83	Area 14	1.93					
Area 13	1.94	Area 15	2.80					
Total =	12.23		6.71	0		0		0
Flume 6	F	lume 7		Flume 8	Flume 9		Flume 10	

Flume 6		Flume 7		Flume 8		Flume 9		Flume 10
Area 5	1.66	Area 8	2.66	Area 20	1.19	Area 10	1.68	This flume discharges directly into
Area 6	2.81	Area 9	0.86	Area 25	2.12	Area 11	5.22	a concrete catch basin at the toe
Area 16	1.78	Area 18	5.06	Area 34	1.35	Area 21	1.62	of slope, therefore, no energy
Area 17	3.04	Area 19	2.16			Area 22	7.54	dissipator is needed.
		Area 24	3.61			Area 26	2.10	
		Area 33	3.24			Area 27	7.46	
Total =	9.29	Total =	17.59	Total =	4.66	Total =	25.62	_
* Plagra nata th	at the tot	tal flow rate at a	ach fluma	calculated above	may not	roflect the flow	ata chaw	n in the HydroCAD Model due

Please note that the total flow rate at each flume calculated above may not reflect the flow rate shown in the HydroCAD Model due to the inflow to the flume occuring at different times during the storm event. The calculation above reflects the peak flow rate.

Using Figure 9.14 (See Sheet 4), enter the Froude Number and the Energy from Step 2 to determine the from the downslope flume pipe and inlet sizing calculation.

### Results:

The energy dissipator structures for the 18" dia. downslope flume pipes will consist of dissipator structures with widths (WB) of 6 feet, with the remaining dimensions from Table 9.2 on Sheets 5 and 6.

Riprap at the Flume energy dissipator outlets will consist of WisDOT Select Crush Material (D50= 2.2 inches) (See Page 3). The riprap apron footprint will be based on the energy dissipator width, the rock chute, and/or the outlet swale geometry (See Plan Set).

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Subject: Energy Dissipator Sizing	Chk'd: RJG	Date	8/30/23

# Client: WPL Calculations:

Ho =

### For 18" dia. downslope flume pipes

From Reference #2:

Job No. 25222260.00

Flow rate (Q) = 25.6 cfs Pipe velocity (V) = 31.4 ft/s Flow area (A) = Q/V = 0.82 sf

Design procedure from pg. 9-40 of Reference #1:

Step 1: Compute the Equivalent Depth of Flow Entering Dissipator:

 $Y_e = (A/2)^{1/2}$  where:  $Y_e = Equivalent depth$  A = Area (from above)

 $Y_e = 0.64 \text{ ft}$ 

Step 2: Compute the Froude Number and the energy at the end of the pipe:

 $Fr = V/[(g^*Y_e)^{1/2}] \qquad \text{where:} \quad Fr = Froude \ Number} \\ V = Velocity \ (from \ above) \\ g = Gravity \ constant \ (32.2 \ ft/sec^2) \\ Y_e = Equivalent \ depth \ (from \ Step \ 1 \ above) \\ H_o = Y_e + V^2/2g \qquad \text{where:} \quad H_o = Energy \ at \ the \ end \ of \ the \ pipe \\ Y_e = Equivalent \ depth \ (from \ above) \\ V = Velocity \ (from \ above) \\ V = Velocity \ (from \ above) \\ \end{cases}$ 

Step 3: Determine H_a/W_B and calculate the required width of the energy dissipator:

Using Figure 9.14 (See Sheet 4), enter the Froude Number and the Energy from Step 2 to determine the width of the energy dissipator.

From Figure 9.14,  $H_o/W_B =$  2.70  $W_B = H_o/(H_o/W_B)$   $W_B = 5.9$  ft. Use  $W_B =$  6.0 ft.

15.9 ft

Step 4: Obtain the remaining energy dissipator dimensions from Table 9.2 from Reference #1 (see Sheets 5 and 6)

 $g = Gravity constant (32.2 ft/sec^2)$ 

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## Client: WPL Calculations:

Job No. 25222260.00

Step 5: Determine the exit velocity from the energy dissigptor structure and size the riprap apron at the structure outlet. Use the relationship:

$$H_B = Q/(W_B \times V_B) + V_B^2 / 2g = H_o \times (1-H_L/H_o)$$

Where:

Q = 25.6 cfs, flowrate

 $W_B = 6.0$  ft, width of energy dissipator

 $g = 32.2 \text{ ft/s}^2$ , gravity

H_O 15.9 Energy at end of pipe

 $H_L/H_O$  76 %, Energy loss (From Figure 9.15 from Reference #1, see Sheet 3)

 $V_B = Velocity$  at exit of dissipator (ft/s)

HB = Energy at exit of dissipator (ft)

Calculate HB using the second part of the equation:

$$H_B = H_o \times (1 - H_L/H_o)$$

 $H_R =$ 3.82 ft

Using trial and error, select values for V_B and use the first part of the equation to calculate H_B:

Try  $V_B =$ 

1.105 ft/s

 $H_B =$ 

3.87

Based on the energy dissipator structure exit velocity, calculate the riprap size at the dissipator outlet. From Equation 10.6 from Reference #1:

$$D_{50} = (0.692 / (S-1)) \times (V^2/2g)$$

Where:

S = 2.65Specific gravity (See Assumption #1)

V = 1.11 Velocity =  $V_B$  from above.

 $D_{50}$  = riprap size

 $D_{50 \text{ Calc'd}} =$ 

0.008

Round the calculated  $D_{50}$  up to the nearest IDOT standard riprap size:

 $D_{50 Design} =$ 

0.18

Use = Select Crushed Material

with geotextile

Type R

Riprap Type	D ₅₀ (inches)	D ₅₀ (feet)	Riprap Thickness (in)	Geotextile Type
Select Crushed Material	2.2	0.18	5	Type R
Light Riprap	10	0.83	12	Type R
Medium Riprap	12.5	1.04	18	Type HR
Heavy Riprap	16	1.33	24	Type HR
Extra-Heavy Riprap	20	1.67	30	Type HR

from Reference 5

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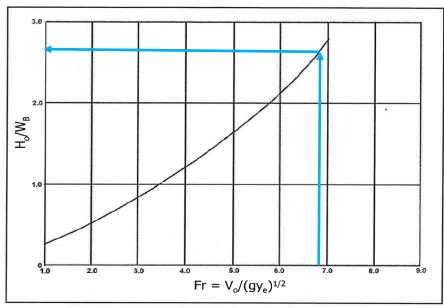


Figure 9.14. Design Curve for USBR Type VI Impact Basin

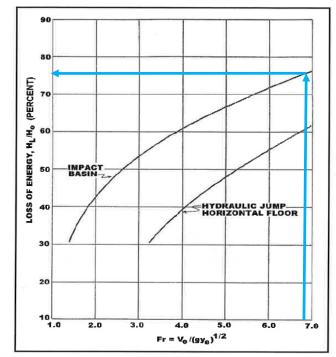


Figure 9.15. Energy Loss of USBR Type VI Impact Basin versus Hydraulic Jump

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Client: WPL Subject: Energy Dissipator Sizing Chk'd: RJG Date: 8/30/23

## Table 9.2 (CU). USBR Type VI Impact Basin Dimensions (ft) (AASHTO, 2005)

W ₅	fh)	h ₂	h ₃	h ₄	L	L	L ₂
4.	3.08	1.50	0.67	1.67	5.42	2.33	3.08
5.	3.83	1.92	0.83	2.08	6.67	2.92	3.83
6,	4.58	2.25	1.00	2.50	8.00	3,42	4.58
7.	5.42	2.58	1.17	2.92	9.42	4.00	5.42
8.	6.17	3.00	1.33	3.33	10.67	4.58	6.17
9.	6.92	3.42	1.50	3.75	12.00	5.17	5.92
10.	7.58	3.75	1.67	4.17	13:42	5.75	7.67
11.	8.42	4.17	1.83	4.58	14.58	6.33	8.42
12.	9.17	4.50	2.00	5.00	16.00	6.83	9.17
13.	10.17	4.92	2.17	5.42	17.33	7.42	10.00
14.	10.75	5.25	2.33	5.83	18.67	8.00	10.75
15.	11.50	5.58	2.50	6.25	20.00	8.50	11.50
16.	12.25	6.00	2.67	6.67	21.33	9.08	12.25
17.	13.00	6.33	2.83	7.08	21.50	9.67	13.00
18.	13.75	6.67	3.00	7.50	23.92	10.25	13.75
19.	14.58	7.08	3.17	7.92	25.33	10.83	14.58
	45.00	7.50	2.22	0.00	50.50	44.40	35.00
20.	15.33	7.50	3.33	8.33	26.58	11.42	15.33
Ws	W	W ₂	t,	t ₂	t ₃	t.	t _s
W ₈	W ₁	W ₂	t, 0.50	t ₂	t ₃	t _e 0.50	t _s
W _s 4. 5.	W ₁ 0,33 0,42	W ₂ 1.08 1.42	t ₁ 0.50 0.50	t ₂ 0.50 0.50	t ₃ 0.50 0.50	t. 0.50 0.50	t _s 0.25 0.25
W ₈ 4. 5.	W ₁ 0,33 0,42 0,50	W ₂ 1.08 1.42 1.67	t ₁ 0.50 0.50 0.50	t ₂ 0.50 0.50 0.50	t ₃ 0.50 0.50 0.50	t _e 0.50 0.50 0.50	t ₅ 0.25 0.25 0.25
W _s 4. 5. 6.	W ₁ 0,33 0.42 0.50 0.50	W ₂ 1.08 1.42 1.67 1.92	t ₁ 0.50 0.50 0.50 0.50	t ₂ 0.50 0.50 0.50 0.50	t ₃ 0.50 0.50 0.50	0.50 0.50 0.50 0.50	0.25 0.25 0.25 0.25
W ₈ 4. 5. 6. 7.	W ₁ 0.33 0.42 0.50 0.50 0.58	W ₂ 1.08 1.42 1.67 1.92 2.17	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50	t ₂ 0.50 0.50 0.50 0.50 0.50	t ₃ 0.50 0.50 0.50 0.50 0.50 0.50	0.50 0.50 0.50 0.50 0.50 0.50	0.25 0.25 0.25 0.25 0.25
W ₈ 4. 5. 6. 7. 8. 9.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.5	t ₂ 0.50 0.50 0.50 0.50 0.50 0.58 0.58	t ₃ 0.50 0.50 0.50 0.50 0.50 0.50 0.67	0.50 0.50 0.50 0.50 0.50 0.50 0.50	t _s 0.25 0.25 0.25 0.25 0.25 0.25 0.25
W _s 4. 5. 6. 7. 8. 9.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67	t ₂ 0.50 0.50 0.50 0.50 0.50 0.58 0.58 0.67	t ₃ 0.50 0.50 0.50 0.50 0.50 0.50 0.75	0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.58 0.67	t _s 0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25
W _s 4. 5. 6. 7. 8. 9. 10.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67	t ₂ 0.50 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75	t ₃ 0.50 0.50 0.50 0.50 0.50 0.50 0.75 0.75	0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25
W ₆ 4. 5. 6. 7. 8. 9. 10. 11.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.83	0.50 0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33
W ₆ 4. 5. 6. 7. 8. 9. 10. 11. 12.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.83 0.83	0.50 0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33
W ₆ 4. 5. 6. 7. 8. 9. 10. 11. 12. 13.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00 1.08	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92 1.00	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.83 0.83 0.92	0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83 0.92	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33 0.33
W ₈ 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00 1.08 1.17	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.67	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92 1.00 1.00	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.75	0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83 0.92 1.00	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33 0.42 0.42
W ₈ 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14. 15.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00 1.08 1.17 1.25	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00 3.00 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.67 0.75	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92 1.00 1.00	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.75	0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83 0.92 1.00	0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33 0.42 0.42
W ₈ 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14. 15. 16.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00 1.08 1.17 1.25 1.33	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00 3.00 3.00 3.00 3.00 3.0	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.67 0.75	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92 1.00 1.00 1.00	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.83 0.83 0.92 1.00 1.00	0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83 0.92 1.00 1.00	0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33 0.33 0.42 0.42 0.50
W ₈ 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14. 15.	W ₁ 0.33 0.42 0.50 0.50 0.58 0.67 0.75 0.83 0.92 1.00 1.08 1.17 1.25	W ₂ 1.08 1.42 1.67 1.92 2.17 2.50 2.75 3.00 3.00 3.00 3.00 3.00 3.00	t ₁ 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.67 0.75	t ₂ 0.50 0.50 0.50 0.50 0.58 0.58 0.67 0.75 0.83 0.92 1.00 1.00	t ₃ 0.50 0.50 0.50 0.50 0.50 0.75 0.75 0.75	0.50 0.50 0.50 0.50 0.50 0.58 0.67 0.67 0.75 0.83 0.92 1.00	0.25 0.25 0.25 0.25 0.25 0.25 0.33 0.33 0.42 0.42

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Job No. 25222260.00	Project: Columbia Energy Center MOD 12-13	By: SJL	Date: 8/28/23
Client: WPL	Subject: Energy Dissipator Sizing	Chk'd: RJG	Date: 8/30/23

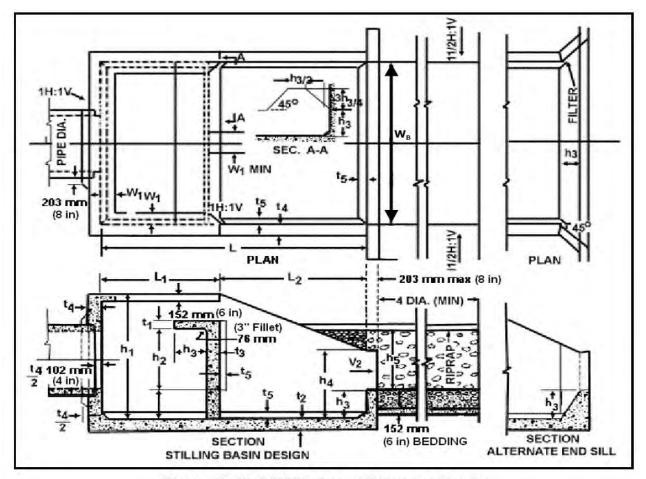


Figure 9.13. USBR Type VI Impact Basin

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Chk'd. RIG	Date	8/30/23

Client: WPL Subject: Energy Dissipator Sizing

Calculations (Continued):

Job No. 25222260.00

## **Downslope Flume 9 - Velocity Calculator (Q = 25.62 cfs)**

Project: Columbia Energy Center MOD 12-13

## Manning Formula Uniform Pipe Flow at Given Slope and Depth

### Inputs:

•		
Pipe Diameter, d₀	18	in
Manning Roughness,		
<u>n</u>	0.0120	
Pressure slope	0.2500	slope
Percent of (or ratio		
to) full depth (100%		
or 1 if flowing full)	0.4705	fraction

### **Results:**

Flow, Q	25.6240	ft^3/s
Velocity, v	31.3544	ft/s
Velocity head, hv	183.3465	in
Flow Area, A	0.8172	ft^2
Wetted Perimeter, P	2.2676	ft
Hydraulic Radius	0.3604	ft
Top Width, T	1.4974	ft
Froude Number, F	7.60	
Shear Stress (tractive		
force), τ	11.0139	psf

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**HawsEDC Calculators** 



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		Calc. No.		
		Rev. No.		
Job No. 25222260.00	Job: Columbia Energy Center MOD 12-13	By: SJL	Date: 8/28/23	
Client: WPL	Subject: Rock Chute Sizing & Riprap Size	Chk'd: RJG	Date: 8/28/23	

### Purpose:

To size the rock chutes to accommodate the 25-year, 24-hour storm event.

### References:

- 1. Rock Chute Design Data spreadsheet Version WI-April-2005, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998.
- 2. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 3. Figure 1 Final Grades (Module 13)
- 4. Stable 25.1 Typical Particle Sizes of Native Sands at 75 Percent Passing (D75) from WisDOT Facilities. Development Manual (FDM).

### Approach:

- 1. Enter Inlet Channel data based on culvert apron or swale geometry Reference #2 and #3.
- 2. Enter Chute data based on slope from Reference #3, start the width, Bw equal to inlet channel Bw.
- 3. Enter Outlet Channel data based on Reference #3, start the width, Bw equal to inlet channel Bw.
- 4. Enter drainage area, apron elevations, flow (Q), and rainfall.
- 5. Adjust Bw for Chute and Outlet Channel until spreadsheet shows the rock chute "will" function adequately.
- 6. Determine rip rap classification based on D50 weight per Reference #4.

### **Assumptions:**

- 1. Assume side slopes of chute and outlet channel are 2:1.
- 2. Assume Factor of Safey is 1.2.
- 3. n-value is based on proposed conditions at the channel.
- 4. Assume Outlet apron depth, d is 1.0 ft.
- 5. Freeboard is 1.0 ft.
- 6. Use 25-year, 24-hour storm event flow (Reference #2) for  $Q_{high}$  and  $Q_{low}$ .
- 7. Classification of riprap is based on weight (Reference #4).

### Calculations:

See attached spreadsheet calcs for each rock chute.

### Results:

The rock chutes are adequately designed to accommodate the flows from the 25-year, 24-hour storm event.

Rock Chute	Width (ft)	Thickness (in)	Apron Width (ft)	Apron Length (ft)	D ₅₀ (in)	WisDOT Rip Rap Classification
RC1	8	4	8	2	2	Select Crushed Material, Type R
RC2	6	12	6	7	5.9	Light Riprap Type R
RC3	6	8	6	5	3.8	Light Riprap Type R
RC4	6	9	6	6	3.8	Light Riprap, Type R

(Version WI-April-2005, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998)

Revised for WisDOT 9/2010 Project: COL - Mod 12-13 RC1 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 0.0 Bottom Width = 8.0 Bottom Width = 8.0 Side slopes = 2.0 (m:1) Side slopes = 4.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.030 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0600 ft./ft. Bed slope = **0.1769** ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Outlet 806.0 ft. --- (H_{drop} = Note: The total required capacity is routed Apron elev. --- Inlet = 829.0 ft. --Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw):  $Q_{high} = 2.3$ cfs High flow storm through chute ➤ Tw (ft.) = Program  $Q_{low} = 2.3$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 3.03 \text{ ft.} (3.03 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 3.06 \text{ ft.}$  $h_{cv} = 0.07 \text{ ft.} (0.07 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.2 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.1 \text{ ft.}$  $H_{\rm p} = 0.03 \, \text{ft}$  $d_1 = 0.09 \text{ ft.}$ Inlet  $(0.03 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height, d₂ = 0.19 ft. (0.19 ft.) Channe (0.09 ft.)Inlet Apron  1   $y_{n} = 0.38$  ft.  $10y_{c} =$ Tw+d = 1.28 ft. - Tw o.k.(0.38 ft.) (1.28 ft.) - Tw o.k. 40*Design D₅₀ = 6 ft Velocity_{inlet} = 3.93 fps radius 0.28 ft. (0.28 ft.) Outlet Channel at normal depth Critical Slope check upstream is unstable Slope = 0.005 ft./ft Geotextile ¹ **Note**: When the normal depth  $(y_n)$  in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 0.95 fpsVelocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness =  $d_2 =$ 0.19 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 8.2 ft

(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)
Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12-13 RC2 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 6.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 1.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.012 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0001 ft./ft. Bed slope = 0.2319 ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Apron elev. --- Inlet = 820.0 ft. --Outlet 804.0 ft. --- (H_{drop} = Note: The total required capacity is routed Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw): Q_{high}= **12.1** cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 12.1$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.12 \text{ ft.} (0.12 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.77 \text{ ft.}$  $h_{cv} = 0.21 \text{ ft.} (0.21 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.68 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.34 \text{ ft.}$  $H_{\rm p} = 0.64 \, \text{ft}$  $d_1 = 0.29 \text{ ft.}$ Inlet  $(0.64 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height,  $d_2 = 0.73$  ft. (0.73 ft.) Channe (0.29 ft.) Inlet Apron  $^{1}\dot{y}_{n} = 1.33 \text{ ft.}$  $10y_c = 5$ Tw+d = 1.86 ft. - Tw o.k.(1.33 ft.) (1.86 ft.) - Tw o.k. 40*Design D₅₀ = 16 ft Velocity_{inlet} = 1.24 fps radius 0.86 ft. (0.86 ft.) Outlet at normal depth Channel Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.81 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness = 11.8 in.  $d_2 =$ 0.73 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 6.8 ft

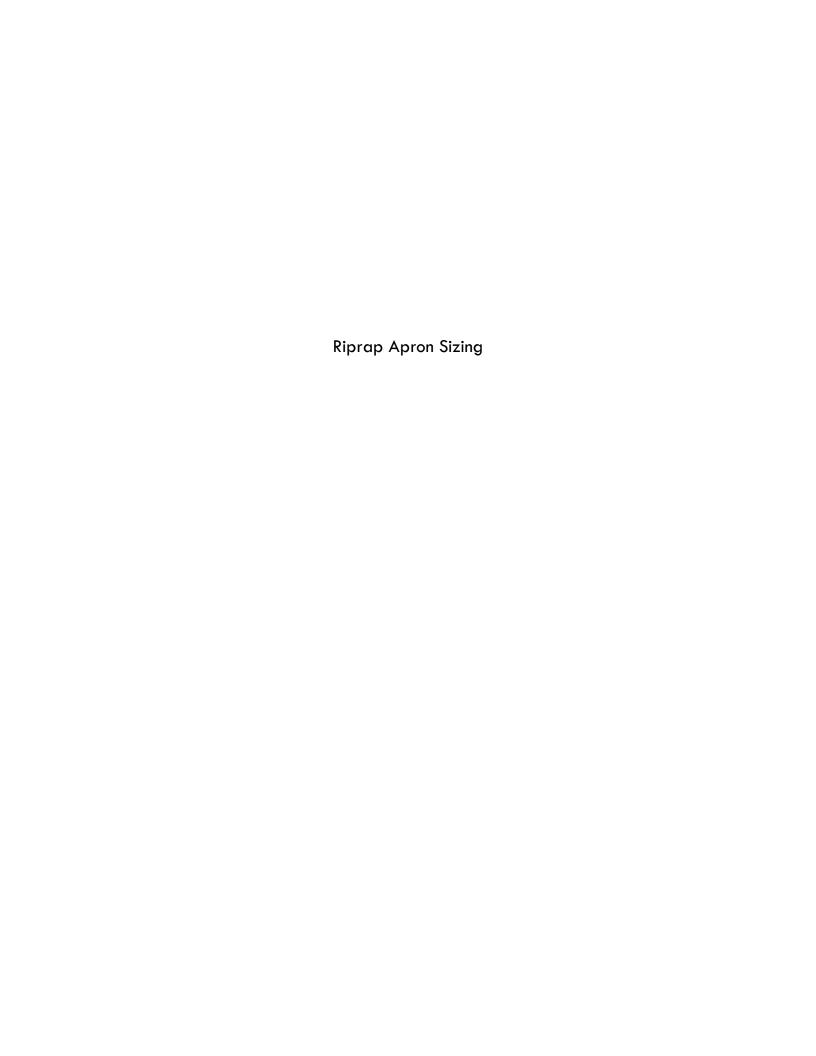
(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)
Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12-13 RC3 County: Columbia Checked by: RJG Designer: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 6.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 1.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.012 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0001 ft./ft. Bed slope = **0.1545** ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Apron elev. --- Inlet = 821.0 ft. --Outlet 804.0 ft. --- (H_{drop} = Note: The total required capacity is routed Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw): Q_{high}= **6.6** cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 6.6$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.09 \text{ ft.} (0.09 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.52 \text{ ft.}$  $h_{cv} = 0.15 \text{ ft.} (0.15 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.47 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.23 \text{ ft}$  $H_{\rm p} = 0.43 \, \text{ft}$  $d_1 = 0.21 \text{ ft.}$ Inlet  $(0.43 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height, d₂ = 0.46 ft. (0.46 ft.) Channe (0.21 ft.)Inlet Apron  1   $y_{n} = 0.93$  ft. Tw+d = 1.61 ft. - Tw o.k.(0.93 ft.) (1.61 ft.) - Tw o.k. 40*Design D₅₀ = 11 ft Velocity_{inlet} = 1.02 fps radius 0.61 ft. (0.61 ft.) Outlet Channel at normal depth Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.49 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge .04 cfs/ft Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness Tw + d =Tailwater above outlet apron Rock thickness = 7.7 in.  $d_2 =$ 0.46 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d2. B' = 6.8 ft

(Version WI-April-2005, Based on <u>Design of Rock Chutes</u> by Robinson, Rice, Kadavy, ASAE, 1998)

Revised for WisDOT 9/2010

Revised for WisDOT 9/2010 Project: COL - Mod 12-13 RC4 County: Columbia Designer: RJG Checked by: SJL Date: August 28, 2023 Date: 08/28/23 Input Geometry: Upstream Channel **Downstream Channel** Bottom Width = 4.0 Bottom Width = 6.0 Bottom Width = 6.0 Side slopes = 2.0 (m:1) Side slopes = 4.0 (m:1) Factor of safety = 1.20 1.2 Min Mannings n value = 0.045 Side slopes = 2.0 (z:1) 2.0:1 max. Mannings n value = 0.045 Bed slope = 0.0288 ft./ft. Bed slope = 0.2212 ft./ft Bed slope = 0.0050 ft./ft. 3.0:1 max. Freeboard = 1.0 ft. Note: Use procedures 13-30-15 or upstream and downstream Mannings n Outlet apron depth, d = 1.0 ft. Base flow = 0.0Flow and Elevation Data: Apron elev. --- Inlet = 824.9 ft. -Outlet 817.6 ft. --- (H_{drop} = Note: The total required capacity is routed Degree of angularity = through the chute (principal spillway) or in combination with an auxiliary spillway. Q_{high} = Runoff from design storm 1 --> 50% angular, 50% rounded Q₅ = Runoff from a 5-year,24-hour storm 2 --> 100 % rounded Input tailwater (Tw):  $Q_{high} = 7.0$ cfs High flow storm through chute → Tw (ft.) = Program  $Q_{low} = 7.0$ Low flow storm through chute ➤ Tw (ft.) = Program cfs **Profile and Cross Section (Output):** Starting Station = 3+00.0 Notes:  $h_{pv} = 0.13 \text{ ft.} (0.13 \text{ ft.})$ 1) Output given as High Flow (Low Flow) values.  $H_{pe} = 0.56 \text{ ft.}$  $h_{cv} = 0.15 \text{ ft.} (0.15 \text{ ft.})$ 2) Tailwater depth plus d must be at or above the  $H_{ce} = 0.49 \text{ ft.}$ hydraulic jump height for the chute to function. **Energy Grade Line** 3) Critical depth occurs 2y_c - 4y_c upstream of crest.  $0.715y_c = 0.24 \text{ ft.}$  $H_{\rm p} = 0.43 \, \text{ft}$  $d_1 = 0.21 \text{ ft.}$ Inlet  $(0.43 \text{ ft.}) \text{ y}_{c} =$ Hydraulic Jump Height,  $d_2 = 0.51$  ft. (0.51 ft.) Channe (0.21 ft.)Inlet Apron  1   $y_{n} = 0.45$  ft. Tw+d = 1.63 ft. - Tw o.k.(0.45 ft.) (1.63 ft.) - Tw o.k. 40*Design D₅₀ = 12 ft Velocity_{inlet} = 2.72 fps radius 0.63 ft. (0.63 ft.) Outlet Channel at normal depth Critical Slope check upstream is OK Slope = 0.005 ft./ft Geotextile ¹ Note: When the normal depth (y_n) in the inlet Outlet Apron channel is less than the weir head (H_n), ie., the weir capacity is less d = 1 ft. {1 ft. minimum than the channel capacity, restricted flow or ponding will occur. This  $15(D_{50})(F_s)$ suggested} reduces velocity and prevents erosion upstream of the inlet apron. 1.52 fps Velocity_{outlet} = at normal depth **Profile Along Centerline of Chute Typical Cross Section** Equivalent unit discharge Freeboard = 1 ft SF = Factor of safety (multiplier)  $d_1 =$ Normal depth in chute Geotextile n-value = Manning's roughness coefficient  $D_{50}(SF) =$ Minimum Design D₅₀*  $2(D_{50})(SF) =$ Rock chute thickness 8.8 in Tw + d =Tailwater above outlet apron Rock thickness = 8.8 in.  $d_2 =$ 0.51 ft. Hydraulic jump height Use H_D along chute *** The outlet function adequately but not less than d₂. B' = 6.8 ft



#### SCS ENGINEERS

Sheet No.	1 of 2
Calc. No.	
Rev. No.	2
By: SJL	Date: 8/28/23

Job No. 25222260.00 Job: Columbia Energy Center MOD 12-13 Client: WPL Subject: Riprap Sizing at Culvert Outlet Chk'd: RJG Date: 8/28/23

#### Purpose:

To size the riprap apron dimensions at culvert C2, C3, C4, C5, C8, and C10 based on a 25-year, 24 hour storm event:

#### References:

- 1. "Energy Dissipators," Wisconsin Department of Transportation (WisDOT), Facilities Development Manual (FDM) 13-35-5.
- 2. HydroCAD Report: COL_Mod12-13_HydroCAD Report
- 3. "Rock Riprap Lined Channels," WisDOT FDM 13-30-25.
- 4. Culvert Sizing Calculation.
- 5. WisDOT FDM Chapter 13, Section 30 Rock Riprap Lined Chutes

#### Approach:

Use the equations in Section 5.2 - Riprap Blanket of WisDOT FDM 13-35-5 (Energy Dissipators) to determine the average size of stone (d₅₀) and riprap apron length. Round up the calculated  $d_{50}$  to the nearest WisDOT standard riprap size.

Use WisDOT FDM 13-35 Attachment 5.2 to determine the width of the riprap apron for discharges to a flat area. For discharges to channels, extend riprap across the channel bottom and up the sides.

#### Assumptions:

Assume riprap apron thickness (T) is  $2 * d_{50}$  to protect against washout and undercutting of the riprap.

Assume tailwater depth, TW = 0.40 * D_o

Assume max TW conditions for the riprap apron width.

Assume that when there are multiple culverts, the total discharge to the culverts is distributed evenly through each barrel.

#### Calculation:

From WisDOT Section 5.2 - Riprap Blanket:

 $d_{50}/D_o = 0.020 (D_o/TW) (Q/D_o^{5/2})^{4/3}$ 

 $L_{sp}/D_o = 1.7 (Q/D_o^{5/2}) + 8$ 

$$d_{50} = 0.02 \times (D_o/TW) \times (Q/D_o^{5/2})^{4/3} \times D_o$$
  
 $L_{sp} = (1.7 (Q/D_o^{5/2}) + 8) \times D_o$ 

where:  $D_o = Diameter or width of culvert (ft)$ 

 $Q = Flow \ rate \ (cfs) \ (discharge \ rate \ through \ culvert, \ from \ Worst \ Case \ Condition \ HydroCAD \ Model \ (Reference \ \#2))$ 

TW = Tail water depth (ft)

 $d_{50}$  = Average size of stone (ft)

 $L_{sp}$  = Length of stone protection (Apron Length) (ft)

Location	Total Flow (Q, cfs)	Number of Pipes	D _o (ft)	Q (cfs)	TW (ft)	d _{50 calculated}	d _{50 Design}	$L_{sp}$
Culvert C2	10.95	2	2	5.5	0.80	0.10	0.18	19
Culvert C3	28.58	2	2.5	14.3	1.00	0.20	0.83	26
Culvert C4	34.14	2	2.5	1 <i>7</i> .1	1.00	0.26	0.83	27
Culvert C5	37.41	2	2.5	18.7	1.00	0.29	0.83	28
Culvert C8	0.96	1	1	1.0	0.40	0.05	0.18	10
Culvert 10	2.76	1	2	2.8	0.80	0.04	0.18	18

#### Results:

Below is a summary of the  $d_{50}$ , thickness (T), and configuration of the riprap apron. Also refer to WisDOT FDM Attachment 5.2 (Sheet 2) for details on apron layout. Use WisDOT Light Riprap at culvert discharge.

Location	d ₅₀ (in)*	T (in)	L _{sp} (ft)	W _{sp} (ft)	WisDOT Riprap sizes
Culvert C2	2.2	6	19	See Note 1	Select Crushed Material
Culvert C3	10.0	20	26	See Note 1	Light Riprap
Culvert C4	10.0	20	27	See Note 1	Light Riprap
Culvert C5	10.0	20	28	See Note 1	Light Riprap
Culvert C8	2.2	6	10	See Note 1	Select Crushed Material
Culvert 10	2.2	6	18	See Note 1	Select Crushed Material

^{1.} For discharges to channels, place riprap along channel bottom and up side of channel.

### SCS ENGINEERS

Job No. 25222260.00

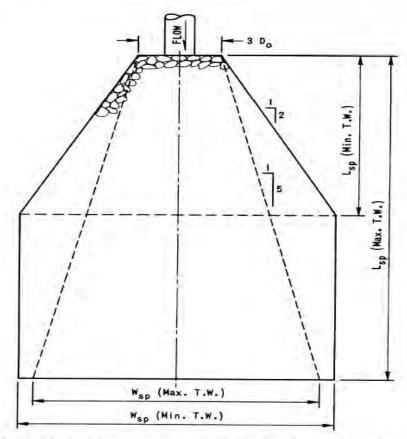
Client: WPL

Sheet No.	2 of 2
Calc. No.	
Rev. No.	
By: SJL	Date: 8/28/23
Chk'd: RJG	Date: 8/28/23

FDM 13-35 Attachment 5.2 Recommended Configuration of Riprap Blanket Subject to Maximum and Minimum Tail Waters

Job: Columbia Energy Center MOD 12-13

Subject: Riprap Sizing at Culvert Outlet



# RECOMMENDED CONFIGURATION OF RIPRAP BLANKET SUBJECT TO MAXIMUM AND MINIMUM TAILWATERS

Source: Miscellaneous paper H-72-5, "Practical Guidance for Estimating and Controlling Erosion at Gulvert Outlets", U.S. Army Engineer Waterways Experiment Station, May, 1972.

Table 25.1 Typical Particle Sizes of Native Sands at 75 Percent Passing (D75)

Riprap Type	D50 (inches)	D50 (feet)	Riprap Thickness (in)	Geotextile Type
Select Crushed Material	2.2	0.18	5	Type R
Light Riprap	10	0.83	12	Type R
Medium Riprap	12.5	1.04	18	Type HR
Heavy Riprap	16	1.33	24	Type HR
Extra-Heavy Riprap	20	1.67	30	Type HR

Source: Table 25.1 from WisDOT FDM.

Appendix F

Closure Plan

# Closure Plan

Columbia Dry Ash Disposal Facility

Phase 1 Module 1

Phase 1 Module 2

Phase 1 Module 3

Phase 1 Module 4

Phase 1 Module 5

Phase 1 Module 6

Phase 2 Module 10

Phase 2 Module 11

Phase 2 Module 12

Phase 2 Module 13

# Prepared for:

Wisconsin Power and Light Company Columbia Energy Center W8375 Murray Road Pardeeville, Wisconsin 53954

# SCS ENGINEERS

25222260.00 | September 1, 2023

2830 Dairy Drive Madison, WI 53718-6751 608-224-2830

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i



# PE CERTIFICATION



I, Phillip Gearing, hereby certify that I am a licensed professional engineer in the State of Wisconsin in accordance with the requirements of ch. A–E 4, Wis. Adm. Code; that this document has been prepared in accordance with the Rules of Professional Conduct in ch. A–E 8, Wis. Adm. Code; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in chs. NR 500 to 538, Wis. Adm. Code.

Specifically,

 This Closure Plan was prepared by me or under my direct supervision and meets the requirements of 40 CFR 257.102(b) and NR 514.07(10)(c)

Mills Hain	September 1, 2023
(signature)	(date)
Phillip E. Gearing	
(printed or typed name)	
License number <u>E-45115</u>	
My license renewal date isJuly 31, :	2024
Pages or sheets covered by this seal:	
ALL	



#### 1.0 INTRODUCTION AND PROJECT SUMMARY

On behalf of Wisconsin Power and Light Company (WPL), SCS Engineers (SCS) has prepared this Closure Plan for the Columbia (COL) Dry Ash Disposal Facility Phase 1, Modules 1 through 6 and Phase 2, Modules 10 through 13 as required by 40 Code of Federal Regulations (CFR) 257.102(b) and Wisconsin Administrative Code NR 514.07(10)(c), as stated below.

<u>40 CFR 257.102(b)</u> "Written closure plan – (1) Content of the plan. The owner or operator of a CCR unit must prepare a written closure plan that describes the steps necessary to close the CCR unit at any point during the active life of the CCR unit consistent with recognized and generally accepted good engineering practices. The written closure plan must include, at a minimum, the information specified in paragraphs (b)(1)(i) through (vi) of this section."

NR 517.07(10)(c) "A written closure plan in accordance with the requirements under s. NR 514.06 (10) and all of the following: (1) A narrative description of how the CCR landfill will be closed, including a description of the steps necessary to close the CCR unit at any point during the active life of the CCR unit, consistent with recognized and generally accepted good engineering practices."

The COL facility includes an active coal combustion residual (CCR) landfill, which currently consists of the following modules, located in Phase 1 and Phase 2 of the facility.

- Phase 1, Module 1 This module has received final cover over outer sideslope areas that will no longer receive additional CCR; intermediate cover has been placed over remaining areas. The final cover placed complies with the CCR Rule.
- Phase 1, Module 2 This module has received intermediate cover over a majority of the in-place CCR.
- Phase 1, Module 3 This module has received intermediate cover over a majority of the in-place CCR.
- Phase 1, Module 4 This module is currently being filled and also has received intermediate cover over areas of the in-place CCR.
- Phase 1, Module 5 This module is currently being filled and has received intermediate cover over areas of the in-place CCR.
- Phase 1, Module 6 This module is currently being filled and has received intermediate cover over areas of the in-place CCR.
- Phase 2, Module 10 This module is currently being filled.
- Phase 2, Module 11 This module is currently being filled.
- Phase 2, Module 12 This module is currently proposed for approval. The module will be constructed and filled following approval.
- Phase 2, Module 13 This module is currently proposed for approval. The module will be constructed and filled following approval.

Phase 1, Modules 1 through 3 were previously described as separate existing CCR landfills although they are contiguous and are managed as a single landfill by the facility and by the Wisconsin Department of Natural Resources (WDNR, or "Department"). WPL has clarified in the operating record for the Columbia facility that Modules 1 Through 3 are one existing CCR landfill as defined in 40 CFR 257.53 of the federal CCR Rule. Phase 1, Modules 4 through 6 and Modules 10 and 11 are considered to be a new CCR landfill that initiated construction after October 19, 2015, and is therefore managed as a separate CCR unit under the CCR Rule even though they are contiguous to the existing CCR landfill (Modules 1 through 3). Phase 2, Modules 12 and 13 will be included in the new CCR landfill, if approved and constructed. Construction of additional modules beyond Modules 12 and 13 is not currently planned prior to retirement of the Columbia Energy Center, which is currently scheduled to occur no later than June 1, 2026.

**Figure 1** shows the site location. **Figure 2** shows the closure areas. A detail of the final cover system is shown on **Figure 3**.

#### 2.0 PROPOSED CLOSURE PLAN NARRATIVE

40 CFR 257.102(b)(1)(i) "A narrative description of how the CCR unit will be closed in accordance with this section."

NR 517.07(10)(c)(1) "A narrative description of how the CCR landfill will be closed, including a description of the steps necessary to close the CCR unit at any point during the active life of the CCR unit, consistent with recognized and generally accepted good engineering practices."

When CCR placement is completed in the CCR unit, or if early closure is required, the unit will be closed by covering the CCR with the final cover system described in **Section 3.0**. Prior to final cover system construction, the CCR surfaces will be graded and compacted to establish a firm subgrade for final cover construction. In addition, all required notifications will be submitted to the WDNR, and WPL will obtain all additional necessary permits (for example, general permit coverage for construction storm water management). WPL may also engage in procurement activities to secure services for installing the final cover system.

The timing for completion of CCR placement in the units that are addressed with this closure plan will depend on CCR generation and disposal rates. Future CCR unit development will also impact the timing of closure. Each of the existing CCR units is designed to receive additional CCR once adjacent units are constructed and overlay airspace is available for filling. Based on the current CCR units alone, if early closure of all units is required, final cover will be placed in the active landfill areas shown on **Figure 2**. A closure schedule is discussed in **Section 6.0** and presented in **Appendix B**.

The initiation of closure activities will commence no later than 30 days after the known final receipt of CCR as required by 40 CFR 257.102(e)(1) and NR 506.083(2)(a), or in accordance with 40 CFR 257.102(e)(2) and NR 506.083(2)(b).

#### 3.0 FINAL COVER SYSTEM AND PERFORMANCE

40 CFR 257.102(b)(1)(iii) "If closure of the CCR unit will be accomplished by leaving CCR in place, a description of the final cover system, designed in accordance with paragraph (d) of this section, and the methods and procedures to be used to install the final cover. The closure plan must also discuss how the final cover system will achieve the performance standards specified in paragraph (d) of this section."

40 CFR 257.102(d) "Closure performance standard when leaving CCR in place."

40 CFR 257.102(d)(1) "The owner or operator of a CCR unit must ensure that, at a minimum, the CCR unit is closed in a manner that will:"

40 CFR 257.102(d)(1)(i) "Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere;"

NR 514.07(10)(c)(3) "A demonstration, including a narrative discussion, of how final closure will meet the performance standards under s. NR 506.083(6)."

NR 506.083(6) "Closure performance standards when leaving CCR in place. An owner or operator of a CCR landfill shall ensure that, at a minimum the CCR landfill is closed in a manner that will achieve all of the following performance standards:"

NR 506.083(6)(a) "Control, minimization or elimination, to the maximum extent feasible, of post-closure infiltration of liquids into the waste and of releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere."

The final cover system design will minimize or eliminate infiltration, as further described below.

40 CFR 257.102(d)(1)(ii) "Preclude the probability of future impoundment of water, sediment, or slurry;"

NR 506.083(6)(b) "Prevention of the impoundment of water, sediment or slurry."

The final cover system will meet these criteria, as further described below.

40 CFR 257.102(d)(1)(iii) "Include measures that provide for major slope stability to prevent the sloughing or movement of the final cover system during the closure and post-closure care period;"

NR 506.083(6)(c) "Slope stability to prevent the sloughing or movement of the final cover system during the closure and long-term care period.

The final cover system is designed to provide slope stability and to prevent sloughing or movement during the closure and post-closure care period. Stability of the final cover system was assessed as part of the WDNR landfill permitting process and is further addressed below.

40 CFR 257.102(d)(1)(iv) "Minimize the need for further maintenance of the CCR unit; and"

NR 506.083(6)(d) "Minimization of the need for long-term maintenance of the CCR landfill."

Maintenance of the final cover will be minimized by the establishment of vegetative cover and the erosion control systems, which are further described below.

40 CFR 257.102(d)(1)(v) "Be completed in the shortest amount of time consistent with recognized and generally accepted good engineering practices."

NR 506.083(6)(e) "Complete closure in the shortest amount of time consistent with recognized and generally accepted good engineering practices."

All closure activities for the CCR units will be completed within 6 months, as stated in **Section 7.0** below.

40 CFR 257.102(d)(2) "Drainage and stabilization of CCR surface impoundments."

This does not apply to the COL CCR landfill units.

40 CFR 257.102(d)(3) "Final cover system"

NR 517.07(10)(c)(2) "A description of the final cover system, designed in accordance with s. NR 504.07, and the methods and procedures to be used to install the final cover."

The existing final cover system (see **Figure 3** for details) in place on part of Module 1 is as follows from the bottom up:

- 3-inch grading layer
- Geosynthetic clay liner (GCL)
- 40-millimeters (mil) linear low-density polyethylene (LLDPE) geomembrane
- 12 inches of drainage material
- 12 inches of rooting zone
- 6 inches of topsoil

Final cover designs have been developed to meet the requirements of NR 504.07 and are discussed in detail below.

The final cover system below will be extended to cover the remaining portion of Module 1 (see **Figure 3** for details). The future Module 1 final cover system consists of the following from the bottom up:

- 3-inch-thick grading layer
- 12-inch-thick capillary break/barrier soil
- 12-inch-thick clay barrier soil
- GCL
- 40-mil polyethylene geomembrane
- 12-inch-thick drainage layer (sand)
- 18-inch-thick rooting zone layer
- 6-inch-thick topsoil laver

These final cover systems meet and exceed the minimum requirements of 40 CFR 257.102(d)(3)(i)(A) through (D) and NR 504.12(4)(b)(1) through (4) as follows:

• Per 257.102(d)(3)(i)(A) and NR 504.12(4)(b)(1), the permeability of the final cover system is less than or equal to the permeability of the bottom liner system and is less than 1x10-5 centimeters per second (cm/sec) required by the rule. The COL cover system contains a GCL with a permeability of 5x10-9 cm/sec. The geomembrane above the GCL makes the cover system even less permeable.

The bottom liner system for the existing CCR landfill in Module 1 is as follows:

- Phase 1, Module 1 South:
  - GCL.
  - 40-mil high density polyethylene (HDPE) geomembrane.
  - The layers of the liner system are less than the cover system layers; therefore, infiltration will be more than the cover system.
- Phase 1, Module 1 North:
  - 3 feet of compacted ash.
  - The liner here does not include a geomembrane, and therefore the infiltration through the cover system will be less than this base liner.

A final cover system will be installed in future remaining areas of final cover north of Module 1 (Phase 1, Modules 2, 3, 4, 5, and 6 and Phase 2, Modules 10, 11, 12, and 13) and consists of the following components, from bottom to top:

- 3-inch-thick grading layer
- 12-inch-thick capillary break/barrier soil
- 12-inch-thick clay barrier soil
- GCL
- 40-mil polyethylene geomembrane
- Geocomposite drainage layer
- 30-inch-thick rooting zone layer
- 6-inch-thick topsoil layer

This final cover meets and exceeds the minimum requirements of 40 CFR 257.102(d)(3)(i)(A) through (D) and NR 504.12(4)(b)(1) through (4) as follows:

• Per 257.102(d)(3)(ii)(A), 257.102(d)(3)(i)(A), and NR 504.12(4)(b)(1), the permeability of the final cover system is less than or equal to the permeability of the bottom liner system and is less than 1x10-5 centimeters per second (cm/sec) required by the rule. The COL cover system contains a GCL with a permeability of 5x10-9 cm/sec. The geomembrane above the GCL makes the cover system even less permeable.

The bottom liner system for the existing CCR landfill is as follows:

- Phase 1, Modules 2 and 3:
  - 2 feet of compacted clay
  - GCL
  - 60-mil HDPE geomembrane

The bottom liner system for the new CCR landfill is as follows:

- Phase 1, Modules 4, 5, and 6 and Phase 2, Modules 10, 11, 12, and 13:
  - 2 feet of compacted clay
  - GCL
  - 60-mil HDPE geomembrane

Based on a comparison of the design slopes and drainage system components in the liner system and final cover system (described in greater detail below), the final cover system is at least equivalent in permeability when compared to the liner system in Phase 1, Modules 1, 2, 3, 4, 5, and 6 and Phase 2, Modules 10, 11, 12, and 13.

- Per 257.102(d)(3)(i)(B), the existing final cover system includes 2.5 feet of soil, which is greater than the 18 inches of earthen material required to minimize infiltration.
- Per 257.102(d)(3)(i)(B), the future Module 1 final cover system includes 5.0 feet of soil, which is greater than the 18 inches of earthen material required to minimize infiltration.
- Per 257.102(d)(3)(ii)(A) and 257.102(d)(3)(i)(B), the future final cover system north of Module 1 includes 5.0 feet of soil, which is greater than the 18 inches of earthen material required to minimize infiltration.
- Per NR 504.12(4)(b)(2), the proposed final cover contains a GCL infiltration layer. Water infiltrating the final cover will be contained in the drainage layers (sand and geocomposite), which will limit infiltration further through the final cover system. A soil barrier layer has been added below the final cover GCL.
- Per 257.102(d)(3)(i)(C) and NR 504.12(4)(b)(3), erosion of the existing final cover system is minimized with a vegetative support layer consisting of 12 inches of uncompacted rooting zone material and 6 inches of topsoil. This provides more than the required 6-inch thickness for plant growth.
- Per 257.102(d)(3)(i)(C) and NR 504.12(4)(b)(3), erosion of the future Module 1 final cover system is minimized with a vegetative support layer consisting of 18 inches of uncompacted rooting zone material and 6 inches of topsoil. This provides more than the required 6-inch thickness for plant growth.
- Per 257.102(d)(3)(ii)(B), 257.102(d)(3)(i)(C), and NR 504.12(4)(b)(3), erosion of the final cover system (north of Module 1) is minimized with a vegetative support layer consisting of 30 inches of uncompacted rooting zone material and 6 inches of topsoil. This provides more than the required 6-inch thickness for plant growth.

Also, the existing final cover system, the future Module 1 final cover system, and the final cover system north of Module 1 limits infiltration while promoting surface water run-off in a controlled manner to minimize erosion and promote stability. The surface layer of 18 inches (existing), 24 inches (future Module 1), or 36 inches (north of Module 1) of soil supports vegetation that assists with erosion control. Water that infiltrates will be collected by the 12-inch drainage layer (existing and future Module 1) or geocomposite drainage layer (north of Module 1) and will be routed to the perimeter drainage system.

In addition, the surface has intermediate drainage swales to reduce the flow lengths down the final cover slope, also aiding in erosion control. Where needed, the intermediate drainage swales are connected to downslope channels to control storm water runoff and prevent erosion of the final cover.

- Per 257.102(d)(3)(i)(D) and NR 504.12(4)(b)(4), the design of the existing final cover system minimizes disruptions to the final cover system. Stability of the final cover system was assessed as part of the WDNR landfill permitting process. The stability calculations are included in Appendix A1.
- Per 257.102(d)(3)(i)(D) and NR 504.12(4)(b)(4), the design of the future Module 1 final cover system minimizes disruptions to the final cover system. Stability of the final cover system was assessed as part of the WDNR landfill permitting process. The stability calculations are included in **Appendix A2**.
- Per 257.102(d)(3)(ii)(C) and NR 504.12(4)(b)(4), the design of the final cover system north of Module 1 minimizes disruptions to the final cover system. Stability of the final cover system was assessed as part of the WDNR landfill permitting process. The stability calculations are included in **Appendix A3**.

The design of the final cover system accommodates settling and subsidence of the CCR fill below the cover. The CCR at COL is placed dry and is compacted in place. CCR continues to consolidate and gain strength as filling progresses prior to final cover placement. The final cover system is designed with a maximum slope of 25 percent (4 horizontal to 1 vertical). Because the final cover has a relatively large positive slope and the CCR has been gaining strength over time, the final cover is expected to easily accommodate the remaining relatively minor settlement potential of the CCR fill when fill placement ends and the landfill is closed.

Construction of each of the final cover systems will be performed per methods and procedures described in NR 504, NR 516, and the site-specific Construction Quality Assurance / Quality Control Plan. All final cover materials will be tested to confirm they meet the code requirements and project documents. Barrier soil, rooting zone, and topsoil layers will be checked for thickness. All areas will be restored after final cover is placed. Vegetation will be monitored and maintained. Construction activities will be documented by a licensed engineer.

#### **4.0** MAXIMUM INVENTORY OF CCR

40 CFR 257.102(b)(1)(iv) "An estimate of the maximum inventory of CCR ever on-site over the active life of the CCR unit."

NR 514.07(10)(c)(4) "An estimate of the maximum volume in cubic yards of CCR that will be disposed on—site over the active life of the CCR landfill."

The following table reflects the estimated maximum volume of CCR disposed on-site at the COL facility.

Area	Maximum Capacity (cy)
Phase 1, Modules 1-6, Phase 2, Modules 10-13	3,630,075

The estimated maximum inventory of CCR ever on-site over the active life of the CCR landfill units is based on the design capacity of the landfill. The maximum design capacity was submitted in the WDNR 2023 Plan of Operation Update, Addendum 2.

#### 5.0 LARGEST AREA OF CCR UNIT REQUIRING FINAL COVER

40 CFR 257.102(b)(1)(v) "An estimate of the largest area of the CCR unit ever requiring a final cover as required by paragraph (d) of this section at any time during the CCR unit's active life."

NR 514.07(10)(c)(5) "An estimate of the largest area of the CCR landfill that will require a final cover at any time during the CCR landfill's active life."

The largest area of each CCR unit requiring final cover is the open area shown on **Figure 2**, with areas as follows:

Areas Requiring Final Cover (acres)						
Phase 1, Modules 1-3	10.5					
Phase 1, Modules 4-6	12.0					
Phase 2, Modules 10-11	6.9					
Phase 2, Modules 12-13	7.1					
Total	36.5					

#### **6.0** SCHEDULE OF SEQUENTIAL CLOSURE ACTIVITIES

<u>40 CFR 257.102(b)(1)(vi)</u> "A schedule for completing all activities necessary to satisfy the closure criteria in this section, including an estimate of the year in which all closure activities for the CCR unit will be completed."

NR 514.07(10)(c)(6) "A schedule for completion of all closure activities, including an estimate of the year in which all closure activities for the CCR landfill will be completed."

CCR placement is anticipated to permanently end at this facility following retirement of the Columbia Generating Station by June 2026, as announced by WPL. Some CCR disposal activity may be necessary following retirement of Columbia as part of decommissioning efforts (for example, cleaning of ducts and other equipment that may contain CCR following retirement). Closure activities are expected to be complete by the end of 2027. The potential schedule for closure of the existing CCR modules was divided into two phases and is provided in **Appendix B**.

#### 7.0 COMPLETION OF CLOSURE ACTIVITIES

**40 CFR 257.102(f)(1)** "Except as provided for in paragraph (f)(2) of this section, the owner or operator must complete closure of the CCR unit:

(i) For existing and new CCR landfills and any lateral expansion of a CCR landfill, within six months of commencing closure activities."

NR 506.083(3)(a) "The owner or operator shall complete closure of the CCR landfill within 6 months of commencing closure activities."

As shown on the enclosed schedule, closure of each CCR unit will be completed within 6 months of commencing closure activities.

<u>40 CFR 257.102(f)(3)</u> "Upon completion, the owner or operator of the CCR unit must obtain a certification from a qualified professional engineer verifying that closure has been completed in accordance with the closure plan specified in paragraph (b) of this section and the requirements of this section."

NR 506.083(1)(b) "Within 30 days following completion of closure of a CCR landfill under sub. (3), the owner or operator shall prepare and submit a notification of closure to the department and place a copy in the facility's operating record. The notification shall include the certification required under s. NR 516.04(3)(d)."

A qualified licensed engineer will oversee the final cover construction. The engineer will verify final cover materials and methods and oversee material testing. At the end of construction, the engineer will provide a report summarizing and documenting construction and will certify compliance with the requirements.

#### 8.0 CERTIFICATION

<u>40 CFR 257.102(b)(4)</u> "The owner or operator of the CCR unit must obtain a written certification from a qualified professional engineer that the initial and any amendment of the written closure plan meets the requirement of this section."

NR 500.05 "Unless otherwise specified, all submittals for review and approval of any initial site report, feasibility report, plan of operation site investigation report, remedial action options report, construction documentation report, or closure plan, or any modifications to those plans, shall include all of the following:

(4) CERTIFICATION. (a) The reports and plan sheets shall be under the seal of a licensed professional engineer."

Phillip Gearing, PE, a licensed professional engineer in the State of Wisconsin has overseen the preparation of this Closure Plan. A certification statement is provided on **page iii** of this plan.

<u>40 CFR 257.102(d)(2)(iii)</u> "The owner or operator of the CCR unit must obtain a written certification from a qualified professional engineer that the design of the final cover system meets the requirement of this section."

Phillip Gearing, PE, a licensed professional engineer in the State of Wisconsin has overseen the design of the final cover system and certifies that the design meets the requirements of 40 CFR 257.102(d). The certification statement is provided on **page iii** of this plan.

#### 9.0 RECORDKEEPING AND REPORTING

40 CFR 257.102(b)(vi)(2)(iii) "The owner or operator has completed the written closure plan when the plan including the certification required by paragraph (b)(4) of this section, has been placed in the facility's operating record as required by Section 257.105(i)(4)."

NR 506.17(2)(e) "The written operating record shall contain the plan of operation, plan modifications, construction documentation, department approvals, annual reports, inspection

records, monitoring and corrective action records, notifications to the department, and records of public comments received during any public comment period."

The Closure Plan will be placed in the facility's operating record and on Alliant Energy's CCR Rule Compliance Data and Information website.

Amendments to the written Closure Plan will be done when a new module is constructed, when there is a change in the operation of the CCR unit that affects the plan, or when unanticipated events warrant revision to the written Closure Plan as required by 40 CFR 257.102(b)(3) and NR 514.07(10)(c)(7).

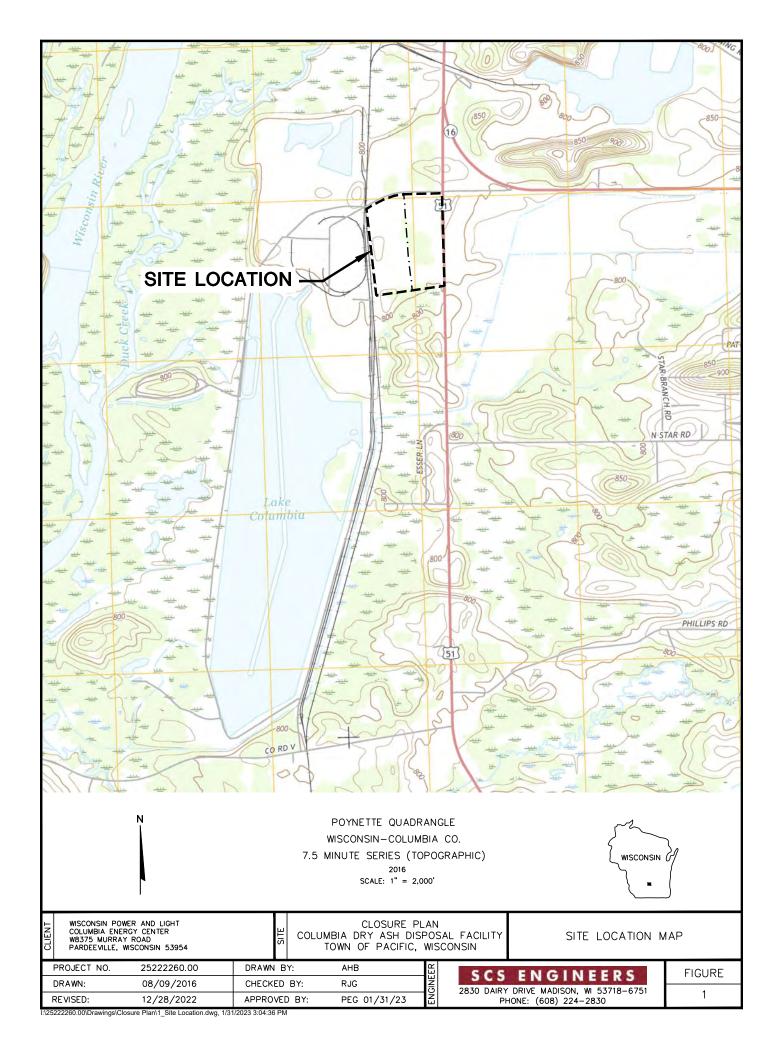
WPL will provide notification as follows:

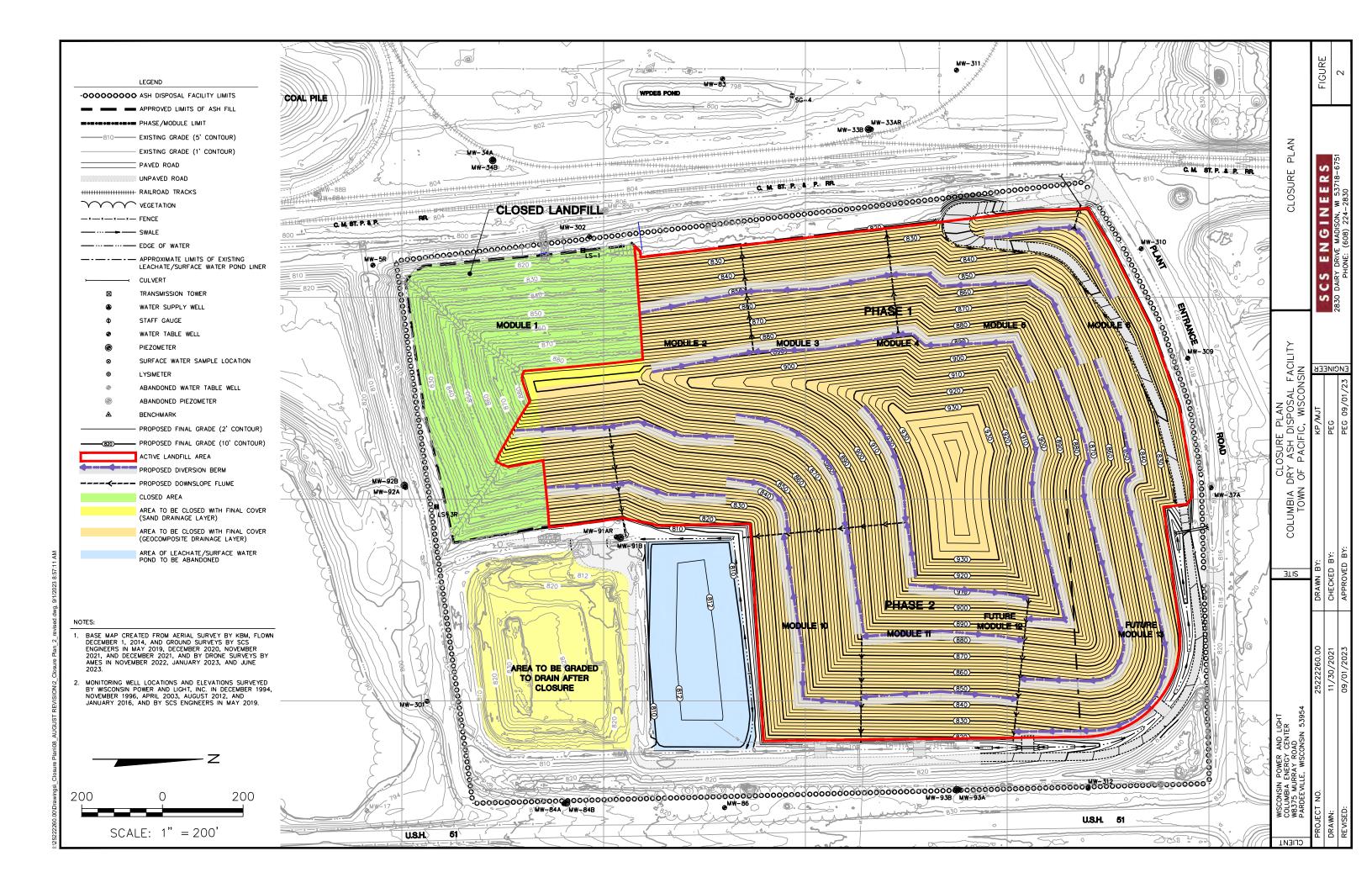
- Intent to initiate closure
- Closure completion
- Availability of the written Closure Plan and any amendments

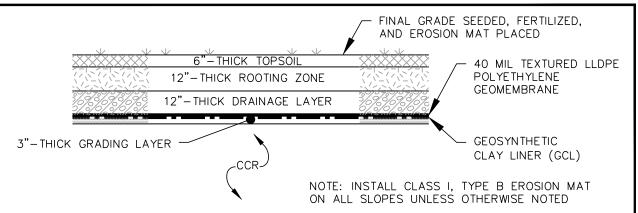
All notifications will be placed in the facility's operating record and on the website per 40 CFR 257.105(i), 257.106(i), 257.107(i), and NR 506.17(2).

# Figures

- 1 Site Location Map
- 2 Closure Plan
- 3 Final Cover System

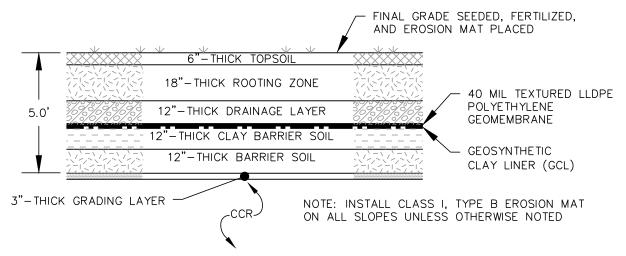






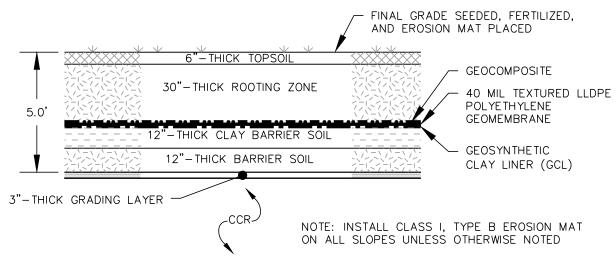
## **EXISTING FINAL COVER SYSTEM**

SCALE: 1" = 4



# FINAL COVER SYSTEM (SAND DRAINAGE LAYER)

SCALE: 1" = 4



## FINAL COVER SYSTEM (GEOCOMPOSITE DRAINAGE LAYER)

SCALE: 1" = 4"

CLIENT	WISCONSIN POWER AND LIGHT COLUMBIA ENERGY CENTER W8375 MURRAY ROAD PARDEEVILLE, WISCONSIN 53954		SITE	CLOSURE PLAN COLUMBIA DRY ASH DISPOSAL FACILITY TOWN OF PACIFIC, WISCONSIN			FINAL COVER SYSTEM				
	PROJECT NO.	25222260.00	DRAWN	BY	r: KP/N	JT	ER	5.05	ENGIN	EEDC	FIGURF
	DRAWN:	08/17/2016	CHECKE	ED I	BY: PEG		빌				1100112
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# Appendix A Stability Calculations

# Appendix A1 Existing Final Cover Stability Calculations



Sheet No.	1
Calc. No.	
Rev. No.	
By: PEG	Date 9/23/10
Childs DIN	Date 0/24/40

			1101.110.		
Job No.	4071	Job: Columbia Ash Generation Landfill	By: PEG	Date 9/23	
Client: Allia	int	Subject: Liner Side Slope Drainage Laver Stability	Chk'd: DLN	Date 9/24	

#### **EVALUATION:**

Evaluate the Phase 1 landfill liner side slope drainage layer for static veneer slope stability.

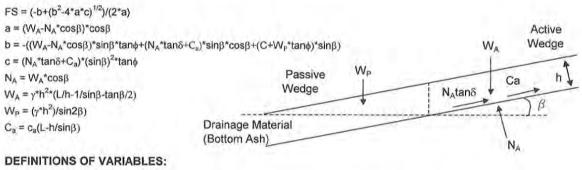
The side slope on the modules base runs at a 3:1 slope for an approximate maximum of 80 feet.

The following calculations evaluate the static veneer slope stability of the 3:1 slope.

#### REFERENCES:

- 1.) Koerner, Robert M. & Te-Yang Soong, Analysis and Design of Veneer Cover Solls, Geosynthetic Research Institute.
- 2.) U.S. Department of Transportation Federal Highway Administration Recycled Materials, Coal Bottom Ash User's Guide

#### **EQUATIONS:**



FS = Factor of Safety

a, b, & c = intermediate variables (= calculated variable)

N_A = Effective force normal to the failure plane of the active wedge ( = calculated variable)

WA = Total weight of active wedge (= calculated variable)

W_P = Total weight of passive wedge (= calculated variable)

 $\beta$  = Soil slope angle beneath the geomembrane ( = 18.42 degrees or 0.322 radians

based on liner slope of 3 to 1 )  $\phi$  = Friction angle of the drainage layer material ( = 35 degrees 0.611 radians based on Ref #2)

 $\phi$  = Friction angle of the drainage layer material ( = ___35__degrees  $\delta$  = Interface friction angle for liner system geosynthetics ( to be determined)

 $c_a$  = Adhesion for liner system geosynthetics at active wedge ( to be determined) , Variable

 $\gamma$  = Unit weight of the drainage layer material ( = 135 pcf based on conservative wet density of bottom ash).

C = Cohesive force along the failure plane of the passive wedge ( assumed 0 for drainage layer material)

Ca = Adhesive force of the active wedge for the liner system geosynthetics

h = Thickness of the drainage layer material(= 1.0 foot based on base design)

L = Length of slope measured along the geomembrane ( = 80 feet based on base design)



Job: Columbia Ash Generation Landfill

Subject: Liner Side Slope Drainage Layer Stability

Sheet No. 2

Calc. No.

Rev. No.

By: PEG Date 9/23/10

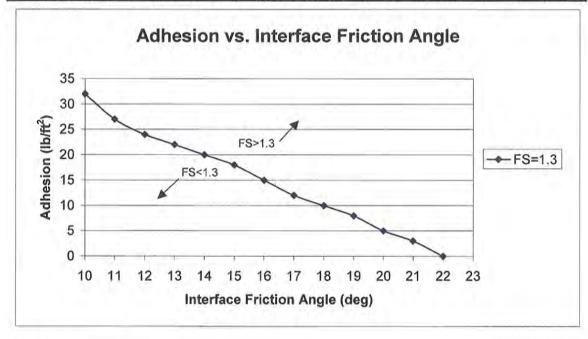
Date 9/24/10

Chk'd: DLN

#### CALCULATIONS:

Client: Alliant

δ		Ca	W _A	W _P	NA	Ca	а	b	С	FS
(deg)	(rad)	(lb/ft ² )	(lb/ft)	(lb/ft)	(lb/ft)	(lb/ft)	(lb/ft)	(lb/ft)	(lb/ft)	
10	0.175	32	10,350	225	9,820	2,459	981	-1,535	293	1.3
11	0.192	27	10,350	225	9,820	2,075	981	-1,473	279	1.3
12	0.209	24	10,350	225	9,820	1,844	981	-1,457	275	1.3
13	0.227	22	10,350	225	9,820	1,690	981	-1,465	277	1.3
14	0.244	20	10,350	225	9,820	1,537	981	-1,473	279	1.3
15	0.262	18	10,350	225	9,820	1,383	981	-1,482	281	1.3
16	0.279	15	10,350	225	9,820	1,153	981	-1,468	277	1.3
17	0.297	12	10,350	225	9,820	922	981	-1,455	274	1.3
18	0.314	10	10,350	225	9,820	768	981	-1,465	277	1.3
19	0.332	8	10,350	225	9,820	615	981	-1,477	279	1.3
20	0.349	5	10,350	225	9,820	384	981	-1,465	277	1.3
21	0.367	3	10,350	225	9,820	231	981	-1,478	280	1.3
22	0.384	0	10,350	225	9,820	0	981	-1,468	277	1.3



#### CONCLUSION:

The landfill liner side slope drainage layer was evaluated for static veneer slope stability along its longest slope. Calculations were performed to determine the minimum adhesion necessary for a range of interface friction angles to reach a FS of 1.3 or greater. Each interface friction angle and the coinciding adhesion was graphed in order to easily determine if a material interface is acceptable along the side slope.



4071

Alliant

Job No.

Client:

Sheet No. 1 of 1

Calc. No.

Rev. No.

By PEG Date 9/27/10

Chk'd DLN Date 9/29/10

Purpose: Determine the maximum shear stress acting on a Geosynthetic Clay Liner (GCL) and the GCL internal shear strength required to provide a minimum slope stability safety factor (FS) of 1.5 for the liner system.

Job: Columbia Ash Generation Landfill

Subject: GCL Internal Shear for Liner System

Approach: Use maximum shear stress formula and assumed values.

References: Design of GCL Barrier for Final Cover Side Slope Applications Gregory N. Richardson, Ph.D., P.E. Geosynthetics '97 - 541

Calculation: The maximum shear stress acting on the GCL can be calculated as follows:

$$\tau_{act} = W_T \sin \beta$$

$$\beta = 18.4^{\circ}$$

$$W_T = v * h$$

Where,

y = Ash Unit Weight = 135 pcf h = drainage layer thickness = 1 ft

$$\tau_{act}$$
 = 42.6 psf

$$FS = \frac{\tau_{resist}}{\tau_{act}} = 1.5$$

$$\tau_{\text{resist}} = FS * \tau_{\text{act}} = 1.5 * 42.6 = 64 \text{ psf}$$

Assumptions: 1. Slope angle, β=18.4° (3:1 horizontal/vertical liner side slope).

2. Ash unit weight, y = 135 pcf

Conclusions: For a total weight of the leachate drainage layer of 135 psf and a slope angle of 3:1, the maximum shear stress will be 42.6 psf. A minimum GCL internal shear strength of 64 psf is required to provide a slope stability safety factor of 1.5.

I:\4071\Calculations\[GCL Internal Shear Stress_100929.xls]GCL Internal Shear